

SITE
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Location Plan



Drawing List

GENERAL INFORMATION DRAWINGS (1 DWG.)		REV #	REV DATE	HVAC DRAWINGS (Continued)		REV #	REV DATE	PLUMBING DRAWINGS (Continued)		REV #	REV DATE	ELECTRICAL DRAWINGS (Continued)		REV #	REV DATE
CS3.2	Cover Sheet - Package #3 (Volume 2 of 2)	1	1/29/10	H2.3	HVAC Piping Third Floor	20	1/29/10	PA1.1	Plumbing - Pratt Garage Level Basement	1	1/29/10	EAD1.1	Pratt Garage Tenth Floor Demolition	1	1/29/10
STRUCTURAL DRAWINGS (31 DWGS. TOTAL)															
S0.1	General Notes	3	1/29/10	H2.6	HVAC Penthouse, 5th Floor Roof Plan	20	1/29/10	PA1.2	Plumbing - Pratt Garage Ninth Floor	1	1/29/10	EAD1.2	Pratt Garage Roof Demolition	1	1/29/10
S1.0	Garden Level and Foundation Plan	3	1/29/10	H2.7	HVAC Penthouse Roof	20	1/29/10	PA1.3	Plumbing - Pratt Garage Tenth Floor	1	1/29/10	EA1.1	Pratt Garage Level 1 - Power	1	1/29/10
S1.1	First Floor Framing Plan	3	1/29/10	H3.0	Shaft Part Plans	20	1/29/10	FP0.1	Notes, Abbrev., Legend, & Schedules	1	1/29/10	EA1.2	Pratt Garage Ninth Floor - Power	1	1/29/10
S1.2	Second Floor Framing Plan	3	1/29/10	H3.1	Shaft Section	20	1/29/10	FP1.0	Garden Level Plan	5	1/29/10	EA1.3	Pratt Garage Tenth Floor - Power	5	1/29/10
S1.3	Third Floor Framing Plan	3	1/29/10	H3.2	Mechanical Room Garden Level	20	1/29/10	FP1.1	First Floor Plan	1	1/29/10	EA2.1	Pratt Garage Tenth Floor - Lighting	2	1/29/10
S1.3A	Pool Support Framing Plan	3	1/29/10	H3.3	Mechanical Room Penthouse	20	1/29/10	FP1.2	Second Floor Plan	5	1/29/10	EA4.1	Athletic Center One-Line Diagram	3	1/29/10
S1.4	Fourth Floor Framing Plan	3	1/29/10	H3.4	HVAC Sections	20	1/29/10	FP1.3	Third Floor Plan	4	1/29/10	ELECTRONIC SECURITY DRAWINGS (11 DWGS.)		REV #	
S1.5	Fifth Floor Framing Plan	3	1/29/10	H3.5	Part Plans	20	1/29/10	FP1.3A	Lower Pool Level Plan	2	1/29/10	ES0.1	Electronic Security Data Sheet	1	1/29/10
S1.6	Penthouse Floor/Low Roof Plan	3	1/29/10	H3.6	Mechanical Rooms, Penthouse, Chiller Plant	3	1/29/10	FP1.4	Fourth Floor Plan	2	1/29/10	ES0.2	Electronic Security - Schedules & Details	6	1/29/10
S1.7	Roof Framing Plan	3	1/29/10	H4.0	HVAC Schedules	20	1/29/10	FP1.5	Fifth Floor Plan	3	1/29/10	ES1.0	Electronic Security - Garden Level Plan	3	1/29/10
S2.1	Typical Sections and Details	3	1/29/10	H4.1	HVAC Schedules	20	1/29/10	FP1.6	Penthouse Plan	2	1/29/10	ES1.1	Electronic Security - First Floor Plan	3	1/29/10
S2.2	Typical Sections and Details	3	1/29/10	H4.2	HVAC Schedules	20	1/29/10	FPA1.1	Pratt Garage Level B-1 Plan	4	1/29/10	ES1.2	Electronic Security - Second Floor Plan	2	1/29/10
S2.3	Typical Sections and Details	3	1/29/10	H5.0	HVAC Details	20	1/29/10	FPA1.2	Pratt Garage Tenth Floor Plan	1	1/29/10	ES1.3	Electronic Security - Third Floor Plan	4	1/29/10
S2.4	Typical Sections and Details	3	1/29/10	H5.1	HVAC Details	20	1/29/10	FPA1.3	Pratt Garage Track Mezzanine Plan	2	1/29/10	ES1.4	Electronic Security - Fourth Floor Plan	4	1/29/10
S2.5	Typical Sections and Details	1	1/29/10	H5.2	Details, Flow Schematics	20	1/29/10	FP2.1	Fire Alarm Riser Diagram	3	1/29/10	ES1.5	Electronic Security - Fifth Floor Plan	2	1/29/10
S3.1	Sections and Details	2	1/29/10	H5.3	Flow Schematic	20	1/29/10	FP2.2	Sprinkler Riser	1	1/29/10	ES1.6	Electronic Security - Penthouse Floor Plan	2	1/29/10
S3.2	Sections and Details	3	1/29/10	H5.4	Flow Schematic	20	1/29/10	FP2.3	Fire Alarm Announcer	2	1/29/10	ESA1.1	Electronic Security - Tenth Floor Plan	3	1/29/10
S3.3	Sections and Details	3	1/29/10	H5.5	Riser Diagram Chiller Plant	2	1/29/10	ES3.0	Electronic Security - Risers	1	1/29/10	ELEVATOR DRAWINGS (1 DWG.)		REV #	
S3.4	Sections and Details	3	1/29/10	H6.0	Automatic Temperature Controls	20	1/29/10	ELECTRICAL DRAWINGS (38 DWGS.)		REV #	REV DATE	COMMUNICATIONS DRAWINGS (23 DWGS.)		REV #	REV DATE
S3.5	Sections and Details	3	1/29/10	H6.1	Automatic Temperature Controls	20	1/29/10	E0.1	Notes, Abbrev., & Legend	1	1/29/10	T0.1	Telecom - Data Sheet	3	1/29/10
S3.6	Sections and Details	3	1/29/10	H6.2	Automatic Temperature Controls	20	1/29/10	E0.2	Site Plan	4	1/29/10	T0.2	Telecom - Details	3	1/29/10
S3.7	Sections and Details	3	1/29/10	H6.3	Automatic Temperature Controls	20	1/29/10	E1.0	Garden Level Plan - Power	11	1/29/10	T1.0	Telecom - Garden Level Plan	3	1/29/10
S4.1	Schedules and Details	3	1/29/10	H6.4	Automatic Temperature Controls	20	1/29/10	E1.1	First Floor Plan - Power	6	1/29/10	T1.1	Telecom - First Floor Plan	5	1/29/10
S4.2	Column Schedule	2	1/29/10	H6.5	Automatic Temperature Controls	4	1/29/10	E1.2	Second Floor Plan - Power	9	1/29/10	T1.2	Telecom - Second Floor Plan	3	1/29/10
S4.3	X-Braced Frames and Details	2	1/29/10	HAD1.1	HVAC Demolition Athletic Center 10th Floor	1	1/29/10	E1.3	Third Floor Plan - Power	7	1/29/10	T1.3	Telecom - Third Floor Plan	2	1/29/10
S4.4	Column and Base Plate Details	2	1/29/10	HAD1.2	HVAC Demolition Athletic Center Roof	1	1/29/10	E1.3A	Lower Pool Level Plan - Power	2	1/29/10	T1.4	Telecom - Fourth Floor Plan	4	1/29/10
S4.5	Column and Base Plate Details	2	1/29/10	HAD1.1	Ductwork Athletic Center 10th Floor	3	1/29/10	E1.4	Fourth Floor Plan - Power	7	1/29/10	T1.5	Telecom - Fifth Floor Plan	2	1/29/10
S5.1	Steel Truss Elevations and Details	2	1/29/10	HAD1.2	Ductwork Athletic Center Roof	1	1/29/10	E1.5	Fifth Floor Plan - Power	6	1/29/10	T1.6	Telecom - Penthouse Floor Plan	1	1/29/10
PRATT GYMNASIUM (3 DWGS.)		PLUMBING DRAWINGS (23 DWGS.)		REV #	REV DATE	E2.0		Garden Level Plan - Lighting		7	1/29/10	T2.0	Telecom - Part Plans	2	1/29/10
SA1.1	Mezzanine Level Plan - Pratt Gymnasium	1	1/29/10	P0.1	Plumbing General Notes, Abbs., Legend, Schedules	4	1/29/10	E2.1	First Floor Plan - Lighting	5	1/29/10	T2.1	Telecom - Risers	2	1/29/10
SA1.2	Root Framing Plan - Pratt Gymnasium	1	1/29/10	P0.2	Plumbing - Site Work Plan	4	1/29/10	E2.2	Second Floor Plan - Lighting	5	1/29/10	TA1.1	Telecom - Tenth Floor Plan	2	1/29/10
SA3.1	Sections and Details - Pratt Gymnasium	1	1/29/10	P1.1	Plumbing - Foundation Plan	1	1/29/10	E2.3	Third Floor Plan - Lighting	5	1/29/10	AV0.1	Audio Visual - Data Sheet	4	1/29/10
HVAC DRAWINGS (43 DWGS.)		REV #		REV DATE		E2.4		Lower Pool Level Plan - Lighting		6	1/29/10	AV0.2	Audio Visual - Schedules & Details	1	1/29/10
H0.0	Legend	1	1/29/10	P1.2	Plumbing - Second Floor	4	1/29/10	E2.5	Fourth Floor Plan - Lighting	1	1/29/10	AV1.0	Audio Visual - Garden Level Plan	3	1/29/10
H1.0	Ductwork Garden Level	20	1/29/10	P1.3	Plumbing - Third Floor	4	1/29/10	E2.6	Fifth Floor Plan - Lighting	8	1/29/10	AV1.1	Audio Visual - First Floor Plan	4	1/29/10
H1.1	Ductwork First Floor	20	1/29/10	P1.3A	Plumbing - Low Level Pool	5	1/29/10	E2.7	Penthouse Plan - Lighting	4	1/29/10	AV1.2	Audio Visual - Second Floor Plan	7	1/29/10
H1.2	Ductwork Second Floor	20	1/29/10	P1.4	Plumbing - Fourth Floor	3	1/29/10	E2.8	Details	3	1/29/10	AV1.3	Audio Visual - Third Floor Plan	5	1/29/10
H1.3	Ductwork Third Floor	20	1/29/10	P1.5	Plumbing - Fifth Floor	2	1/29/10	E2.9	Campus Distribution	1	1/29/10	AV1.4	Audio Visual - Fourth Floor Plan	8	1/29/10
H1.3A	Ductwork Low Level Pool	20	1/29/10	P1.6	Plumbing - Garden Level	1	1/29/10	E2.10	Campus Center One-Line Diagram	2	1/29/10	AV1.5	Audio Visual - Fifth Floor Plan	2	1/29/10
H1.4	Ductwork Fourth Floor	20	1/29/10	P1.7	Plumbing - Roof	1	1/29/10	E2.11	Schedules - Panelboards	4	1/29/10	AV1.6	Audio Visual - Risers	2	1/29/10
H1.5	Ductwork Fifth Floor	20	1/29/10	P2.1	Plumbing - Part Plans	2	1/29/10	E5.1	Schedules - Panelboards	5	1/29/10	AV1.7	Audio Visual - Tenth Floor Athletic Center	2	1/29/10
H2.0	HVAC Piping Garden Level	20	1/29/10	P2.2	Plumbing - Part Plans	5	1/29/10	E5.2	Schedules - Panelboards	3	1/29/10	ACOUSTICAL CONSULTANT: THE SEXTANT GROUP		REV #	
H2.1	HVAC Piping First Floor	20	1/29/10	P3.1	Plumbing Details	1	1/29/10	E5.3	Schedules - Panelboards	8	1/29/10	720 River Commons		1	1/29/10
H2.2	HVAC														

1. CODE

A. ALL CONSTRUCTION SHALL CONFORM WITH THE PROVISIONS OF THE 2003 IBC CODE, AND BALTIMORE CITY SUPPLEMENTS.

2. DESIGN LOADING

A. THE DESIGN DEAD LOAD FOR THE FRAMING IS AS FOLLOWS:

ROOF	FLOORS	MULTI-PURPOSE ROOM FLOOR			
MEMBRANE ROOFING	10 PSF	FLOORING	1 PSF	FLOORING	1 PSF
INSULATION	2 PSF	CONC. SLAB AND DECK	45 PSF	CONC. SLAB AND DECK	45 PSF
STRUCTURAL FRAMING	5 PSF	STRUCTURAL FRAMING	5 PSF	STYROFOAM	1 PSF
METAL DECK	2 PSF	CEILING	1 PSF	NORMAL WT. CONC. TOPPING	50 PSF
CEILING	1 PSF	MECHANICAL	3 PSF	STRUCTURAL FRAMING	5 PSF
MECHANICAL	3 PSF	MISCELLANEOUS	2 PSF	CEILING	1 PSF
MISCELLANEOUS	2 PSF			MECHANICAL	3 PSF
				MISCELLANEOUS	2 PSF
TOTAL DEAD LOAD	25 PSF	TOTAL DEAD LOAD	57 PSF	TOTAL DEAD LOAD	108 PSF
BALLROOM FLOOR					
FLOORING	1 PSF				
CONC. SLAB AND DECK	45 PSF				
STRUCTURAL FRAMING	5 PSF				
ACOUSTICAL CEILING	10 PSF				
MECHANICAL	3 PSF				
MISCELLANEOUS	2 PSF				
TOTAL DEAD LOAD	66 PSF				

THE MINIMUM DESIGN LIVE LOADINGS FOR ALL NEW FRAMING IS AS FOLLOWS:

ROOF	30 PSF	100 PSF UNIFORM LOAD
FRAMED FLOORS	100 PSF	
STAIRS & LOBBIES	100 PSF	
CORRIDORS	100 PSF	
EQUIPMENT RMS., STORAGE	150 PSF	
PATIO ROOF	100 PSF	
BOOKSTORE	125 PSF	
CONSTRUCTION LIVE LOAD FOR COMPOSITE BEAM BEFORE 75% OF f _c	20 PSF	

B. LIVE LOAD REDUCTION BASED ON SECTION 1607.9 IBC 2003 AND SHALL NOT EXCEED:

50% FOR MEMBERS SUPPORTING 1 FLOOR
40% FOR COLUMNS, BEAMS, GIRDERS AND FOOTINGS SUPPORTING 2 OR MORE FLOORS

C. LATERAL LOADS:

SEISMIC LOAD:

SOIL SITE CLASS - "D" (BASED ON GEOTECHNICAL REPORT PREPARED BY T.L.B. ASSOCIATES, INC. DATED MAY 11, 2006).

S_s = 0.169 G MAPPED SHORT PERIOD RESPONSE SPECTRAL ACCELERATION (BASED ON ADDENDUM TO GEOTECHNICAL REPORT PREPARED BY T.L.B. ASSOCIATES, INC. DATED AUGUST 11, 2006).

S₁ = 0.051 G MAPPED 1S PERIOD RESPONSE SPECTRAL ACCELERATION (BASED ON ADDENDUM TO GEOTECHNICAL REPORT PREPARED BY T.L.B. ASSOCIATES, INC. DATED AUGUST 11, 2006).

F_a = 1.6 SITE CLASS COEFFICIENT FOR SHORT PERIODS (TABLE 1615.12(1))

F_v = 2.4 SITE CLASS COEFFICIENT FOR 1S PERIOD (TABLE 1615.12(2))

C_t = .020 BUILDING PERIOD COEFFICIENT FOR CONCENTRICALLY BRACED STEEL FRAMES.

SEISMIC USE GROUP - II (1604.5)

SEISMIC DESIGN CATEGORY - "B" (TABLE 1616.3 (1), 1616.3(2))

RESPONSE MODIFICATION FACTOR R = 3 FOR "STRUCTURAL STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE" (TABLE 1617.6)

SEISMIC BASE SHEAR IS DETERMINED BASED ON FOLLOWING GRAVITY LOAD PARTICIPATION

a. TOTAL DEAD LOAD

b. 10 PSF PARTITION LOAD

c. OPERATING WEIGHT OF PERMANENT MECHANICAL EQUIPMENT (ELEVATOR EQUIPMENT, MECHANICAL AND ELECTRICAL ROOMS)

SEISMIC REDUNDANCY COEFFICIENT = 1.0 (1617.2.1)

WIND LOAD: (SECTION 1609 IBC 2003, SECTION 6 ASCE 7-02)

MEAN ROOF HEIGHT h_m = 124'-8"

WIND SPEED V = 90 mph

EXPOSURE CATEGORY = "B" (1609.4 IBC 2003)

IMPORTANCE FACTOR I_w = 1.15 (TABLE 1604.5)

D. THE CONTRACTOR IS CAUTIONED THAT THE LIVE LOAD CAPACITIES NOTED ABOVE HAVE BEEN REDUCED FOR MEMBERS SUPPORTING 150 SQUARE FEET OF FLOOR OR MORE IN ACCORDANCE WITH THE PROVISIONS OF THE BUILDING CODE.

E. THE STRUCTURAL DRAWINGS ARE SUPPLEMENTAL TO THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO CHECK AND COORDINATE ALL THE DRAWINGS BEFORE FABRICATION AND/OR INSTALLATION OF ANY WORK. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT OF ANY DISCREPANCIES.

3. GENERAL

A. ALL WORK SPECIFIED HEREIN SHALL BE INSPECTED IN ACCORDANCE WITH THE BUILDING CODE AND ALL LOCAL ORDINANCES. THE OWNER SHALL HIRE AN EXPERIENCED, QUALIFIED INSPECTOR TO PERFORM ALL THE REQUIRED INSPECTION WORK. THE ENGINEER WILL NOT PERFORM THE REQUIRED AS A PART OF THEIR DESIGN SERVICES. THE ENGINEER MAY VISIT THE INSPECTION SITE TO ASCERTAIN GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS. SUCH SITE VISITS ARE NOT TO BE CONSTRUED AS MEETING ANY INSPECTION REQUIREMENTS UNLESS THE ENGINEER SPECIFICALLY SO STATES IN WRITING.

B. ALL STAIRS, RAILINGS, METAL STUD WALLS, STOREFRONTS, AND EXTERIOR CEILINGS SHALL BE DESIGNED FOR ALL APPLICABLE DEAD LOADS AND FOR ALL LIVE, WIND AND SEISMIC LOADS AS SPECIFIED IN THE LOCAL BUILDING CODE. SHOP DRAWINGS AND CALCULATIONS FOR THESE ITEMS SHALL BE SIGNED AND SEALED BY A STRUCTURAL ENGINEER LICENSED IN THE LOCAL JURISDICTION AND SHALL BE SUBMITTED TO THE ARCHITECT.

4. FOUNDATIONS

A. ALL FILL UNDER FOOTINGS AND SLABS SHALL BE COMPAKTED TO A DRY DENSITY OF AT LEAST 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY AASHTO T-180 (ASTM D-1557).

B. ALL EXCAVATION, BACKFILLING, AND FILLING OPERATIONS BENEATH THE BUILDING SLABS AND FOUNDATIONS, AND ALL COMPACTION TESTS AND INSPECTION, SHALL BE DONE UNDER THE DIRECTION AND SUPERVISION OF A REGISTERED PROFESSIONAL SOILS ENGINEER RETAINED BY THE OWNER. ALL SOIL, EQUIPMENT AND PROCEDURES SHALL BE APPROVED BY THE SOILS ENGINEER PRIOR TO ALL EARTHWORK OPERATIONS.

C. PREPARATION OF THE SUBGRADE AND PLACEMENT OF CONTROLLED FILL SHALL BE DONE IN ACCORDANCE WITH THE GEOTECHNICAL REPORT (MAY 11, 2006 WITH AUGUST 11, 2006 ADDENDUM), A COPY OF WHICH IS INCLUDED IN THE SPECIFICATIONS AND WITH WHICH THE CONTRACTOR MUST BE THOROUGHLY FAMILAR.

D. CAISONS ARE DESIGNED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT (MAY 11, 2006 WITH AUGUST 11, 2006 ADDENDUM) BY T.L.B. ASSOCIATES INC., FOR AN ALLOWABLE END BEARING CAPACITY OF 100 KSF, AND SKIN-FRICTION CAPACITY OF 0.5 KSF (BELOW ELEV. 43'). SOIL ABOVE ELEV. 43' IS DISREGARDED FOR SKIN FRICTION. BOTTOM OF CAISON ELEVATIONS SHOWN ON PLAN ARE APPROXIMATE (FOR BIDDING PURPOSES) AND MAY HAVE TO BE ADJUSTED IN THE FIELD. CAISONS SHALL BE DRILLED WITHOUT DISTURBING SURROUNDING SOIL. USE CASINGS OR SHORING TO PREVENT SLIDING OR CAVING FROM SEEPAGE. KEEP EXCAVATIONS FREE OF STORM WATER AND GROUND WATER INFILTRATION. CAISON SHALL BE WITHIN ACI 336.1 TOLERANCES. CONCRETE SHALL BE PLACED IN ACCORDANCE WITH THE LOCAL BUILDING CODE, ACI-318 (LATEST LOCAL APPROVED EDITION), AND THE SPECIFICATIONS.

E. ALL SPREAD FOOTINGS HAVE BEEN DESIGNED FOR AN ASSUMED ALLOWABLE NET SOIL BEARING PRESSURE OF 5000 PSF. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE ASSUMED SOIL BEARING PRESSURE. SHOULD THE SOIL BEARING PRESSURE BE FOUND TO BE LESS THAN 5000 PSF, THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ARCHITECT.

5. CAST-IN-PLACE CONCRETE

5A. GENERAL CONSTRUCTION

A. ALL CONCRETE WORK SHALL CONFORM TO THE LATEST APPROVED (BY LOCAL GOVERNMENT) EDITIONS OF THE FOLLOWING A.C.I. AND A.S.T.M. DOCUMENTS:

ACI-301 SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS

ACI-302.1R FLOOR AND SLAB CONSTRUCTION

ACI-318 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE

ACI-214 COMPRESSION TESTS

ACI-306 COLD WEATHER

ACI-315 DETAILING

ACI-347 FORMWORK

ACI-305 HOT WEATHER

ACI-211 PROPORTIONS OF CONCRETE

ACI-304 PLACING CONCRETE

ASTM C94 READY-MIX CONCRETE

B. ALL FIELD AND LAB TESTING OF CONCRETE SHALL CONFORM TO THE LATEST APPROVED (BY LOCAL GOVERNMENT) EDITIONS OF ASTM:

ASTM C31 FIELD CYLINDER SPECIMENS

ASTM C143 SLUMP TEST

ASTM C231 AIR CONTENT (WHEN REQUIRED)

ASTM C39 LAB TESTING CYLINDERS

ASTM C172 SAMPLING FRESH CONCRETE

ASTM C42 HARDENED CORES (WHEN REQUIRED)

C. ALL CONCRETE, UNLESS NOTED OTHERWISE, SHALL BE STONE AGGREGATE CONCRETE HAVING THE FOLLOWING MINIMUM 28 DAY COMPRESSIVE STRENGTHS:

ALL CONCRETE UNO - 3500 PSI

FOUNDATIONS, FOUNDATION WALLS, GRADE BEAMS, PIERS AND BELOW GRADE WORK, CAISONS - 5000 PSI

CONCRETE ON COMPOSITE METAL DECK - 3500 PSI (LIGHTWEIGHT 110 PCF)

SLABS ON GRADE - 3500 PSI

CONCRETE AT ELEVATOR MACHINE ROOM - 5000 PSI (LIGHTWEIGHT 110 PCF)

D. ALL CONCRETE EXPOSED TO WEATHER SHALL HAVE AN AIR ENTRAINMENT OF 6% ± 1.5%. UNLESS NOTED OTHERWISE, NO ADMIXTURES CONTAINING CALCIUM CHLORIDE SHALL BE PERMITTED. MAXIMUM AGGREGATE SIZE FOR CONCRETE SHALL BE 3/4" UNO, 1" FOR FOUNDATIONS, FOUNDATION/BASEMENT WALLS, PIERS AND BELOW GRADE WORK. ALL CONCRETE SHALL CONTAIN A WATER REDUCING ADMIXTURE.

E. ALL CONCRETE MIX DESIGNS AND ADMIXTURES SHALL BE APPROVED BY THE ARCHITECT 30 DAYS PRIOR TO INITIATION OF FIRST POUR.

F. ALL REINFORCING BARS SHALL CONFORM TO ASTM A-615 GRADE 60. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, FLAT SHEETS.

G. ALL CONCRETE SHALL BE SAMPLED AND TESTED BY AN AGENCY RETAINED BY THE OWNER. THE CONTRACTOR SHALL NOTIFY THE TESTING AGENCY 48 HOURS PRIOR TO THE POURING OF ANY CONCRETE.

H. PROVIDE WATER STOPS IN ALL CONSTRUCTION AND CONTROL JOINTS IN CONCRETE BELOW GRADE.

I. CONCRETE EXPOSED TO PUBLIC VIEW SHALL MEET THE REQUIREMENTS FOR ARCHITECTURAL CONCRETE OF ACI 301.

J. ALL FORMWORK SHALL BE IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE "FORMWORK FOR CONCRETE", SPECIAL PUBLICATION NO. 4 AND ACI'S "STANDARD RECOMMENDED PRACTICE FOR CONCRETE FORMWORK" (ACI-347, LATEST LOCAL APPROVED EDITION).

K. ALL CONCRETE FILL FOR METAL PIANO STAIRS SHALL BE PEA GRAVEL CONCRETE, HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3500 PSI AT 28 DAYS. MAXIMUM SIZE OF AGGREGATE SHALL BE 1/2".

L. LEAVE-IN-PLACE CONCRETE FORMS SHALL BE "STAY FORM" FORM LINER BY A.M.I.C.O. OR AN APPROVED EQUIVALENT. FORM TO BE 26 GAGE MIN. THICKNESS FABRICATED IN ACCORDANCE WITH ASTM C847.

3. CLEARANCES FOR REINFORCING:

CAISON REINFORCING: 3" CLR.

CAST AGAINST EARTH: 3" CLR.

FORMED & EXPOSED TO EARTH OR WEATHER: #5 OR LARGER 2" CLR.

#5 OR SMALLER 1 1/2" CLR.

CONCRETE S.O.G. (TOP): 2" CLR.

NOT EXPOSED TO EARTH OR WEATHER:

SLABS & WALLS: 3/4" CLR.

CONCRETE BEAM: 1 1/2" CLR.

COLUMN SPIRALS OR TIES: 1 1/2" CLR.

CONCRETE S.O.G. (TOP): 2" CLR.

5B. SLABS ON GRADE

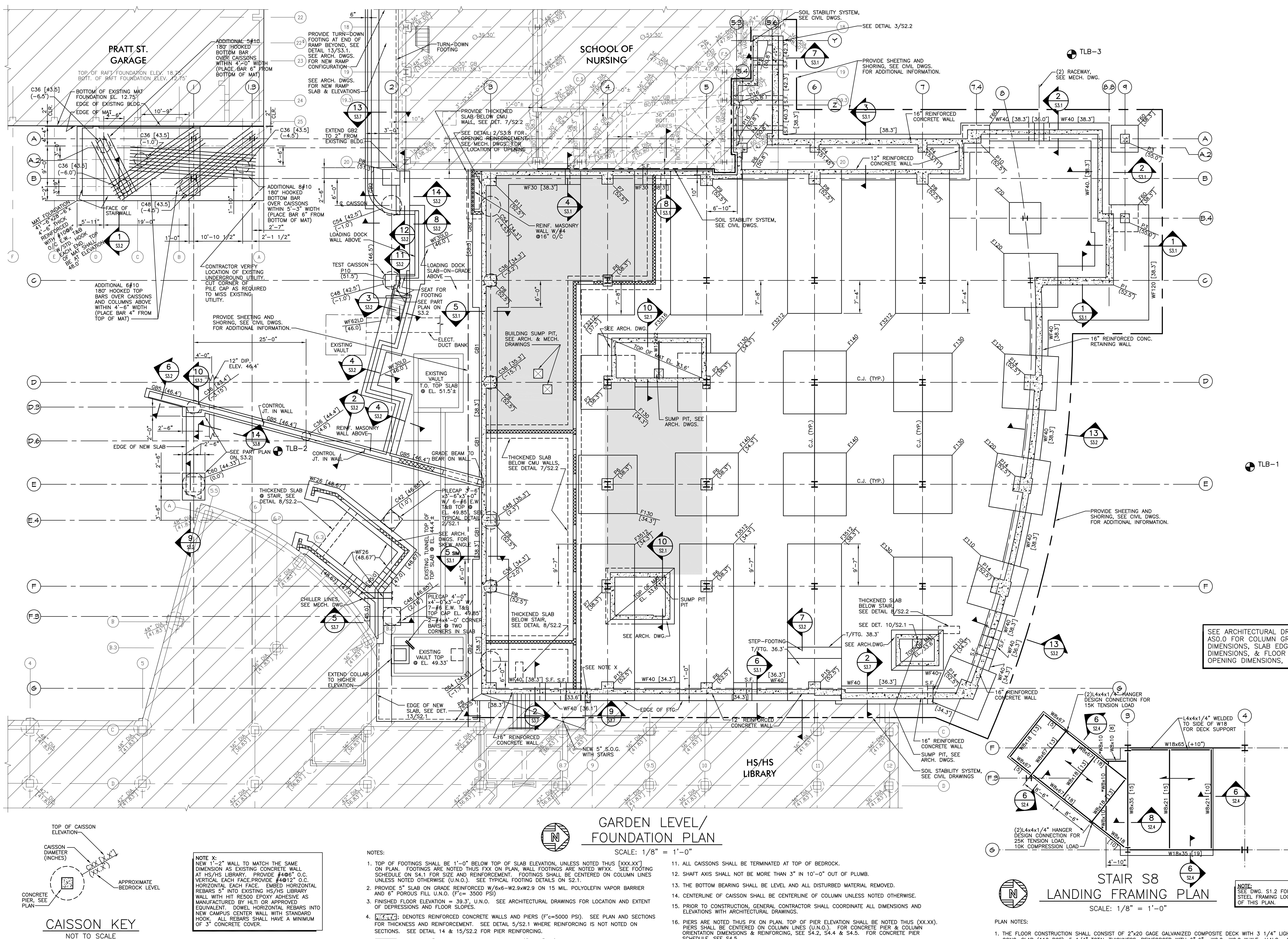
A. ALL SLABS ON GRADE SHALL CONSIST OF A 5" THICK CONCRETE SLAB UNO, REINFORCED WITH 6" X 6" W2.9 X W2.9 WELDED WIRE MESH ON 15 MIL POLYOLEFIN VAPOR BARRIER ON TOP OF 6" POROUS FILL, U.N.O.

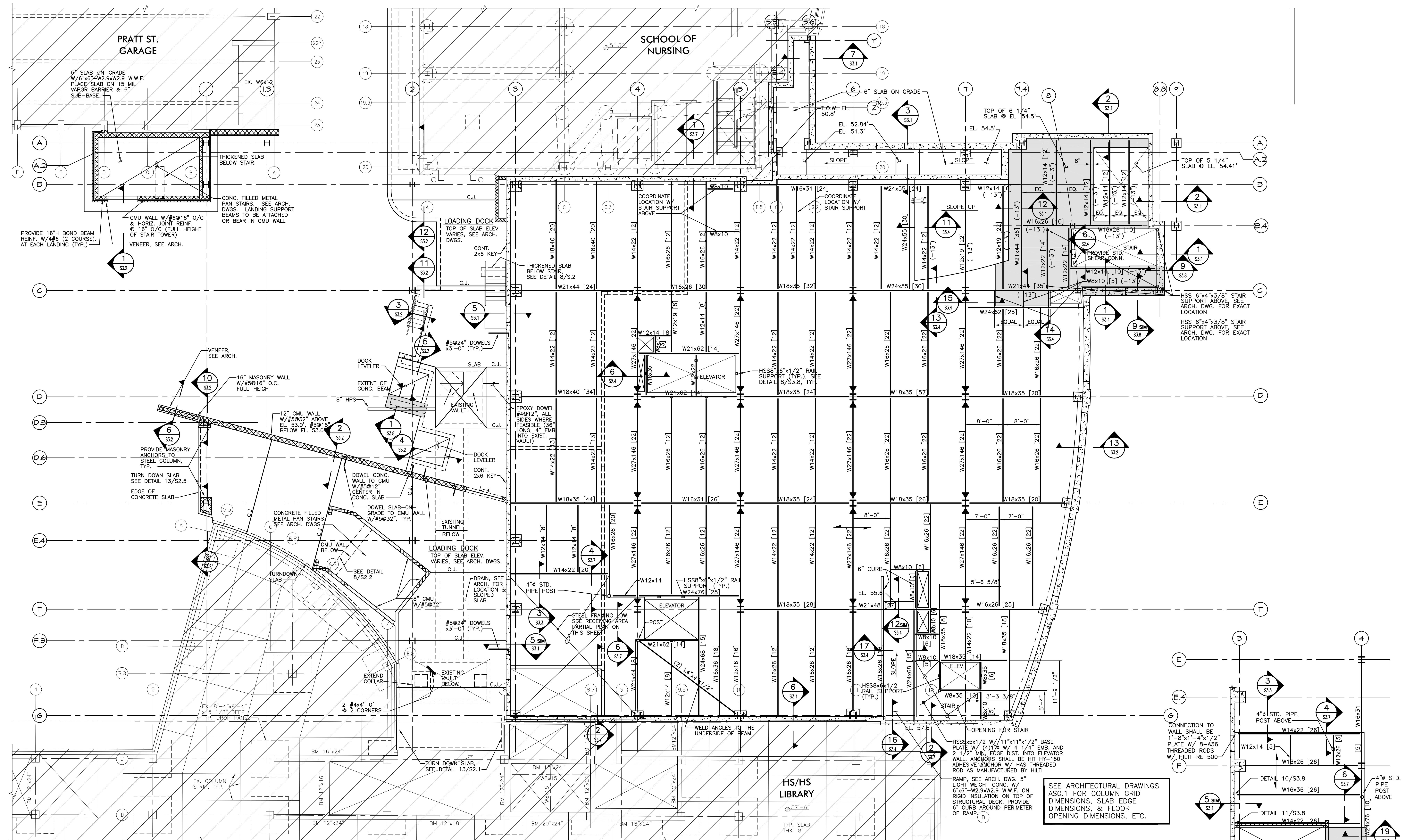
B. CONTROL JOINTS FOR SLABS ON GRADE SHALL BE PROVIDED AT COLUMN CENTERLINE AND MORE FREQUENTLY SUCH THAT THE AREA OF THE SLAB DOES NOT EXCEED 625 SQUARE FEET BETWEEN JOINTS. CONTROL JOINT PANELS SHALL BE RECTANGULAR TYPICALLY IN SHAPE AND SHALL NOT EXCEED A LENGTH-TO-WIDTH RATIO OF 1.5. COORDINATE JOINTS WITHIN CORRIDOR AREAS WITH TILE FINISHES, SEE ARCHITECTURAL DRAWINGS. PROVIDE PROPOSED JOINT LAYOUT FOR REVIEW.

5C. REINFORCING STEEL

A. EXCEPT AS NOTED, ALL REINFORCING SHALL BE HIGH STRENGTH NEW BILLET STEEL CONFORMING TO ASTM DESIGNATION A-615 (LATEST LOCAL APPROVED EDITION) (FY=60,000 PSI). ALL REINFORCING SHALL BE DETAILED, FABRICATED, AND PLACED IN ACCORDANCE WITH THE ACI'S MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES (ACI-315).

5D.





FIRST
FLOOR FRAMING PLAN

SCALE: 1/8" = 1'-0"

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3 01/29/10 RECORD DRAWINGS
 2 05/02/07 BULLETIN #10
 1 04/09/07 BULLETIN #9
 0 02/16/07 ISSUE FOR BID
 Rev. Date: Comment:
 Issued: FEBRUARY 16, 2007

Campus
Center

West Lombard Street
University of Maryland, Baltimore
Baltimore, MD



SEE ARCHITECTURAL DRAWINGS
 ASO.1 FOR COLUMN GRID
 DIMENSIONS, SLAB EDGE
 DIMENSIONS, & FLOOR
 OPENING DIMENSIONS, ETC.

RECEIVING AREA
PARTIAL PLAN

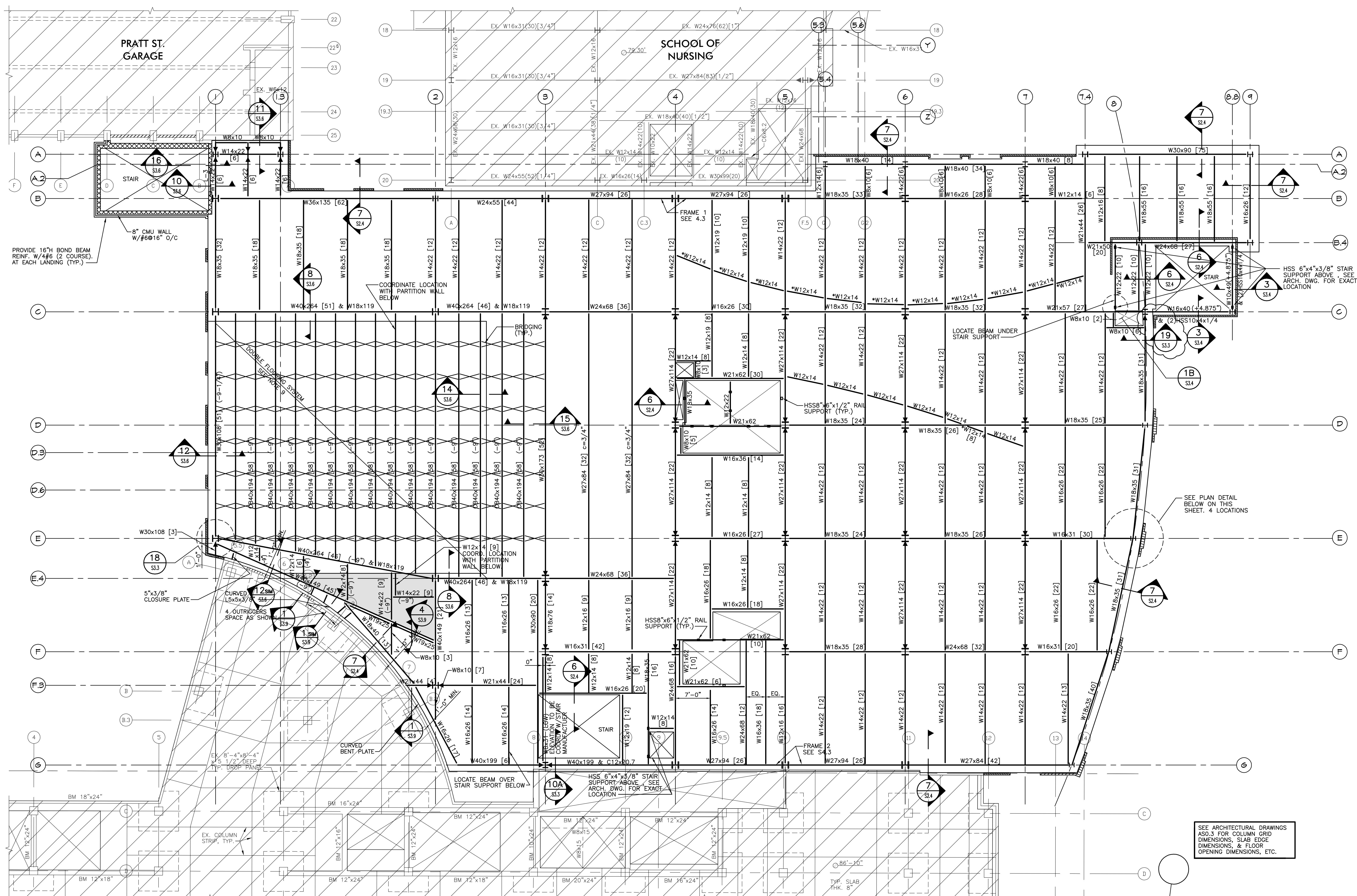
SCALE: 1/8" = 1'-0"

NOTES:

1. THE FLOOR CONSTRUCTION SHALL CONSIST OF 2"x20 GAGE GALVANIZED COMPOSITE DECK WITH 3 1/4" THICKNESS LIGHTWEIGHT CONCRETE SLAB (110 PCF) (TOTAL THICKNESS 5 1/4") REINFORCED WITH 6"x6" -W.2.9xW.2.9 W.W.F. TOP OF STEEL ELEVATION IS 5 1/4" BELOW TOP OF SLAB UNLESS NOTED ± FROM THIS ELEVATION.
2. [X]: INDICATES NUMBER OF 3/4" DIAMETER x 3 1/2" LONG HEADED STUDS WELDED TO TOP FLANGE OF STEEL BEAM EVENLY SPACED ALONG LENGTH. PROVIDE STUDS @ 12" O/C MINIMUM FOR ALL BEAMS U.N.O.
3. —: INDICATES DIRECTION OF SPAN OF FLOOR DECK.
4. C-XXX: INDICATES AMOUNT OF CAMBER.
5. SEE ARCHITECTURAL DRAWINGS FOR ALL ELEVATIONS AND DIMENSIONS NOT SHOWN.
6. ALL BEAMS ARE SPACED EQUALLY BETWEEN COLUMN LINES UNLESS NOTED.
7. RECEIVING AREA ELEVATION SHALL BE 50.85". TOP OF STEEL ELEVATION SHALL BE 5 1/4" BELOW FINISHED FLOOR ELEVATION @ EL. 50.41", UNLESS NOTED (TYP. "X").
8. []: INDICATES EXTENT OF 5 1/4" LONG HEADED STUDS WELDED TO TOP FLANGE OF STEEL BEAM EVENLY SPACED ALONG LENGTH. PROVIDE STUDS @ 12" O/C MINIMUM FOR ALL BEAMS U.N.O.
9. —: INDICATES SPAN OF ONE WAY SLAB.

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CONSTRUCTION
DOCUMENTS

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 WTW PROJECT NO. 703-4091
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THIRD FLOOR FRAMING PLAN

NOTES:

1. THE FLOOR CONSTRUCTION SHALL CONSIST OF 2"X20 GAGE GALVANIZED COMPOSITE DECK WITH 5 1/4" TOTAL THICKNESS LIGHTWEIGHT CONCRETE SLAB (10 PCF) REINFORCED WITH 6"X6"-W2.9XW2.9 W.W.F. SEE DETAILS ON S2.4 FOR TYPICAL COMPOSITE CONSTRUCTION.
2. [X] INDICATES NUMBER OF 3/4" DIAMETER X 3 1/2" LONG HEADED STUDS WELDED TO TOP FLANGE OF STEEL BEAM EVENLY SPACED ALONG LENGTH. PROVIDE STUDS @ 12" O/C MINIMUM FOR ALL BEAMS U.N.O.
3. —— ; INDICATES DIRECTION OF SPAN OF FLOOR DECK.
4. —— ; INDICATES BEAM-TO-COLUMN MOMENT CONNECTION, —— ; INDICATES BEAM-TO-BEAM MOMENT CONNECTION, SEE DETAIL 13/S2.4.
5. C-XXX; INDICATES AMOUNT OF CAMBER.
6. SEE ARCHITECTURAL DRAWINGS FOR ALL ELEVATIONS AND DIMENSIONS NOT SHOWN.
7. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMN LINES UNLESS NOTED.
8. THIRD FLOOR ELEVATION SHALL BE 93.5'. TOP OF STEEL ELEVATION SHALL BE 5 1/4" BELOW FINISH FLOOR ELEVATION, UNLESS NOTED (±X"). (±X), DENOTES STEEL BEAM ELEVATION DEPRESSED BELOW TOP OF BEAM.
9. SEE AS.3 FOR EXACT EXTENT OF DOUBLE FLOOR SYSTEM.
10. CB; INDICATES CASTELLATED BEAM.
11. *; LOCATE BEAM FOR OVERHEAD COOLING SHUTTER OR OVERHEAD COOLING GRILL CONNECTION, SEE ARCH. DWG. FOR LOCATION.
12. LOCATION OF BEAMS ADJACENT TO SLAB OPENING SHALL BE 5" FROM THE CENTERLINE OF BEAM TO EDGE OF SLAB, U.N.O. SEE DETAIL 6/S.4. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF OPENINGS.
13. SEE S4.1 FOR LIGHT-GAGE METAL WALL STUD SCHEDULE, SEE ARCH. DWGS. FOR DEPTH OF ALL STUDS.
14. ■, THICKENED SLAB.

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 2 05/02/07 BULLETIN #10
 1 04/09/07 BULLETIN #9
 0 02/16/07 ISSUE FOR BID
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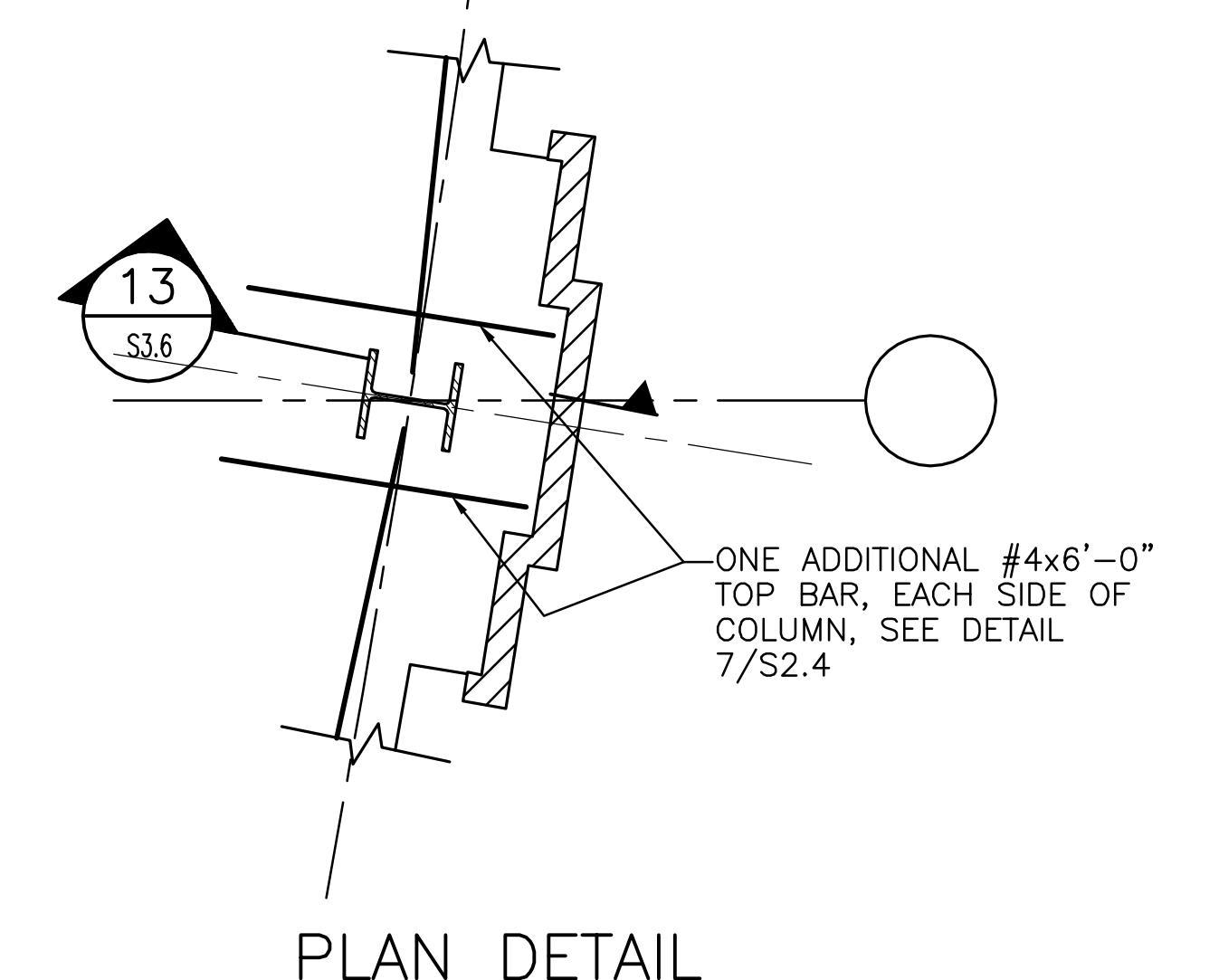
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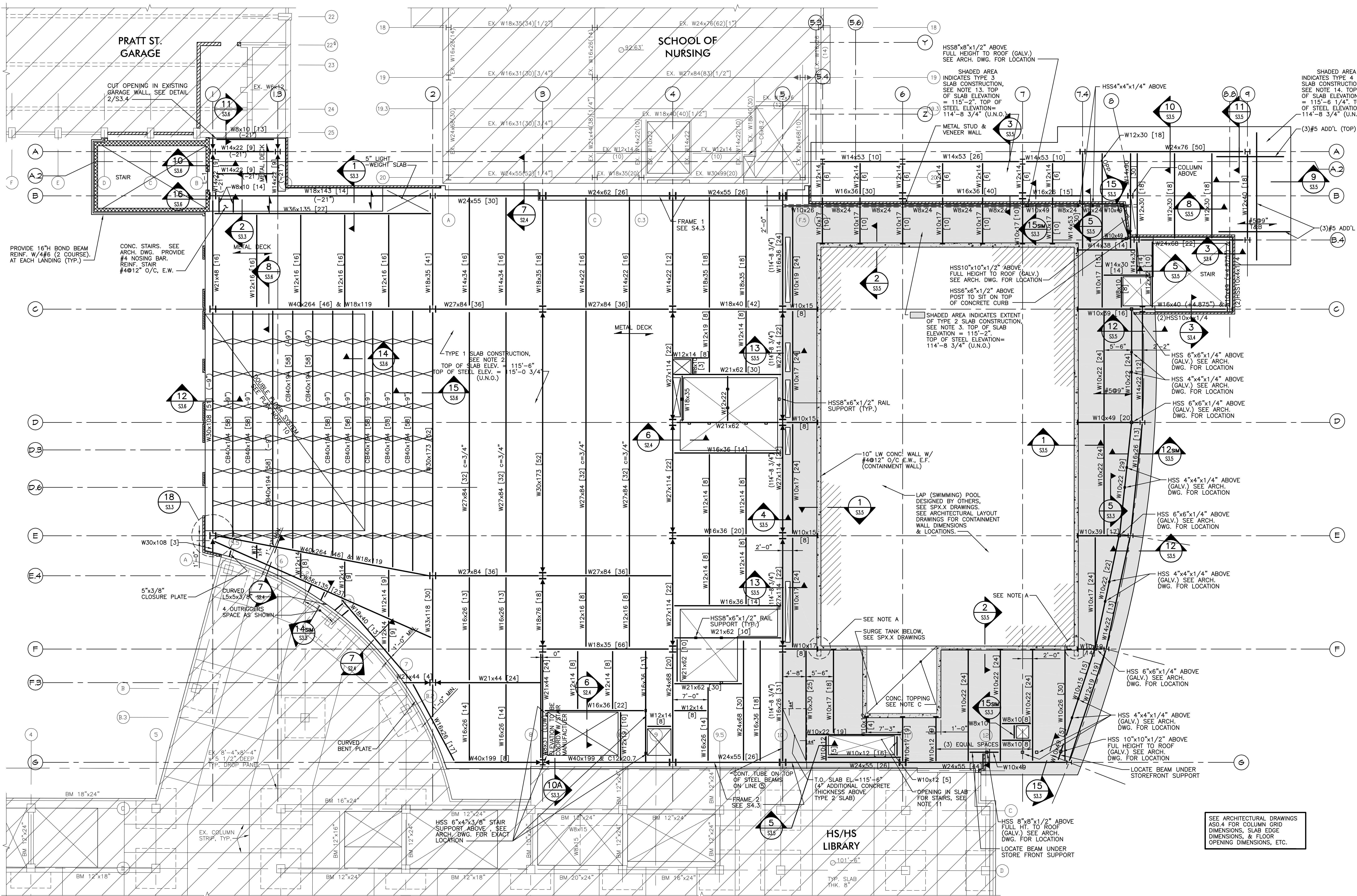


Third Floor Framing Plan

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PLAN DETAIL



FOURTH FLOOR FRAMING PLAN

NOTES

1. FOURTH FLOOR ELEVATION SHALL BE 115.5". TOP OF STEEL ELEVATION VARIES, NOTED ON PLAN THIS ("XXX"-X").
2. TYPE 1 SLAB CONSTRUCTION SHALL CONSIST OF 2" GAGE GALVANIZED COMPOSITE METAL DECK W/ 3/4" LIGHTWEIGHT CONCRETE (110 PSF) SLAB ABOVE (5 1/2" TOTAL THICKNESS), REINFORCE SLAB W/ 6" X 2.9 W.W.F. TOP OF SLAB ELEVATION IS AS NOTED ON PLAN. STEEL BEAMS BELOW SHALL BE SPRAY FIREPROOFED. SEE DETAILS ON S.24 FOR TYPICAL COMPOSITE CONSTRUCTION.
3. TYPE 2 SLAB CONSTRUCTION SHALL CONSIST OF A FORMED 5 1/4" LIGHTWEIGHT CONCRETE SLAB W/ #509" N/S & #4912" E/W, CENTERED IN SLAB, N/S BARS ON BOTTOM. TOP OF SLAB ELEVATION IS AS NOTED ON PLAN. STEEL BEAMS BELOW SHALL BE ENCASED IN CONCRETE, SEE DETAIL 6/S.33.
4. [X] INDICATES NUMBER OF 3/4" DIAMETER HEADED STUDS (3 1/2" LONG FOR TYPE 1 SLAB, 4 1/2" LONG FOR TYPE 2 SLAB) WHICH WILL BE SPANNED BY LENGTH OF STEEL BEAM EVENLY SPACED ALONG LENGTH. PROVIDE STUDS @ 12" O/C MINIMUM FOR ALL BEAMS U.N.O.
5. — : INDICATES DIRECTION OF SPAN OF METAL DECK, OR DIRECTION OF MAIN BARS IN FORMED SLAB.
6. — : INDICATES BEAM-TO-COLUMN MOMENT CONNECTION. — : INDICATES BEAM-TO-BEAM MOMENT CONNECTION, SEE DETAIL 13/S.24.
7. C-XXX": INDICATES AMOUNT OF CANTER.
8. SEE ARCHITECTURAL DRAWINGS FOR ALL ELEVATIONS AND DIMENSIONS NOT SHOWN.
9. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMN LINES UNLESS NOTED.
10. SEE ALSO FOR EXACT EXTENT OF DOUBLE FLOOR SYSTEM.
11. SEE ALSO FOR METAL PAN STAIRS W/ STEEL CHANNEL STRINGERS, STRUCTURALLY DESIGNED BY THE CONTRACTOR. SEE ARCH. DRAWINGS FOR DIMENSIONS.
12. CB: INDICATES CASTELLATED BEAM.
13. TYPE 3 SLAB CONSTRUCTION SHALL CONSIST OF A FORMED 5 1/4" LIGHTWEIGHT CONCRETE SLAB W/ #509" EACH WAY CENTERED IN SLAB, N/S BARS ON TOP. TOP OF SLAB ELEVATION IS AS NOTED ON PLAN. STEEL BEAMS BELOW SHALL BE SPRAY FIREPROOFED.
14. TYPE 4 SLAB CONSTRUCTION SHALL CONSIST OF A FORMED 9 1/2" LIGHTWEIGHT CONCRETE SLAB W/ #509" EACH WAY, TOP OF SLAB ELEVATION #509" EACH WAY. STEEL BEAMS SHALL BE SPRAY FIREPROOFED.
15. LOCATION OF BEAMS ADJACENT TO SLAB OPENINGS SHALL BE 5' FROM THE CENTERLINE OF BEAM TO EDGE OF SLAB, U.N.O. SEE DETAIL 6/S.14. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF OPENINGS.
16. SEE S.4.1 FOR LIGHT-GAGE METAL WALL STUD SCHEDULE, SEE ARCH. DWGS. FOR DEPTH OF ALL STUDS.

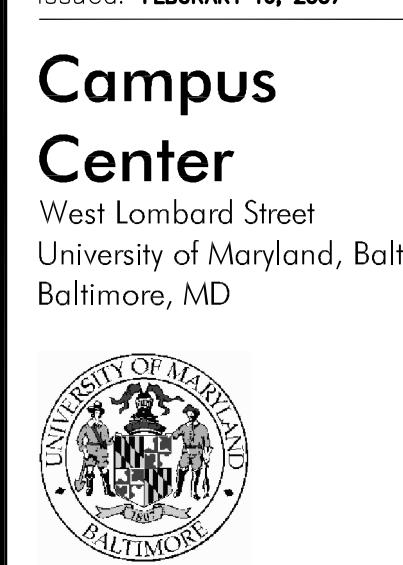
SCALE: 1/8" = 1'-0"

NOTE A: CONCRETE CORBEL, (2) TOTAL.
 SEE DETAIL 7/S.33.

NOTE B: PROVIDE SCHEDULE 40 PIPE SLEEVE FOR FLOOR DRAINS. COORDINATE SIZE & LOCATIONS W/ MECHANICAL AND ARCHITECTURAL DRAWINGS.

NOTE C: TOP SLAB OF TANK TO BE DESIGNED FOR ADDITIONAL 100 PSF LL + 50 PSF DL IN ADDITION TO SELFWEIGHT AND MISCELLANEOUS TANK LOADS.

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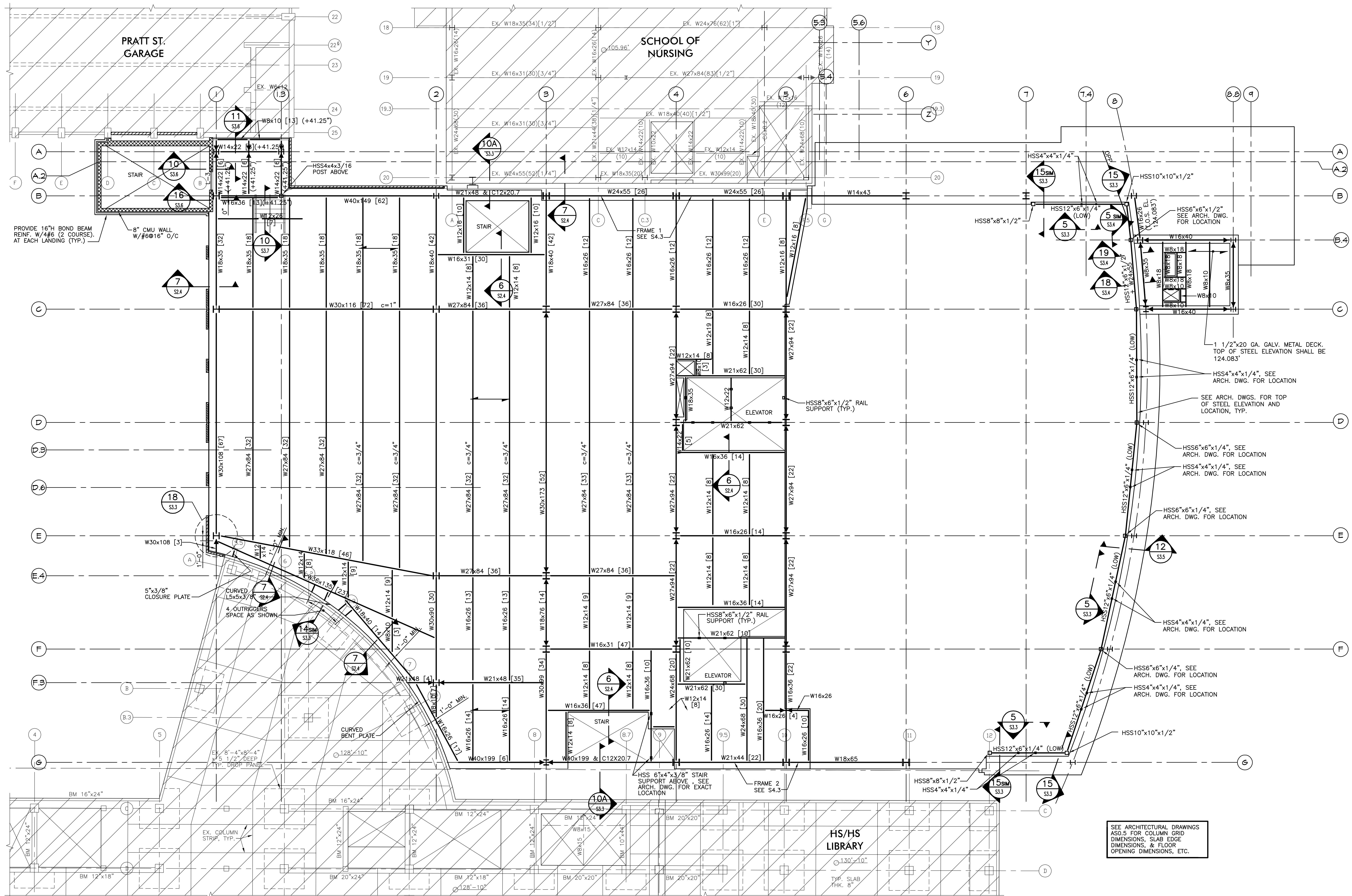


Fourth Floor Framing Plan

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IF THE DRAWING DOES NOT MEASURE EXACTLY 30" X 42", IT HAS BEEN REDUCED OR ENLARGED. PLEASE DO NOT SCALE THIS DRAWING.



FIFTH
FLOOR FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES:

1. THE FLOOR CONSTRUCTION SHALL CONSIST OF 2"X20 GAGE GALVANIZED COMPOSITE DECK WITH 5 1/4" TOTAL THICKNESS LIGHTWEIGHT CONCRETE SLAB (110 PCF) REINFORCED WITH 6"X6"- W2.9X W2.9 W.W.F. SEE DETAILS ON S2.4 FOR TYPICAL COMPOSITE CONSTRUCTION.
2. [X] : INDICATES NUMBER OF 3/4" DIAMETER x 3 1/2" LONG HEADED STUDS WELDED TO TOP FLANGE OF STEEL BEAM EVENLY SPACED ALONG LENGTH. PROVIDE STUDS @ 12" O/C MINIMUM FOR ALL BEAMS U.N.O.
3. : INDICATES DIRECTION OF SPAN OF FLOOR DECK.
4. : INDICATES BEAM-TO-COLUMN MOMENT CONNECTION, : INDICATES BEAM-TO-BEAM MOMENT CONNECTION, SEE DETAIL 13/S2.4.
5. C-XXX": INDICATES AMOUNT OF CAMBER.
6. SEE ARCHITECTURAL DRAWINGS FOR ALL ELEVATIONS AND DIMENSIONS NOT SHOWN.
7. ALL BEAMS ARE SPACED EVENLY BETWEEN COLUMN LINES UNLESS NOTED.
8. THE FLOOR ELEVATION SHALL BE 129.83". TOP OF STEEL ELEVATION SHALL BE 5 1/4" BELOW FINISH FLOOR ELEVATION, UNLESS NOTED. THIS (H.E.) DENOTES STEEL BEAM ELEVATION DEPRESSED BELOW OR ABOVE TOP OF BEAM.
9. CB: INDICATES CASTELLATED BEAM.
10. LOCATION OF BEAMS ADJACENT TO SLAB OPENING SHALL BE 5" FROM THE CENTERLINE OF BEAM TO EDGE OF SLAB, U.N.O. SEE DETAIL 6/S.4. SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF OPENINGS.
11. SEE S4.1 FOR LIGHT-GAGE METAL WALL STUD SCHEDULE, SEE ARCH. DWGS. FOR DEPTH OF ALL STUDS.

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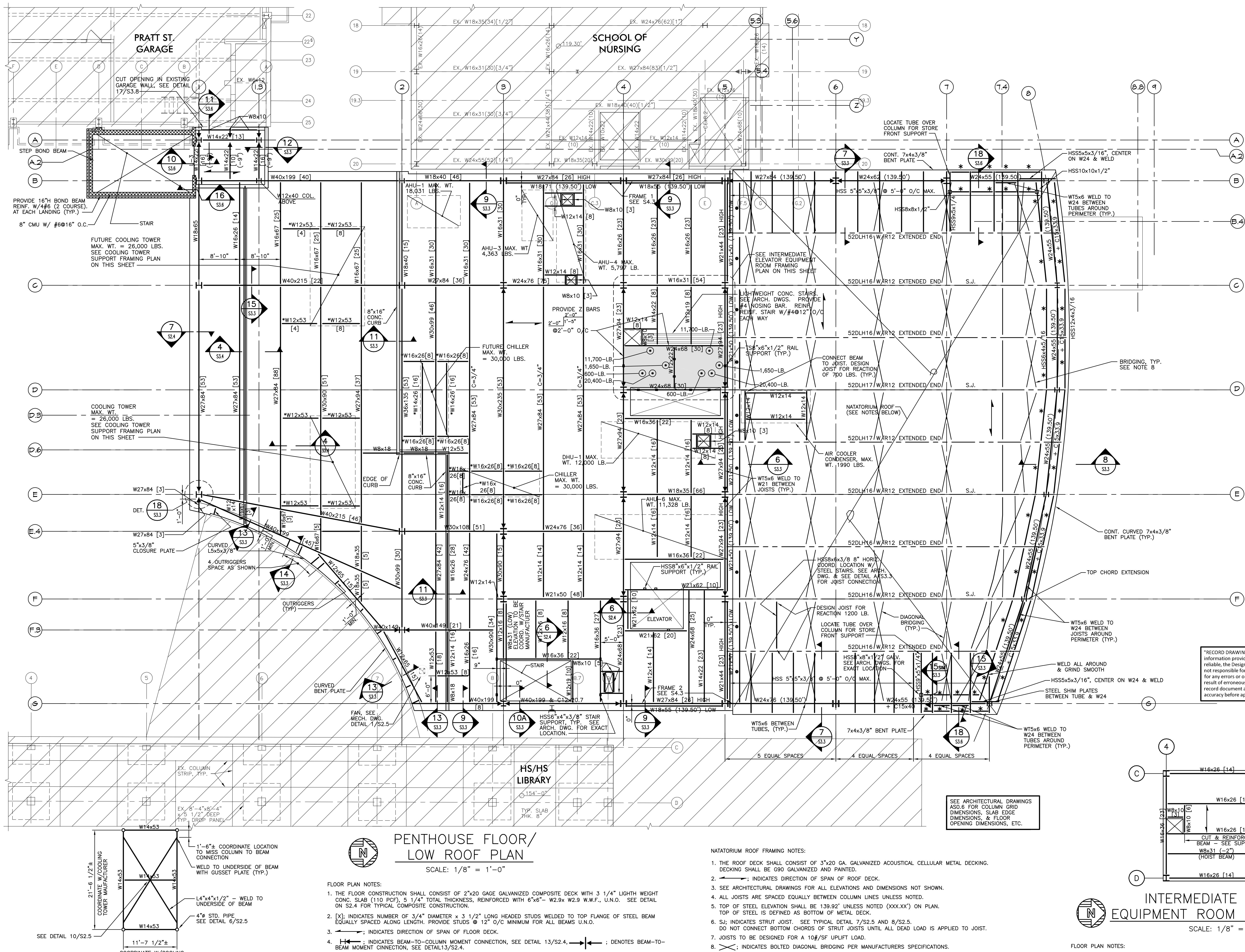
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Fifth Floor Framing Plan

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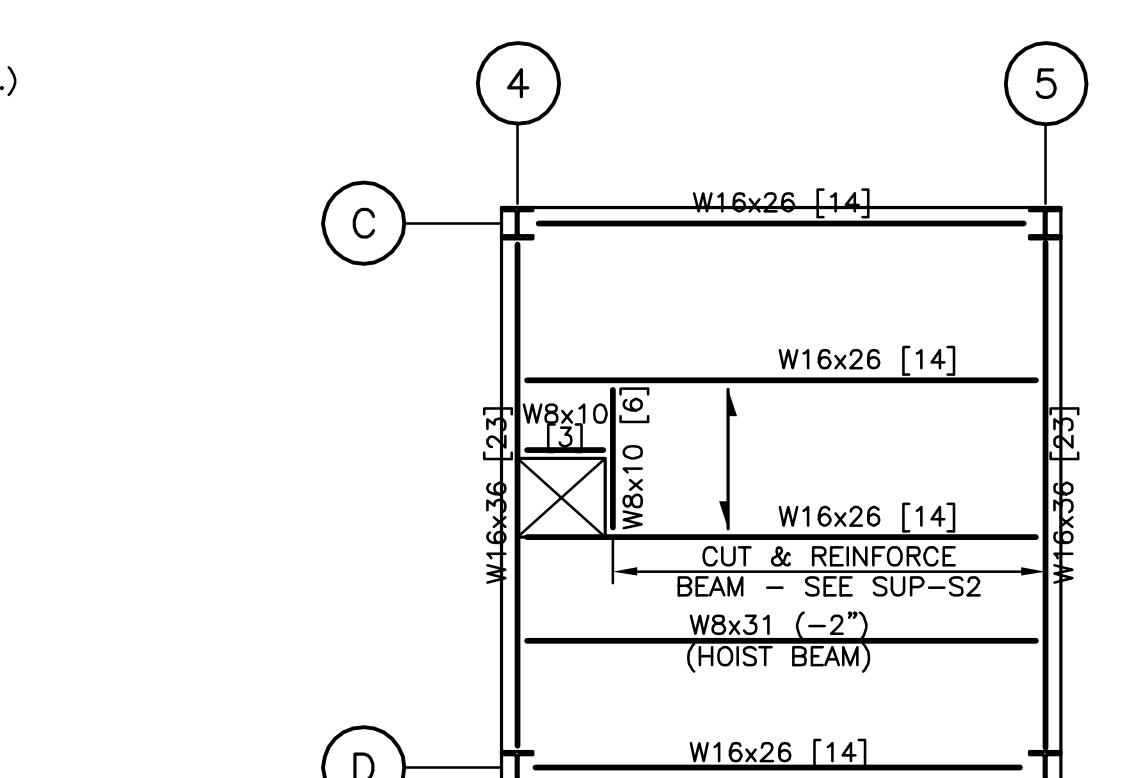
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Penthouse Floor/Low Roof Plan



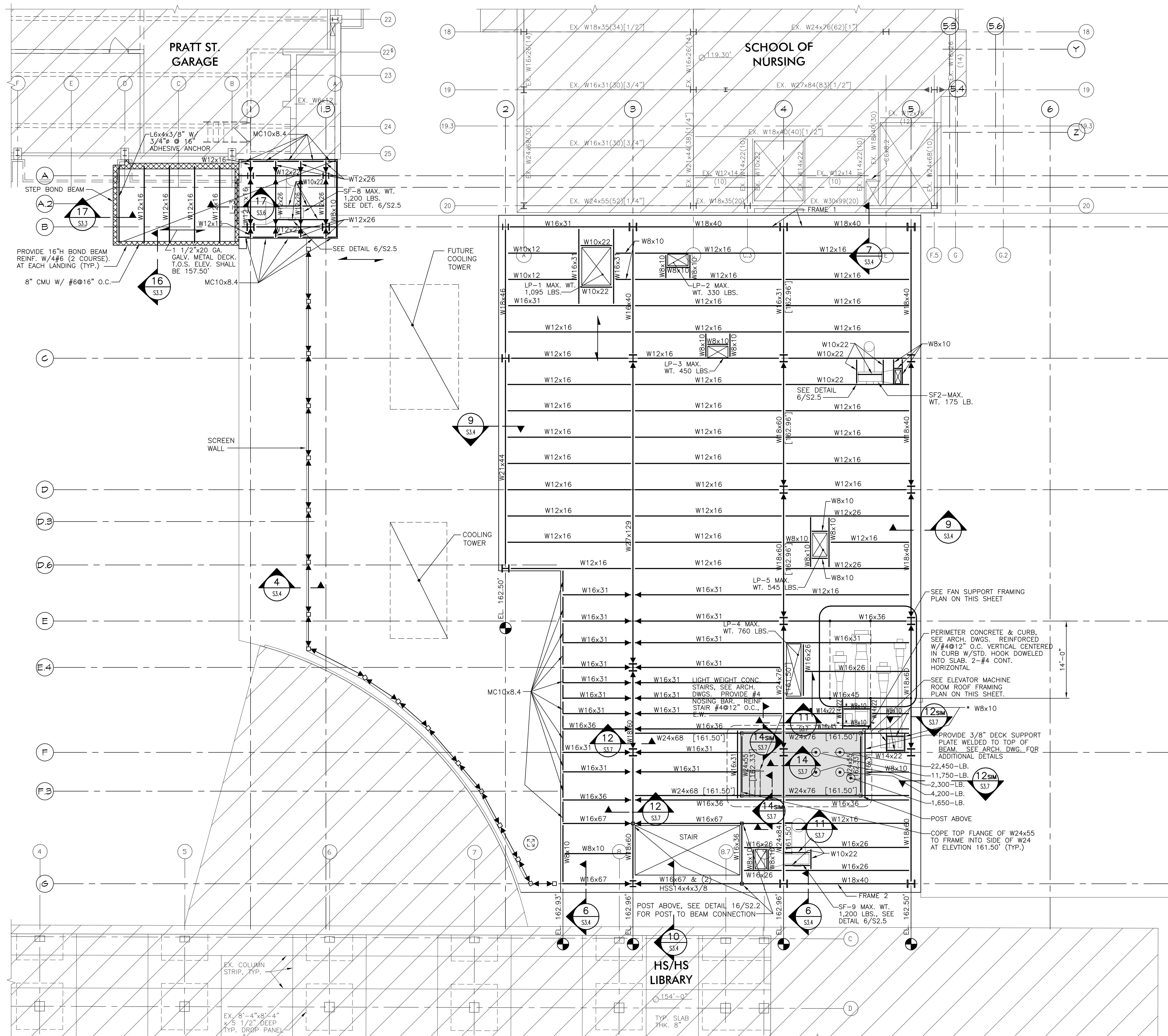
INTERMEDIATE ELEVATOR EQUIPMENT ROOM FRAMING PLAN
 SCALE: 1/8" = 1'-0"

FLOOR PLAN NOTES:

1. THE FLOOR CONSTRUCTION SHALL CONSIST OF 2"X20 GAGE GALVANIZED COMPOSITE DECK WITH 3 1/4" LIGHTWEIGHT CONC. SLAB (110 PCF), 5 1/4" TOTAL THICKNESS, REINFORCED WITH 6"x6" - W2.9xW.9, U.N.O. SEE DETAIL ON S2.4 FOR TYPICAL COMPOSITE CONSTRUCTION.
2. [X] INDICATES NUMBER OF 3/4" DIAMETER X 1/2" LONG HEADED STUDS WELDED TO TOP FLANGE OF STEEL BEAM EQUALLY SPACED ALONG LENGTH. PROVIDE STUDS @ 12" O/C MINIMUM FOR ALL BEAMS U.N.O.
3. → ; INDICATES DIRECTION OF SPAN OF FLOOR DECK.
4. INTERMEDIATE ELEVATOR EQUIPMENT ROOM ELEVATION SHALL BE 155.19'. TOP OF STEEL ELEVATION SHALL BE 5 1/4" BELOW FLOOR ELEVATION. (±x), DENOTES STEEL BEAM ELEVATION DEPRESSED BELOW BOTTOM OF DECK.
5. LIVE LOAD IN THIS AREA DESIGNED FOR 50psf.

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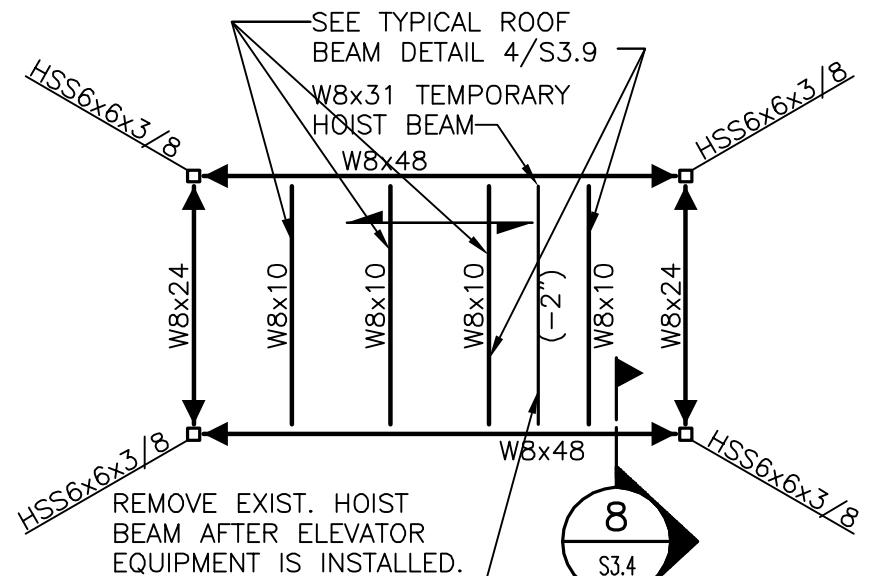
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ELEVATOR MACHINE ROOM ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES:
 1. THE ROOF DECK SHALL CONSIST OF 1 1/2" x 20 GA. GALVANIZED METAL DECKING, U.N.O.
 2. → : INDICATES DIRECTION OF SPAN OF ROOF DECK.
 3. TOP OF STEEL ELEVATION SHALL BE 171.42'. (+X) DENOTES STEEL BEAM ELEVATION DEPRESSED BELOW TOP OF STEEL ELEVATION.
 4. □ : INDICATES BEAM-TO-HOLLOW-STEEL SECTION MOMENT CONNECTION, SEE SECTION 8/S3.4.



ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

NOTES:
 1. THE ROOF DECK SHALL CONSIST OF 1 1/2" x 20 GA. GALVANIZED METAL DECKING, U.N.O.
 2. → : INDICATES DIRECTION OF SPAN OF ROOF DECK.
 3. → : INDICATES BEAM-TO-COLUMN MOMENT CONNECTION.
 4. → : INDICATES BEAM-TO-BEAM MOMENT CONNECTION, SEE DETAIL 13/S2.4.
 5. SEE ARCHITECTURAL DRAWINGS FOR ALL ELEVATIONS AND DIMENSIONS NOT SHOWN.
 6. ALL BEAMS ARE SPACED EQUALLY BETWEEN COLUMN LINES UNLESS NOTED.
 7. BOTTOM OF DECK ELEVATION IS NOTED AS EL. XXX.XX ON PLAN.
 8. [XXX.XX] DENOTES TOP OF STEEL BEAM ELEVATION.
 9. □ : INDICATES 13" THK. FORMED LIGHT-WEIGHT CONCRETE SLAB (5000 PSI) WITH #6@6" O.C. T&B EAST/WEST DIRECTION, #6@12" O.C. T&B NORTH/SOUTH DIRECTION. XXX-LB. DENOTES ELEVATOR LOADS.
 10. LOCATION OF BEAMS ADJACENT TO SLAB OPENING SHALL BE 5" FROM THE CENTERLINE OF BEAM TO EDGE OF SLAB, U.N.O., SEE DETAIL 6/S4. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATION OF OPENINGS.
 11. SEE S4.1 FOR LIGHT-GAGE METAL WALL STUD SCHEDULE, SEE ARCH. DWGS. FOR DEPTH OF ALL STUDS.
 12. * : INDICATES COORDINATE LOCATION OF BEAM WITH CURB. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS.

STAIR S8 ROOF FRAMING PLAN

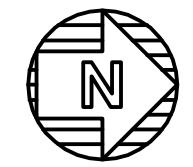
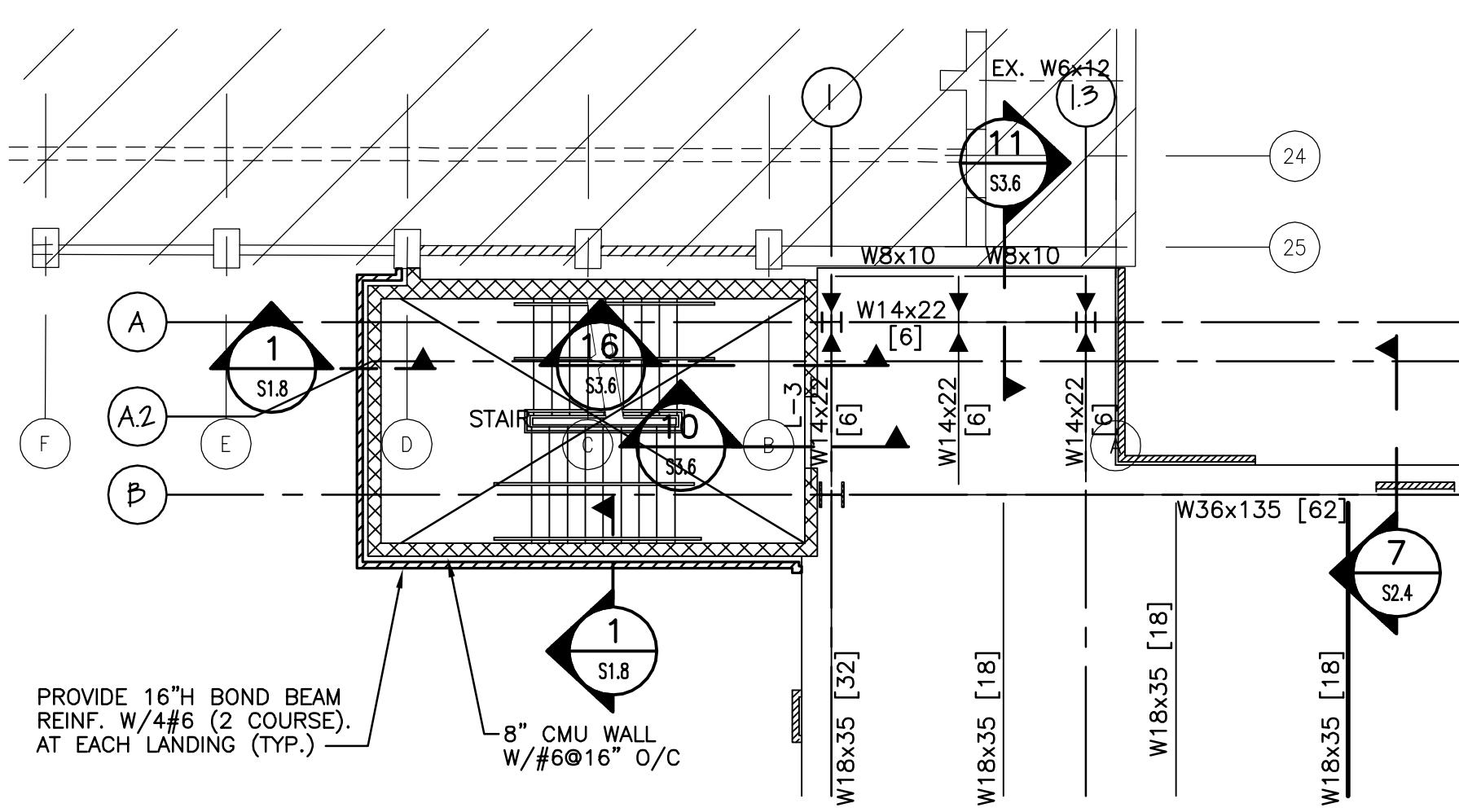
SCALE: 1/8" = 1'-0"

NOTES:
 1. THE ROOF DECK SHALL CONSIST OF 1 1/2" x 20 GA. GALVANIZED METAL DECKING, U.N.O.
 2. → : INDICATES DIRECTION OF SPAN OF ROOF DECK.
 3. TOP OF STEEL ELEVATION SHALL BE 173.0'.
 4. → : INDICATES BEAM-TO-COLUMN MOMENT CONNECTION, SEE DETAIL 16/S2.4.
 5. → : INDICATES BEAM-TO-HOLLOW-STEEL SECTION MOMENT CONNECTION, SEE SECTION 8/S3.4.

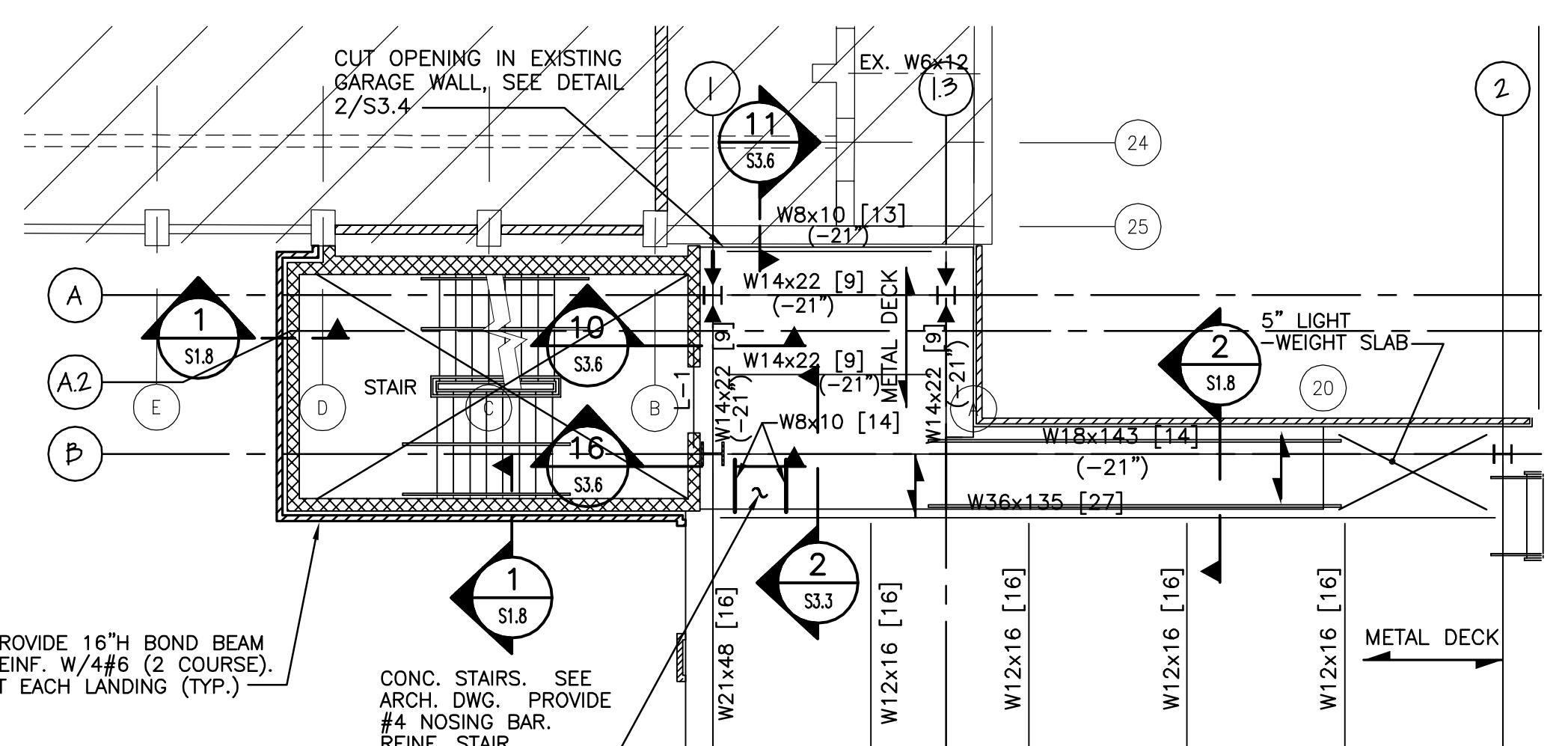
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Roof Framing Plan

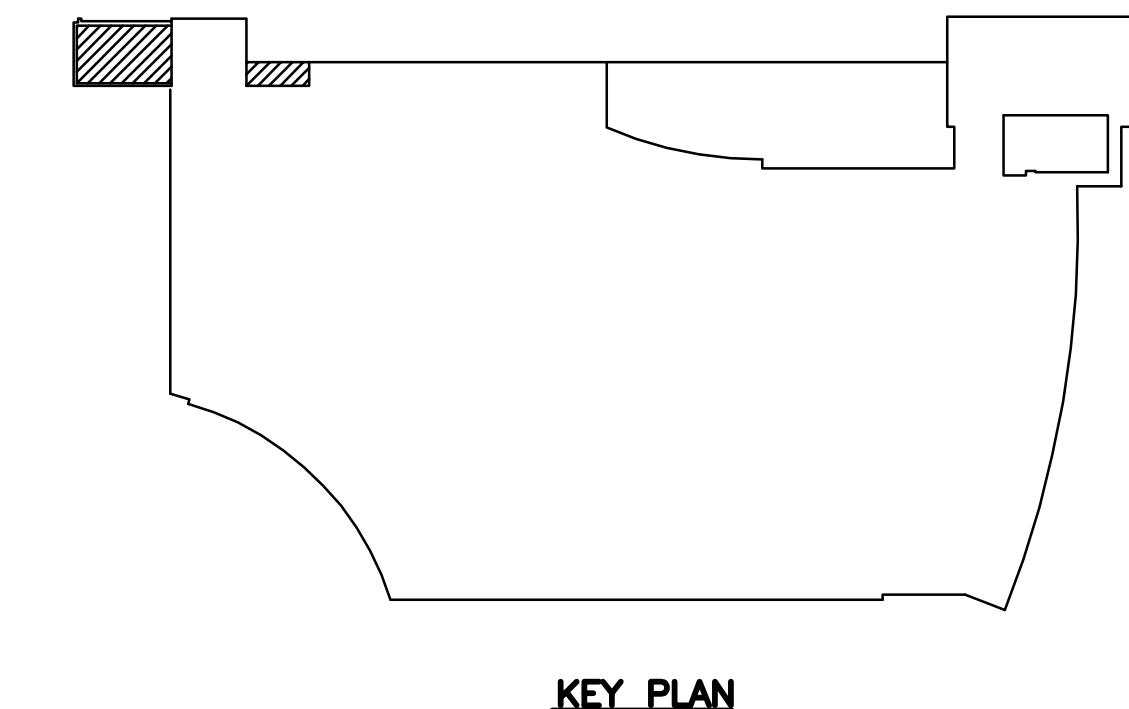
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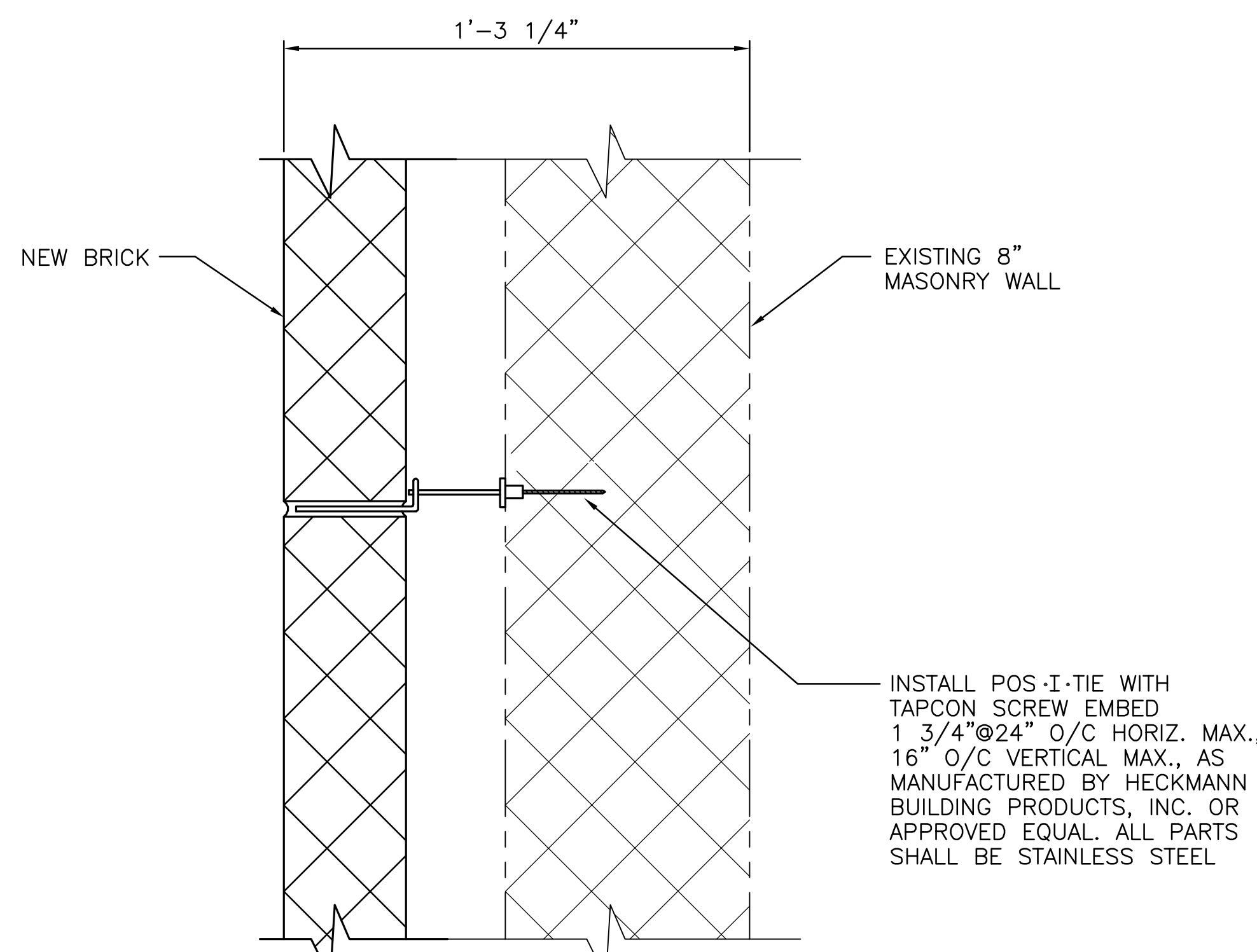
PARTIAL THIRD FLOOR FRAMING PLAN
SCALE: 1/8"=1'-0"



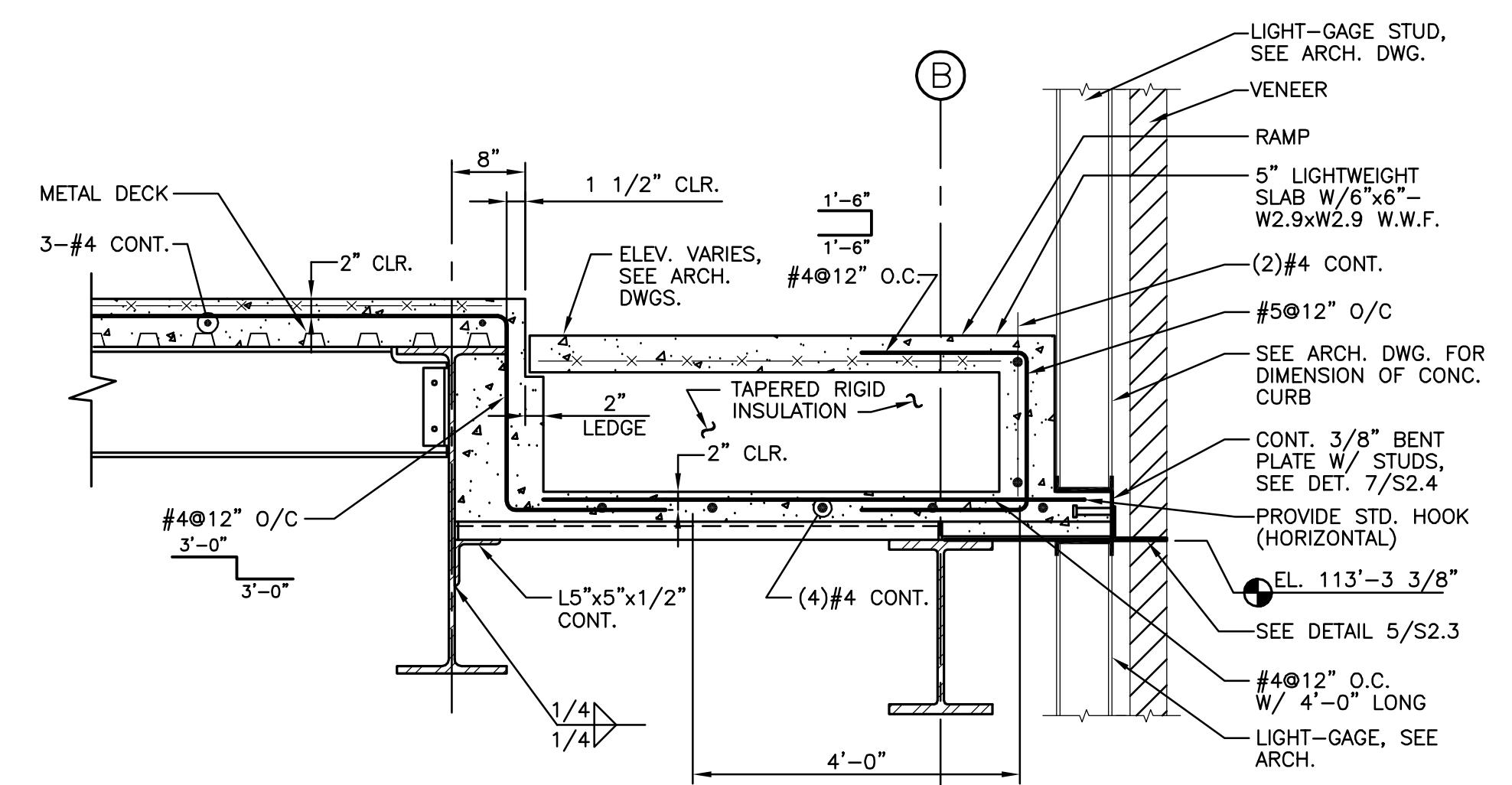
PARTIAL FOURTH FLOOR FRAMING PLAN
SCALE: 1/8"=1'-0"



KEY PLAN



1 SECTION
S1.8 SCALE: 3"=1'-0"



2 SECTION
S1.8 SCALE: 3/4"=1'-0"

GENERAL NOTES

1. INSTALL SHELF ANGLES PER SECTION 2/S1.8 AND SECTIONS ALREADY SPECIFIED IN 100% CONSTRUCTION DOCUMENT DATED MAY 2, 2007. SEE SHEET ALT 2 DATED 05/22/08 BULLETIN #44 FOR LOCATION OF ADDED BRICK.
2. SEE MASONRY WALL LINTEL SCHEDULE ON S4.1 SPECIFIED IN 100% CONSTRUCTION DOCUMENT DATED MAY 2, 2007 FOR ADDED WINDOW LINTELS. SEE SHEET ALT 2 DATED 05/22/08 BULLETIN #44 FOR LOCATION OF ADDED WINDOWS.

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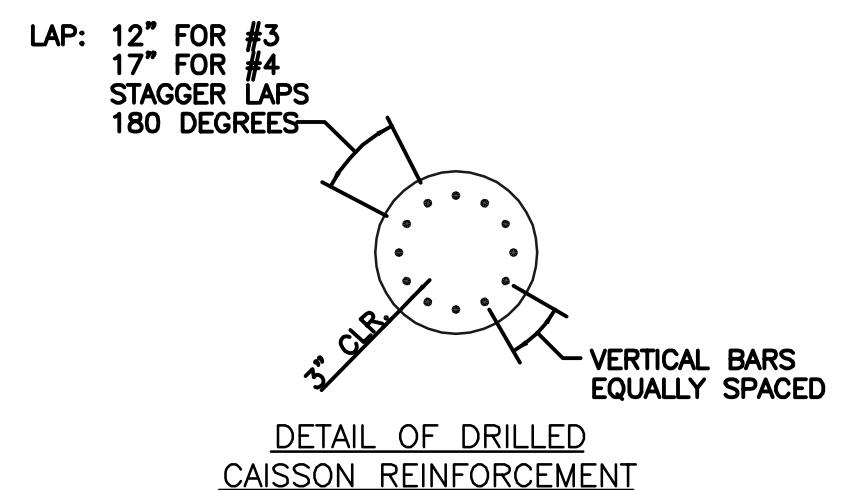
Alternates
10 & 11
Plans And
Sections

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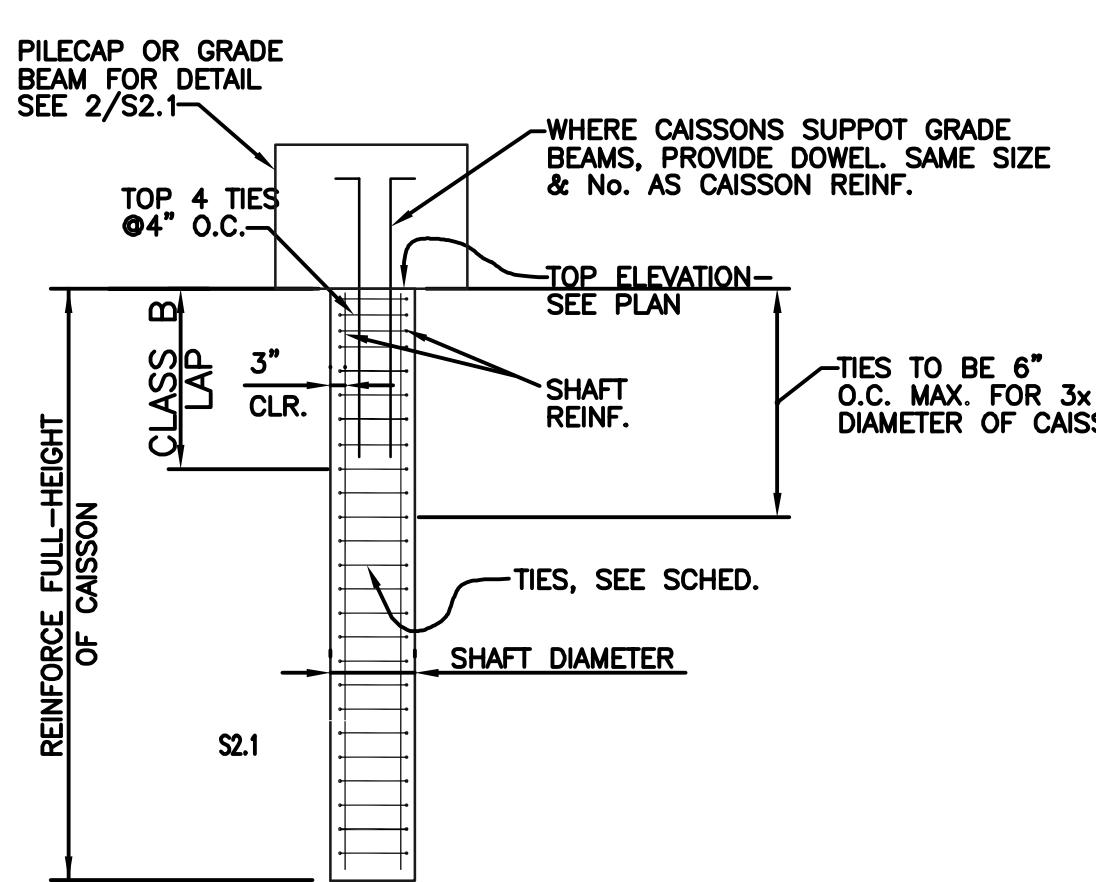
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S1.8



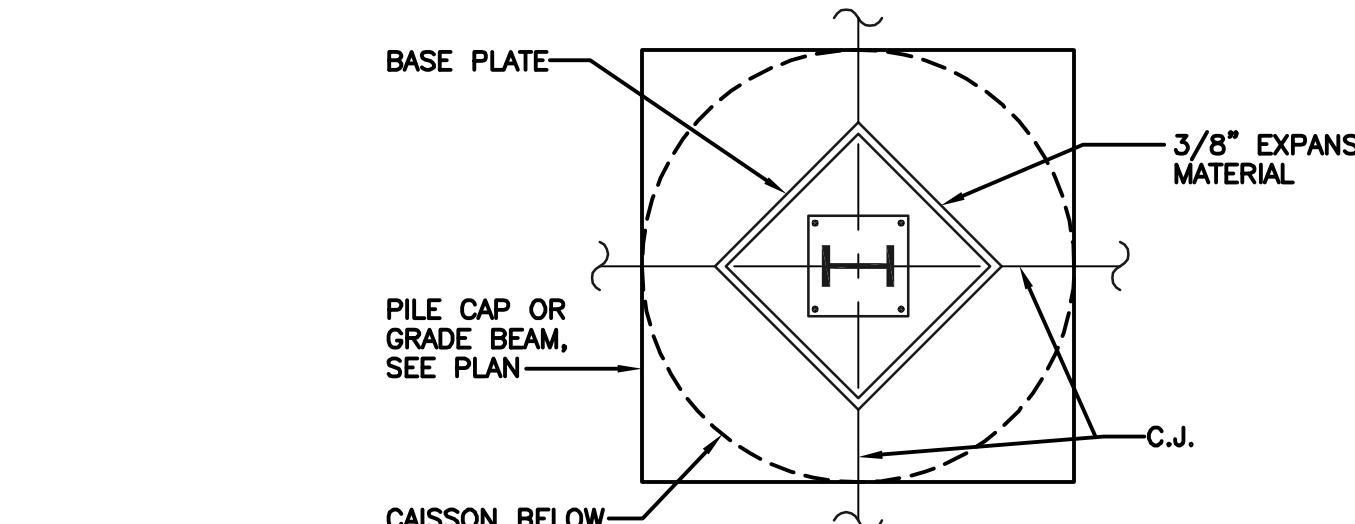
S2.1 SCALE: 3/8"=1'-0"



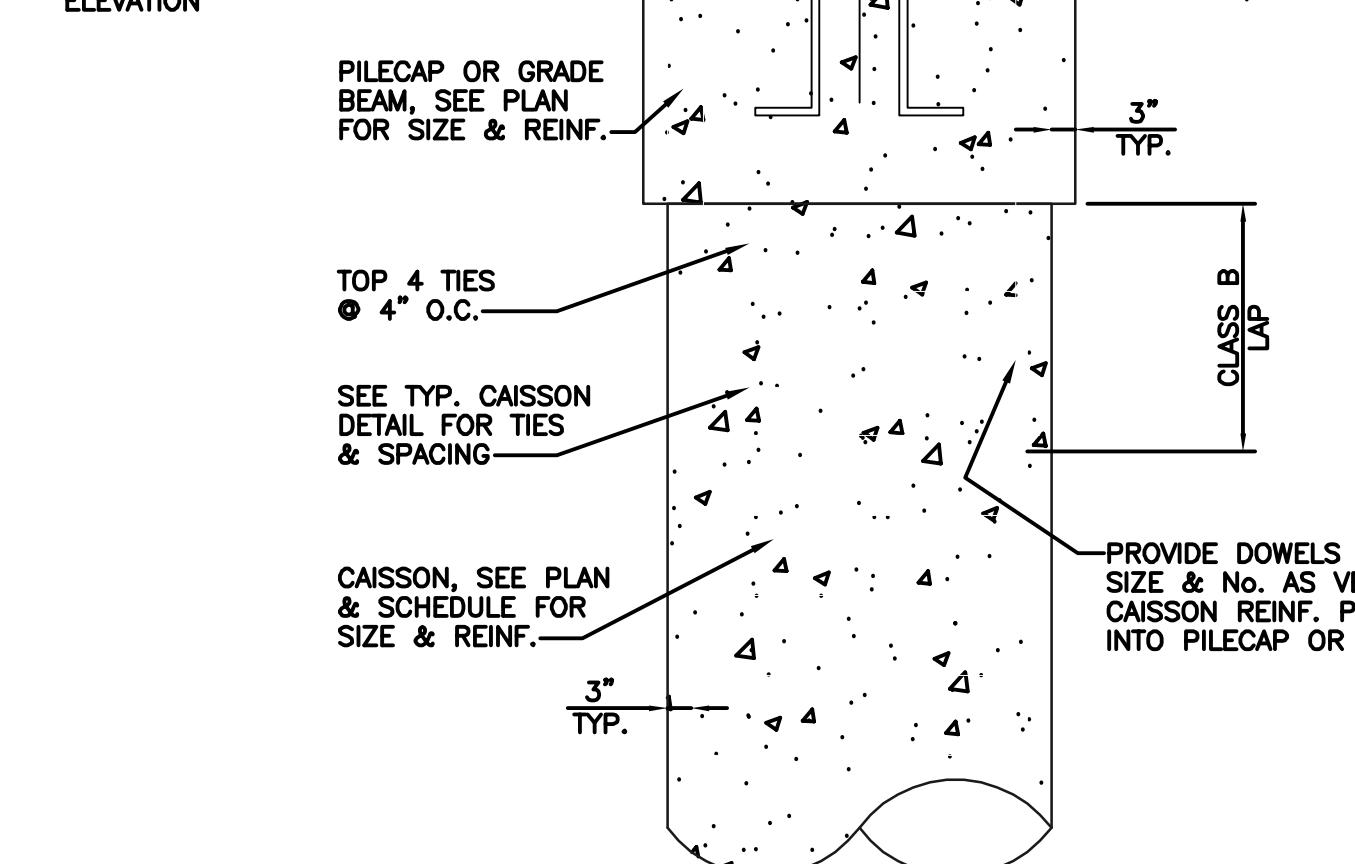
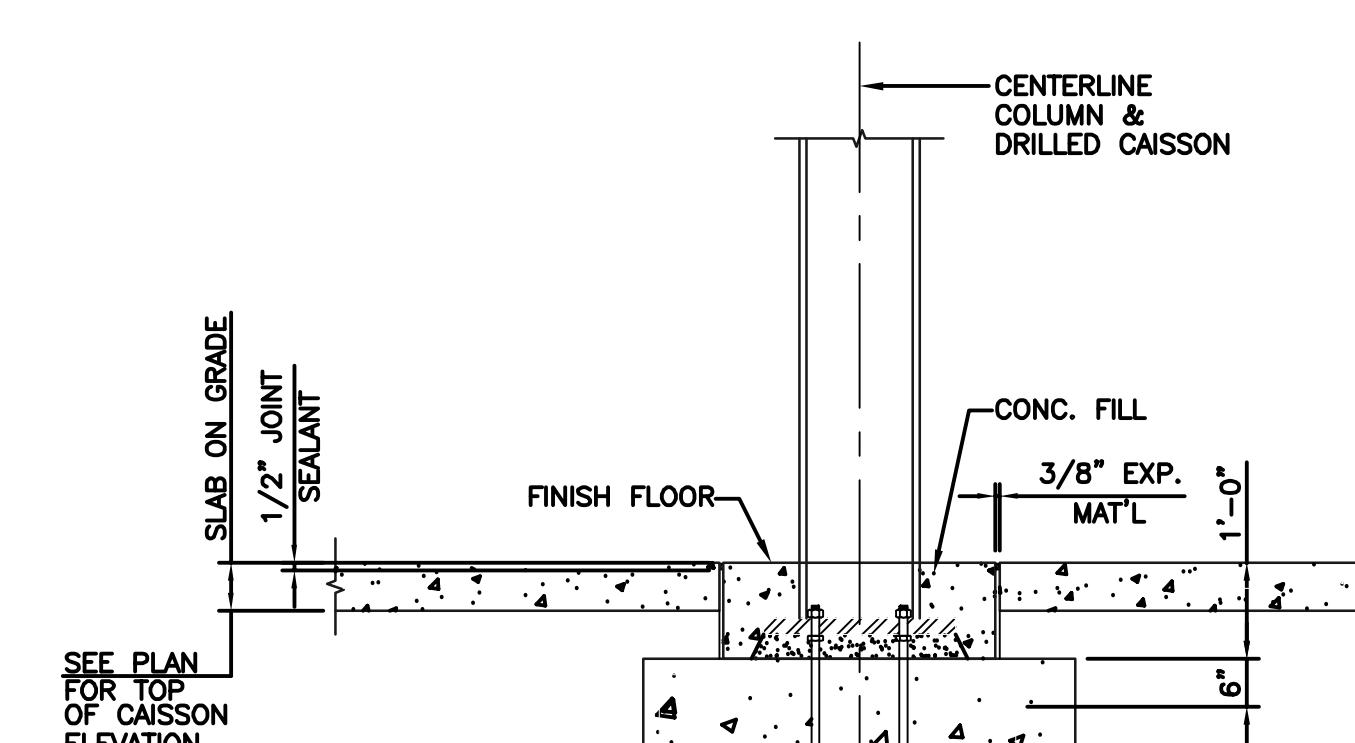
NOTES:
 1. SEE PLANS FOR DIA. OF SHAFT, TOP AND BOTTOM ELEVATIONS
 2. SHAFT AXIS SHALL NOT BE MORE THAN 3" IN 10'-0" OUT OF PLUMB.
 3. THE BOTTOM BEARING SHALL BE LEVEL AND ALL DISTURBED MATERIAL REMOVED.

TYPICAL DRILLED PIER (CAISSON)

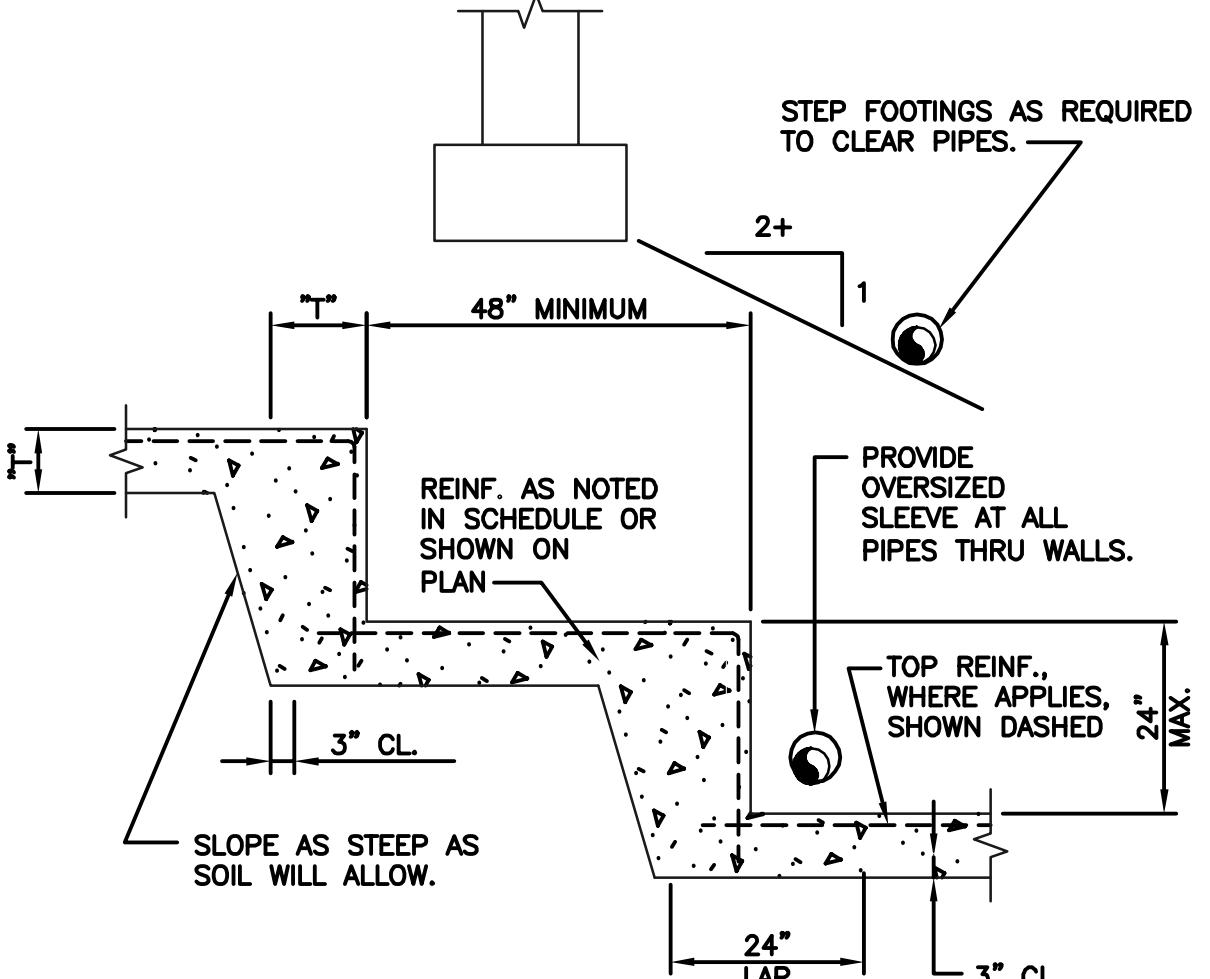
S2.1 NOT TO SCALE



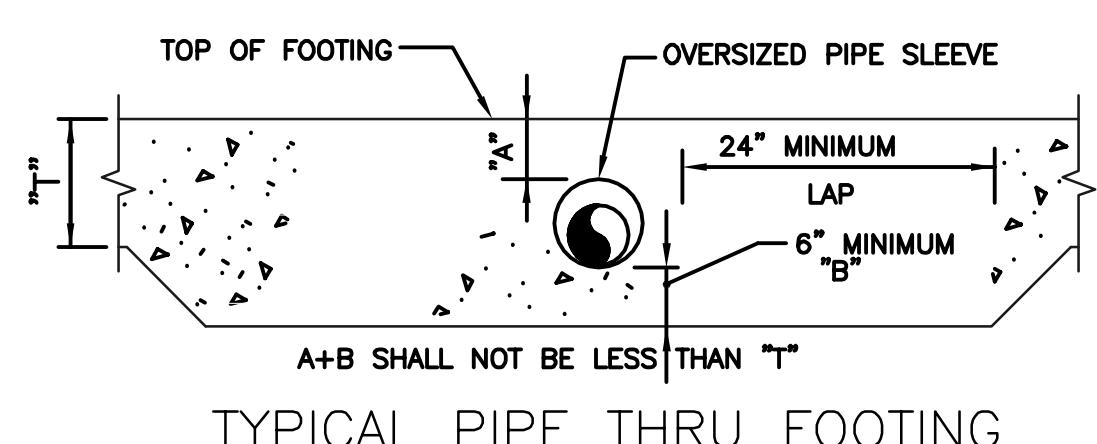
S2.1 SCALE: 1/8"=1'-0"



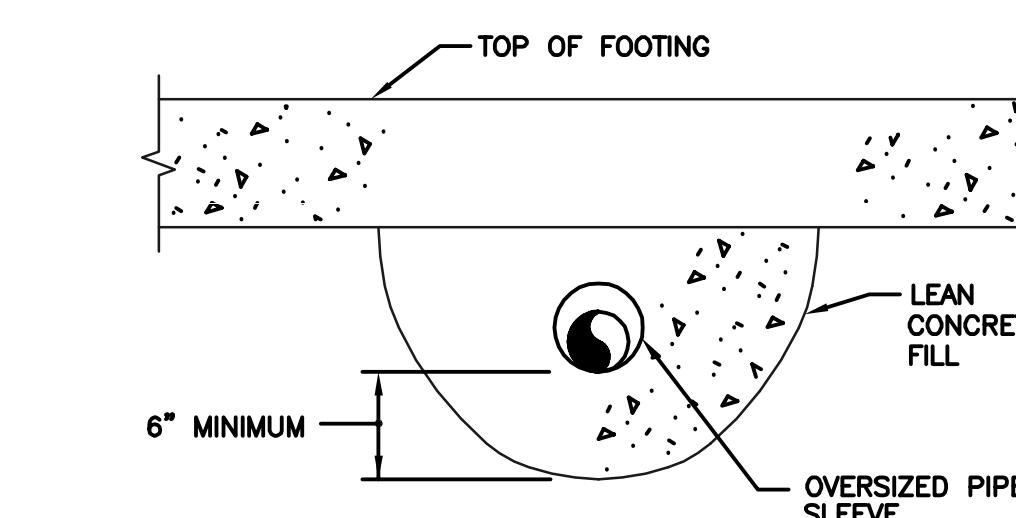
TYPICAL CAISSON ANCHORAGE TO PILE-CAP OR GRADE BEAM



TYPICAL STEPPED FOOTING

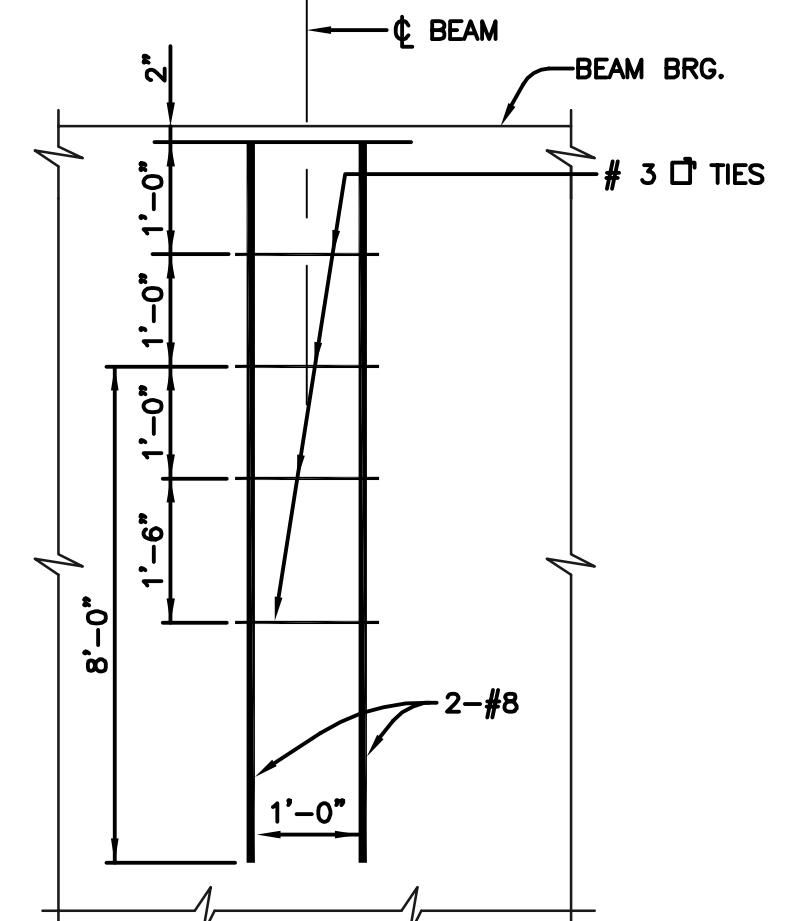


TYPICAL PIPE THRU FOOTING

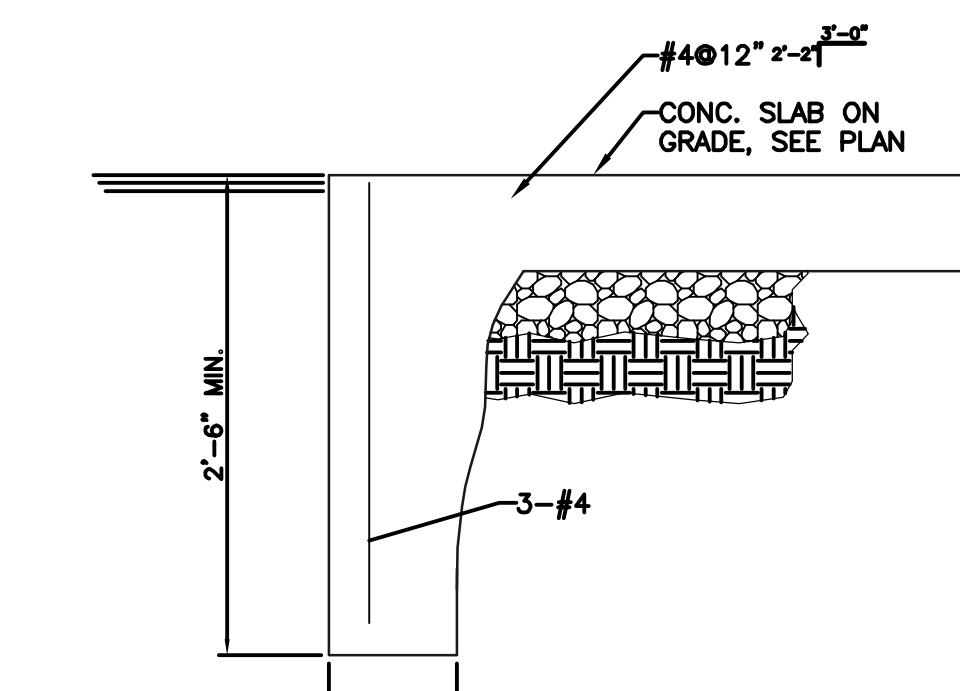


TYPICAL PIPE UNDER FOOTING

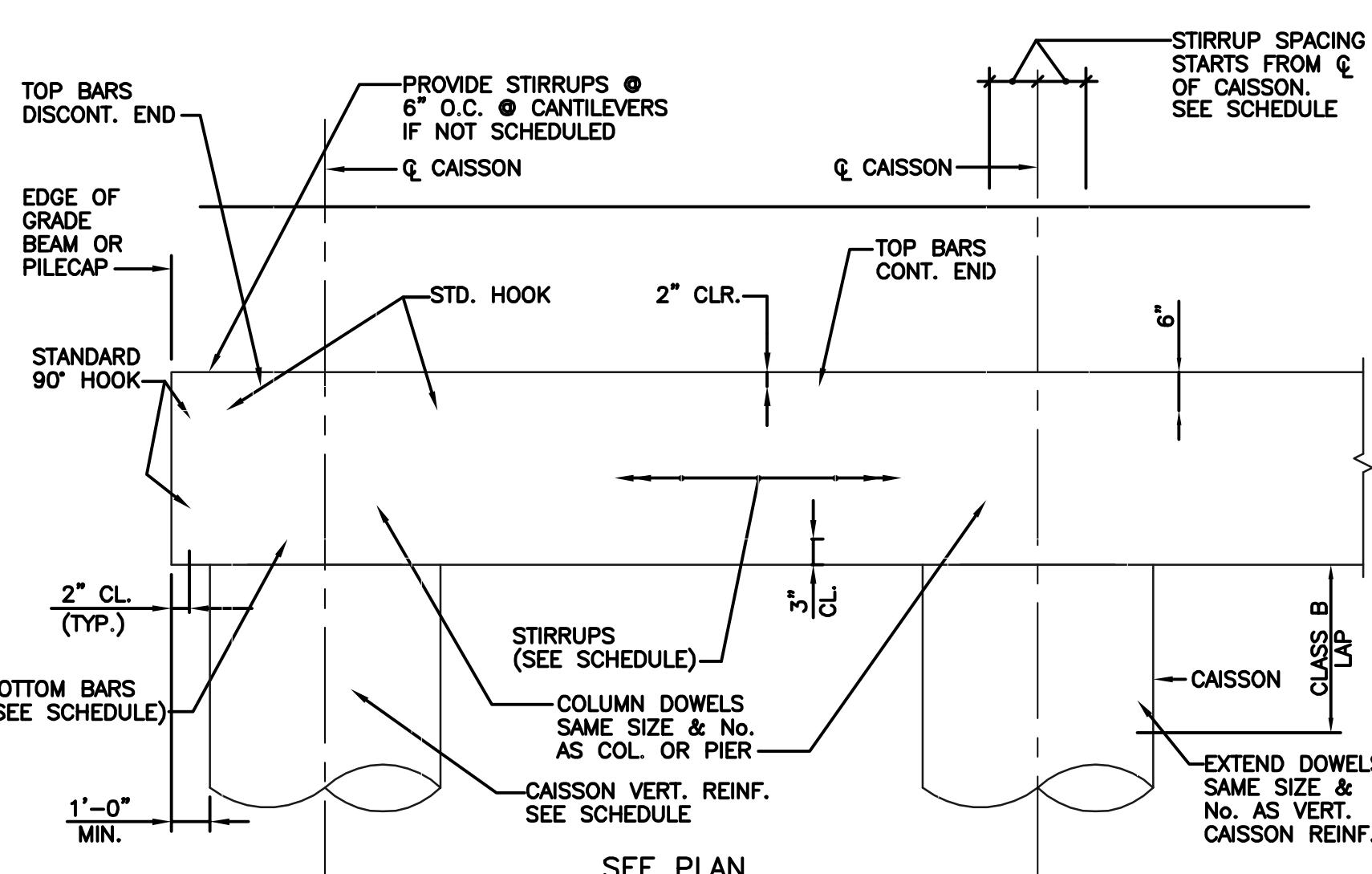
S2.1 NOT TO SCALE



ADDITIONAL STEEL IN CONCRETE WALL @ BEAM BEARING



TYPICAL TURNED-DOWN SLAB

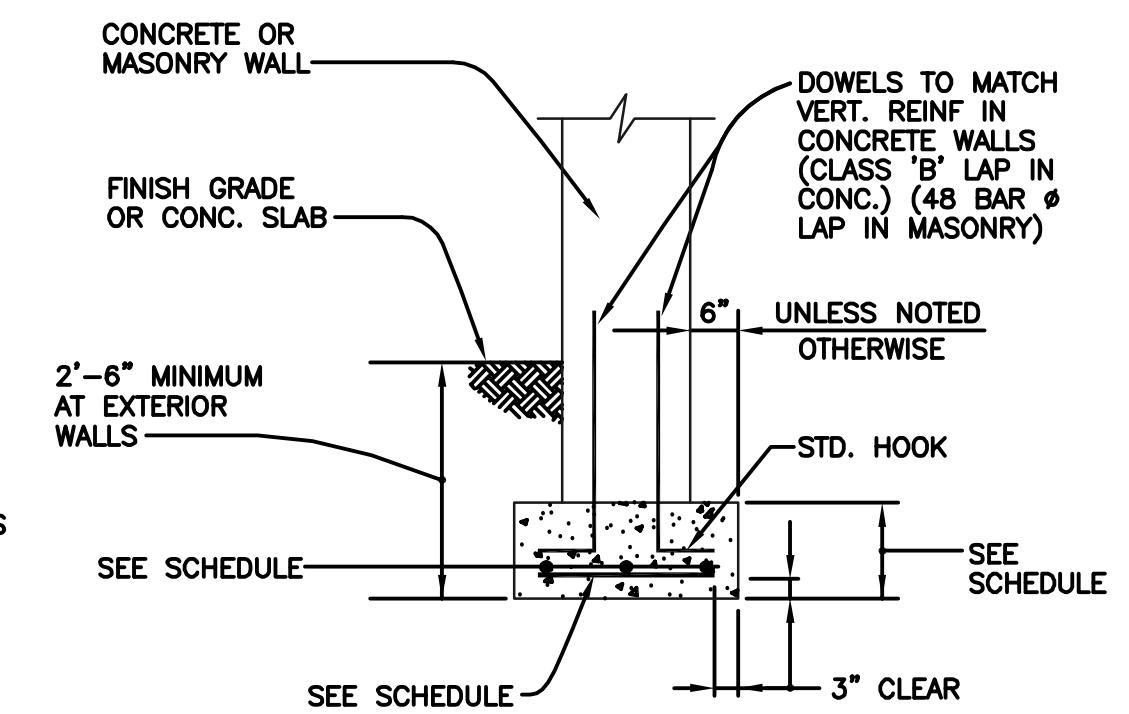


TYPICAL GRADE BEAM REINF. DETAIL

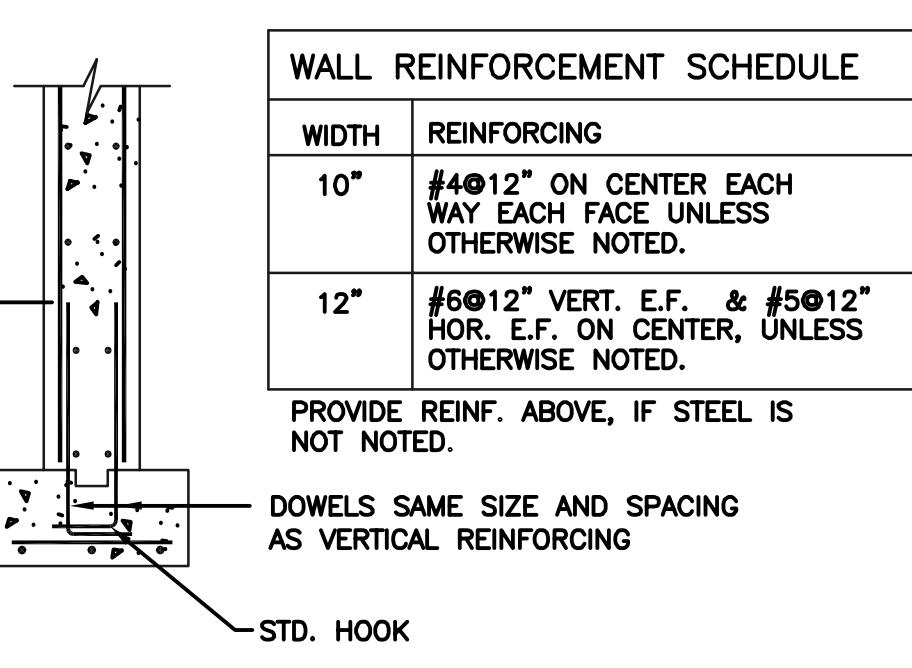
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S2.1 NOT TO SCALE

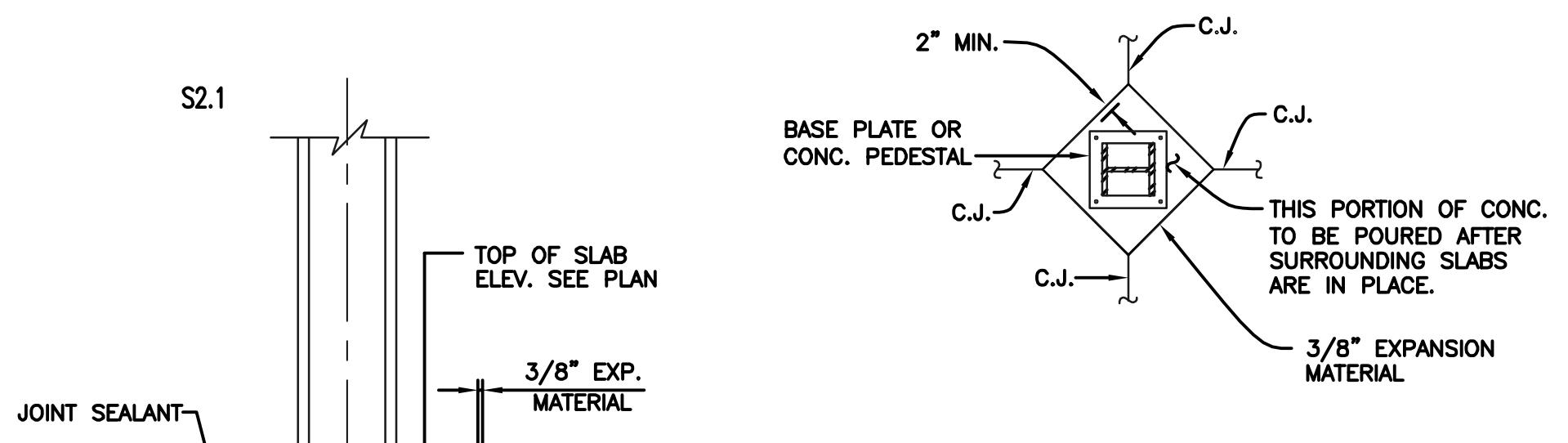
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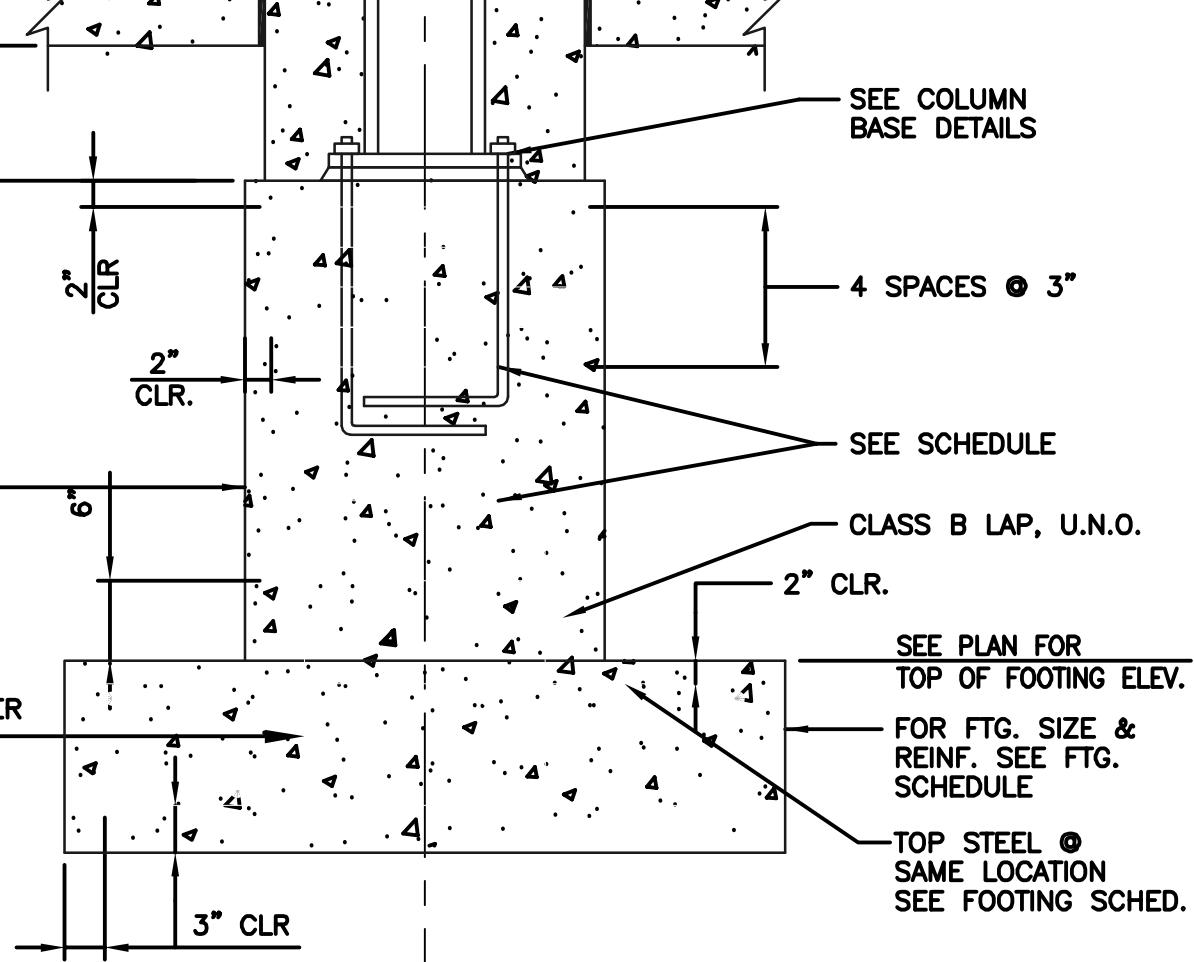
TYPICAL WALL FOOTING



TYPICAL CONCRETE WALL REINFORCING



S2.1 SCALE: 1/2"=1'-0"

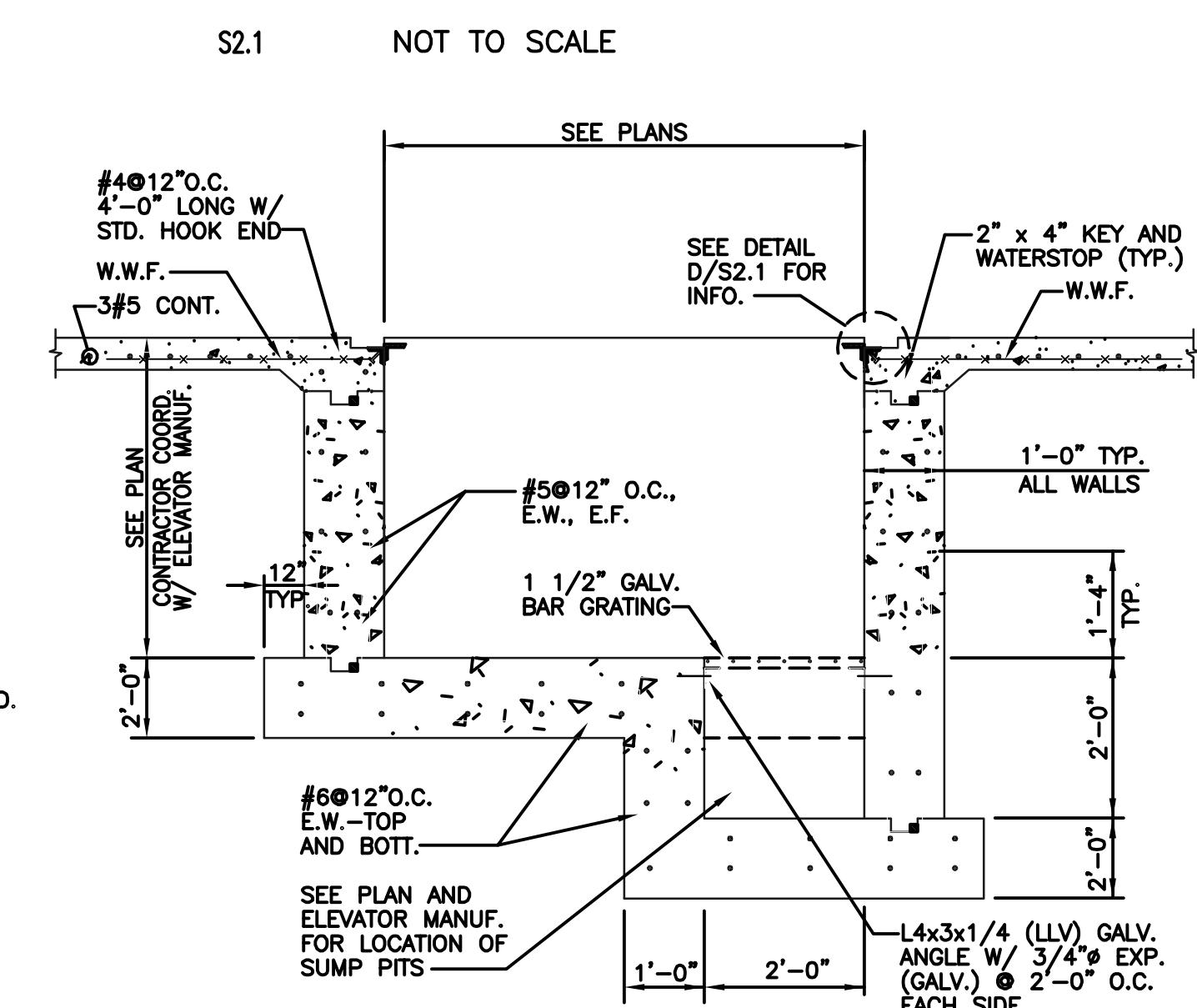


TYPICAL INTERIOR COLUMN FOOTING AND CONCRETE PIER DETAIL

S2.1 SCALE: 3/4"=1'-0"

S2.1 NOT TO SCALE

TYPICAL COLUMN AND FOOTING



S2.1 NOT TO SCALE

3 01/29/10 RECORD DRAWINGS
 2 05/02/07 BULLETIN #10
 1 04/09/07 BULLETIN #9
 5 04/06/07 ISSUE FOR BID
 Rev. Date: Comment:
 Issued: FEBRUARY 16, 2007

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 University of Maryland, Baltimore
 Baltimore, MD



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S2.1

CONC. WALL, SEE PLAN FOR REINF.
STD. HOOK
IF BARS CANNOT BE PULLED UP, IN STEP, PROVIDE HOOKS
CLASS B LAP
STD. HOOK

TYPICAL CONCRETE WALL STEP DETAIL

PROVIDE CONTROL OR CONSTRUCTION JOINTS AS CENTER CONTRACTOR SHALL SUBMIT SHOP DRAWINGS WITH LOCATION OF JOINTS BEFORE CONSTRUCTION
CUT ALTERNATE LONGITUDINAL BARS AT CONTROL AND CONSTRUCTION JOINTS
BENTONITE WATER STOP IN BELOW GRADE BEAM
CONTROL JOINT
CLASS B LAP
STANDARD HOOK
3/4" DEEP GROOVE
BENTONITE WATER STOP IN BELOW GRADE BEAM
CONSTRUCTION JOINT
3/4" KEY (TYP.)

TYPICAL CONCRETE WALL JOINT

1 **DETAIL**
S2.2 **NOT TO SCALE**

2 **DETAIL**
S2.2 **NOT TO SCALE**

3 **DETAIL**
S2.2 **NOT TO SCALE**

4 **DETAIL**
S2.2 **SCALE: 3/4"=1'-0"**

5 **DETAIL**
S2.2 **NOT TO SCALE**

6 **DETAIL**
S2.2 **NOT TO SCALE**

7 **SECTION**
S2.2 **NOT TO SCALE**

8 **DETAIL**
S2.2 **NOT TO SCALE**

9 **DETAIL**
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10 **DETAIL**
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11 **DETAIL**
S2.2 **NOT TO SCALE**

12 **DETAIL**
S2.2 **NOT TO SCALE**

13 **SECTION**
S2.2 **SCALE: 1/2"=1'-0"**

EQUIPMENT PAD NOTES:

1. ALL CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH $f_c' = 4000$ PSI AT 28 DAYS. CONCRETE SHALL HAVE $5\% \pm 1\%$ ENTRAINED AIR.
2. ALL REINFORCEMENT SHALL BE ASTM A-615 GRADE 60.
3. ANCHOR EQUIPMENT TO CONCRETE PAD PER MANUFACTURER'S RECOMMENDATIONS.
4. PROVIDE GALVANIZED STEEL SHIMS AND NON-SHRINK GROUT UNDER EQUIPMENT AS REQUIRED TO LEVEL EQUIPMENT AND FULLY SUPPORT PER MANUFACTURER'S RECOMMENDATIONS.

14 **DETAIL**
S2.2 **NOT TO SCALE**

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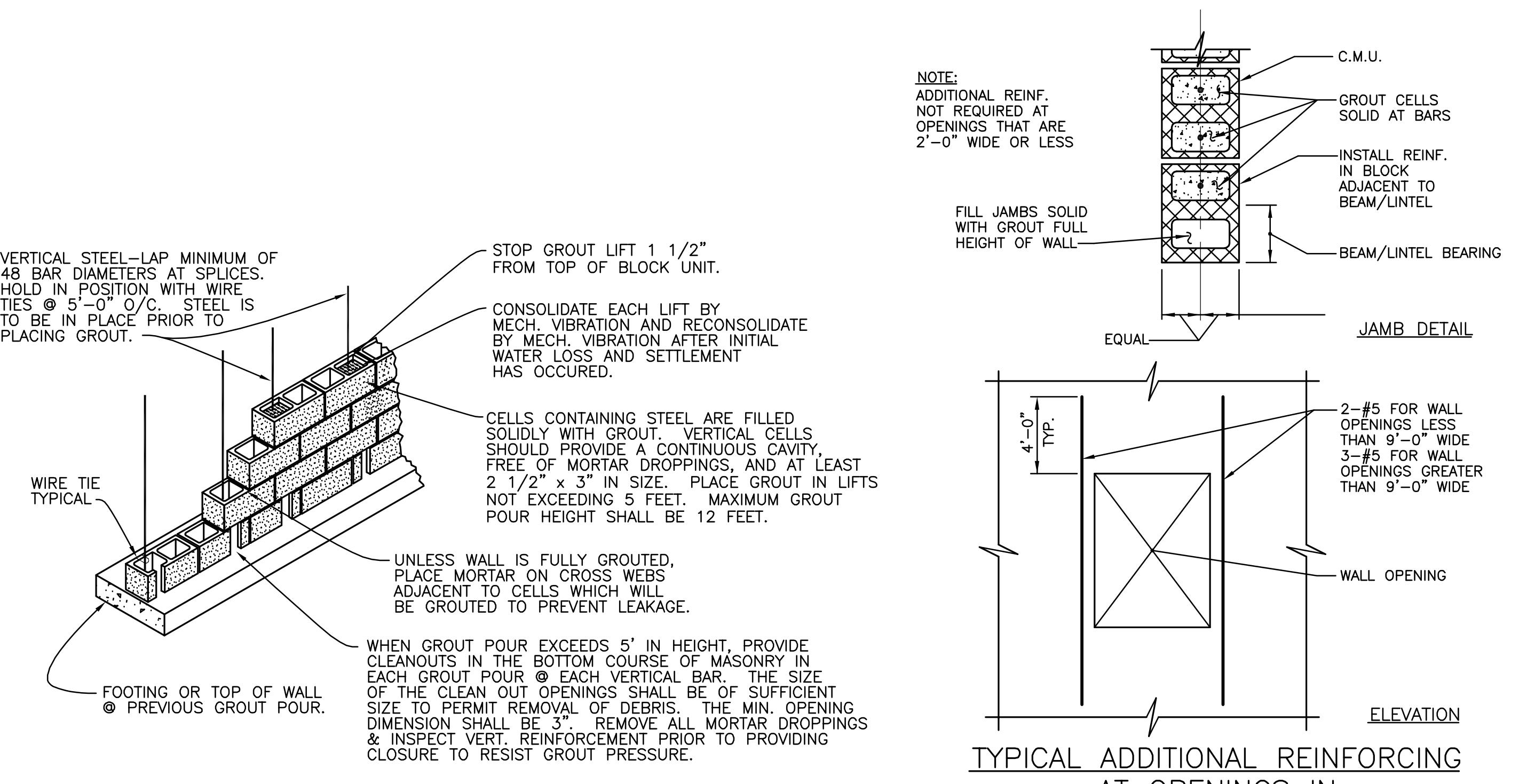
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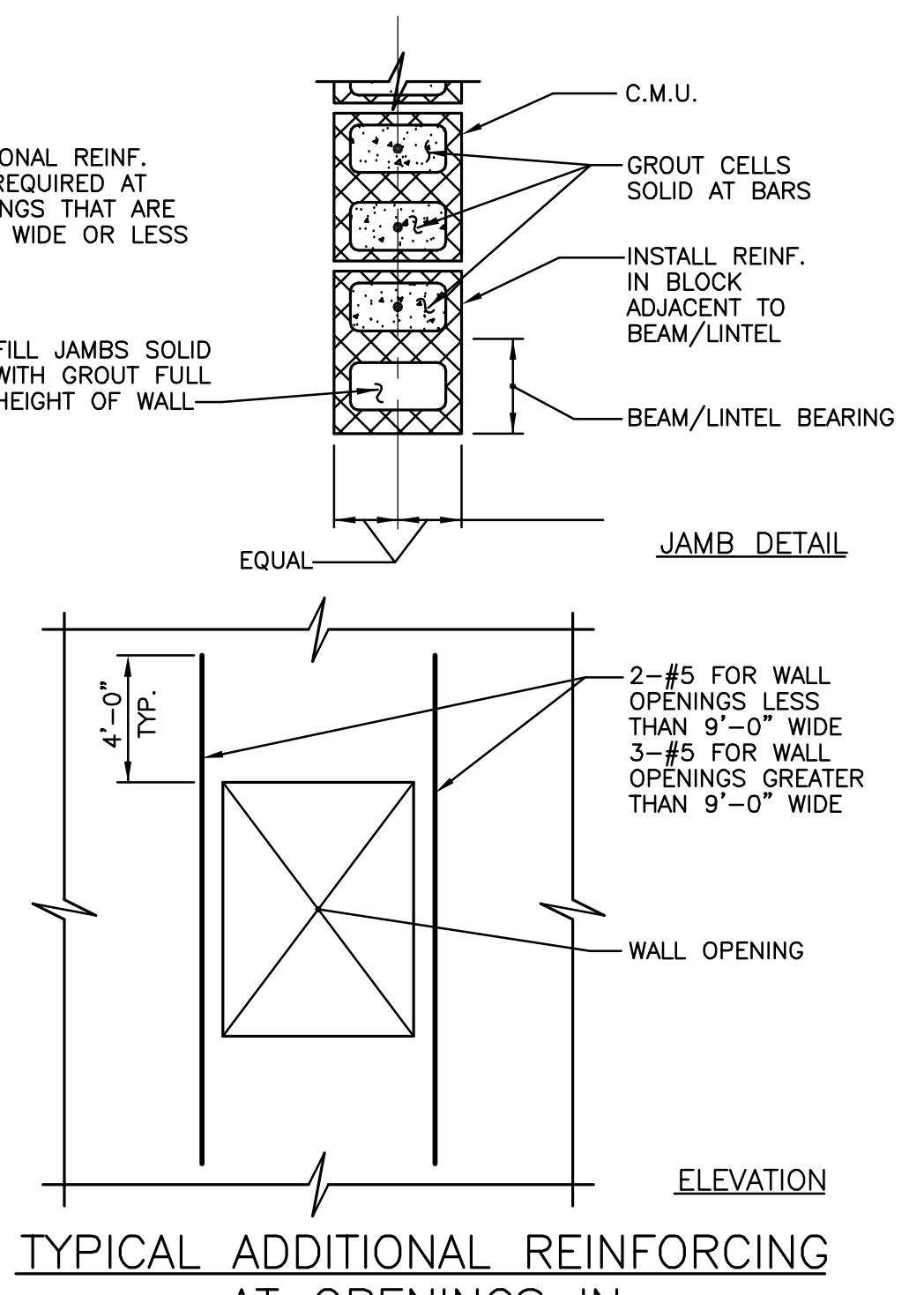
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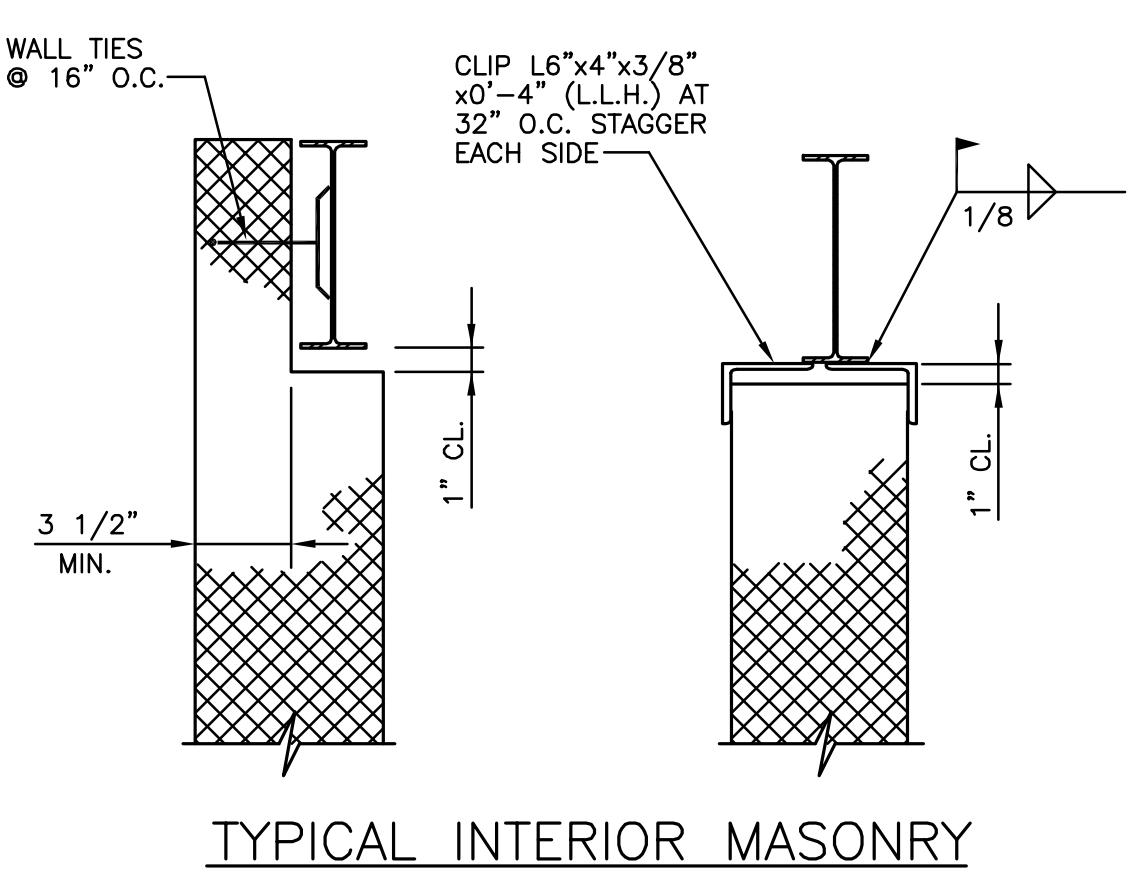
TYPICAL REINFORCED MASONRY CONSTRUCTION

1
S2.3
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NOT TO SCALE



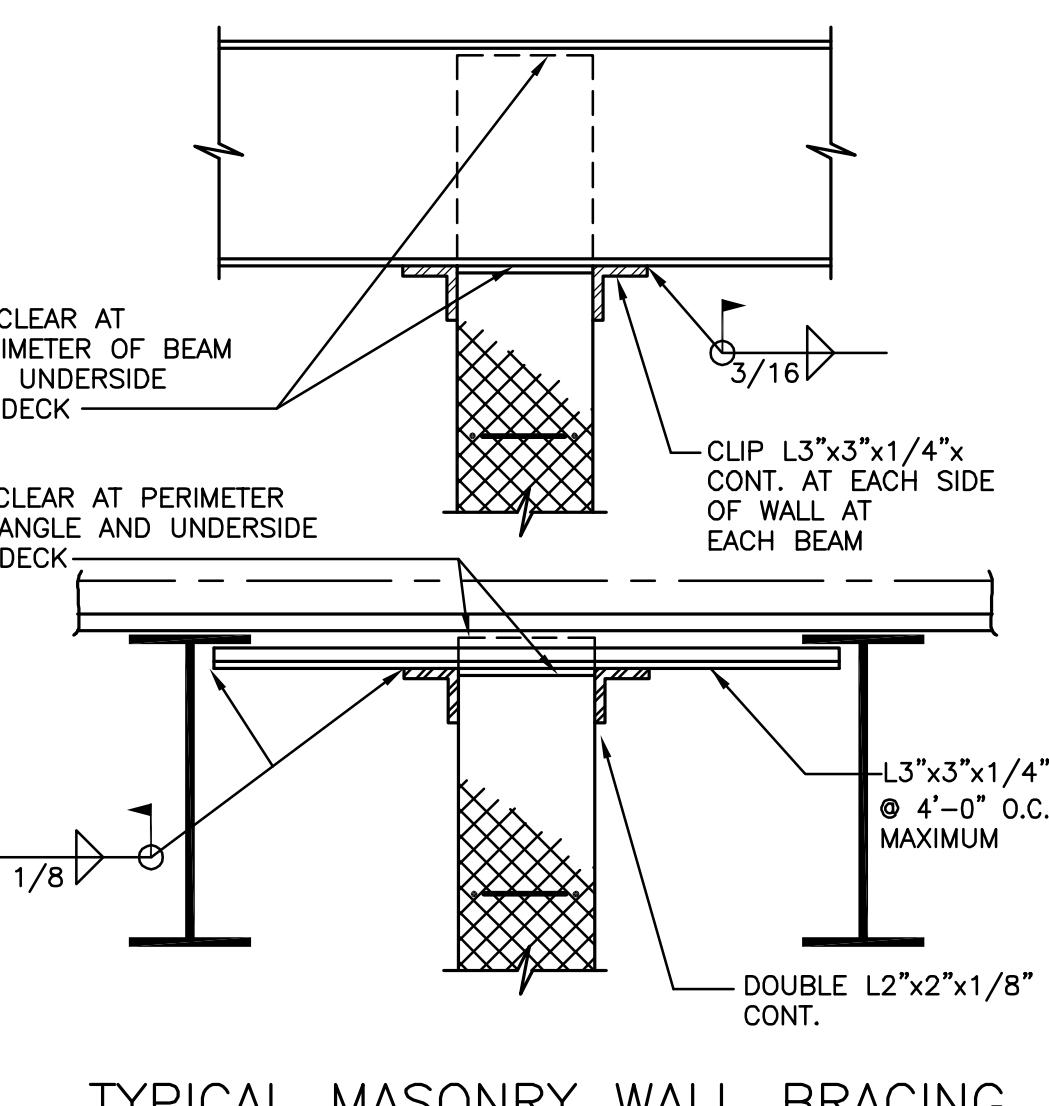
TYPICAL ADDITIONAL REINFORCING AT OPENINGS IN EXTERIOR MASONRY WALLS

2
S2.3
DETAIL
NOT TO SCALE



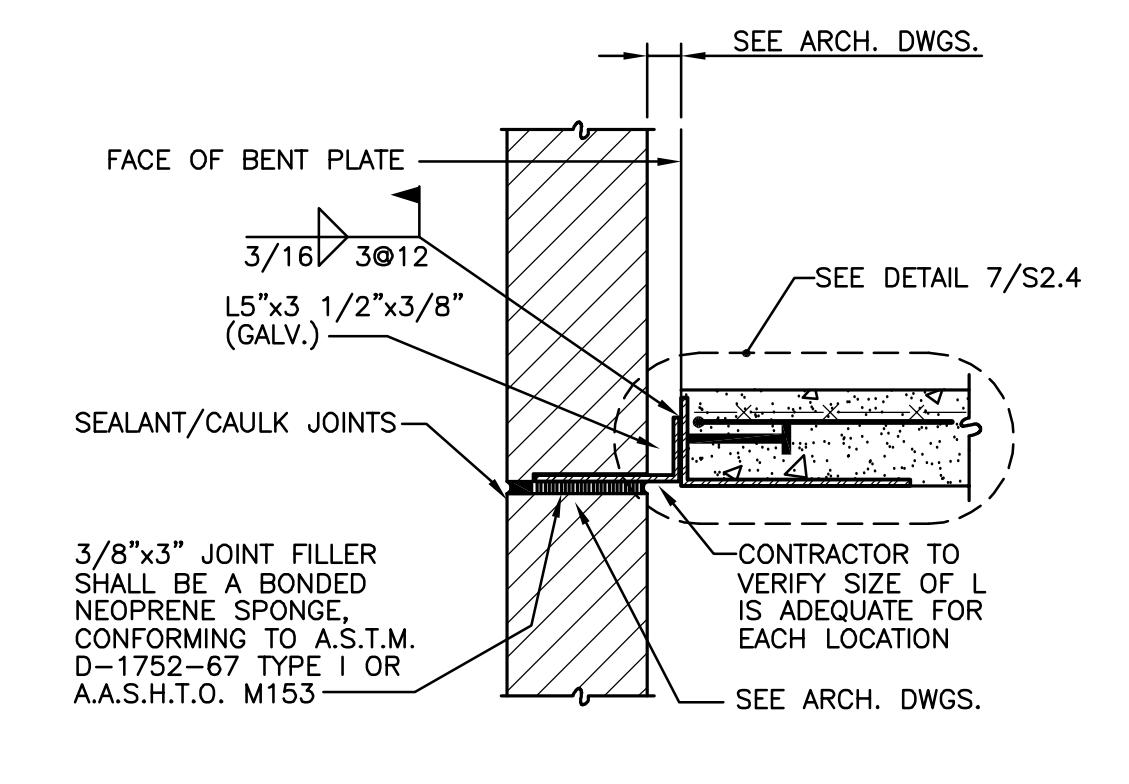
TYPICAL INTERIOR MASONRY WALL BRACING

3
S2.3
DETAIL
NOT TO SCALE



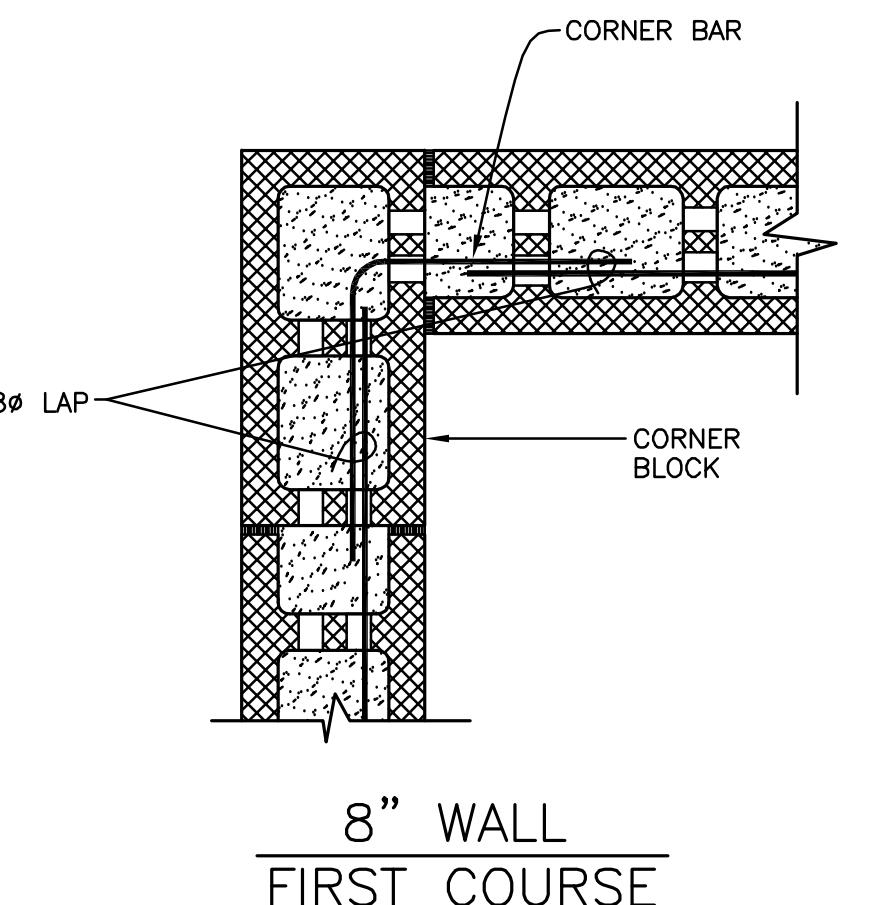
TYPICAL MASONRY WALL BRACING

4
S2.3
DETAIL
NOT TO SCALE

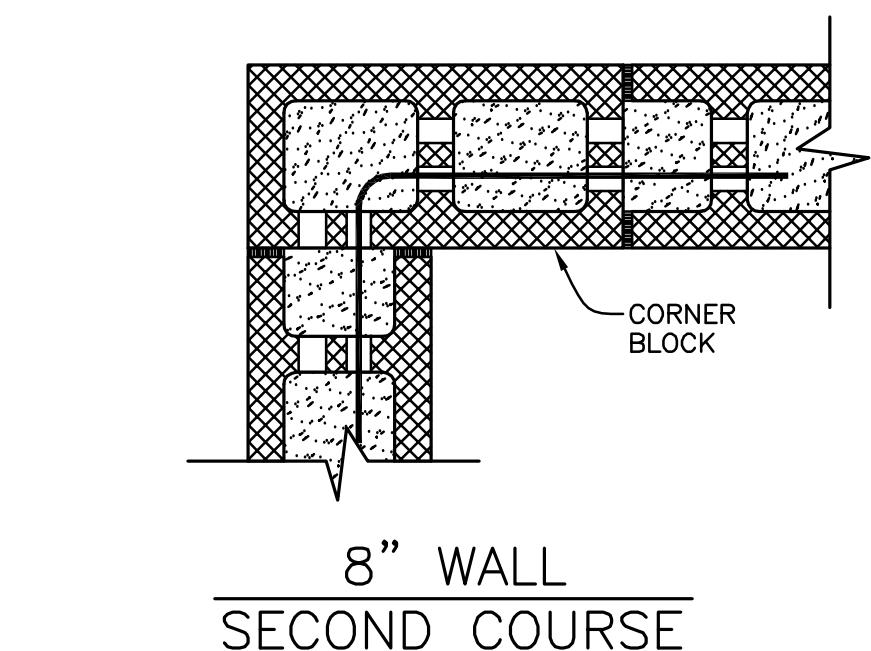


TYPICAL SHELF ANGLE EXTENSION WALL

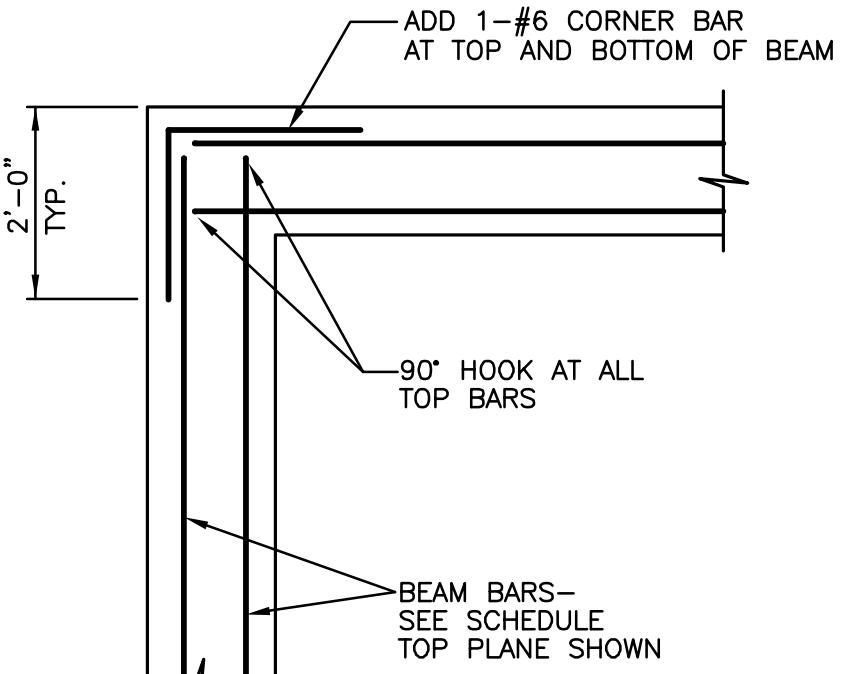
5
S2.3
DETAIL
NOT TO SCALE



8' WALL FIRST COURSE

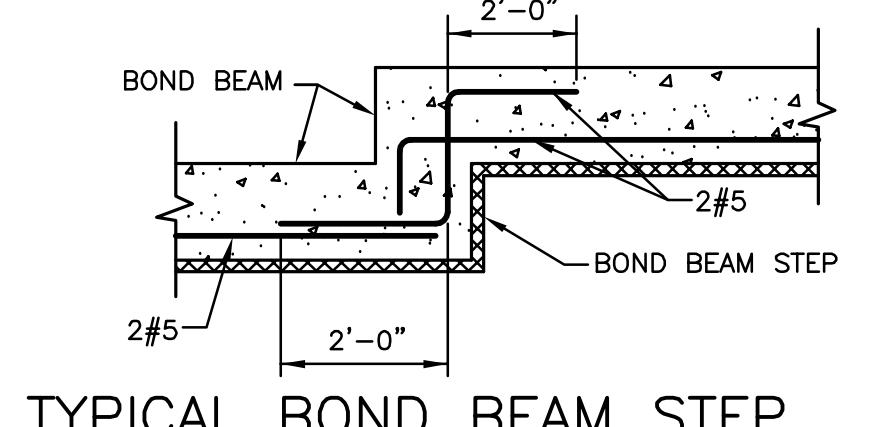


8' WALL SECOND COURSE



Typical Grade Beam Corner Plan

6
S2.3
DETAIL
NOT TO SCALE



7
S2.3
DETAIL
NOT TO SCALE

8
S2.3
DETAIL
NOT TO SCALE

3 01/29/10 RECORD DRAWINGS
 2 05/02/07 BULLETIN #10
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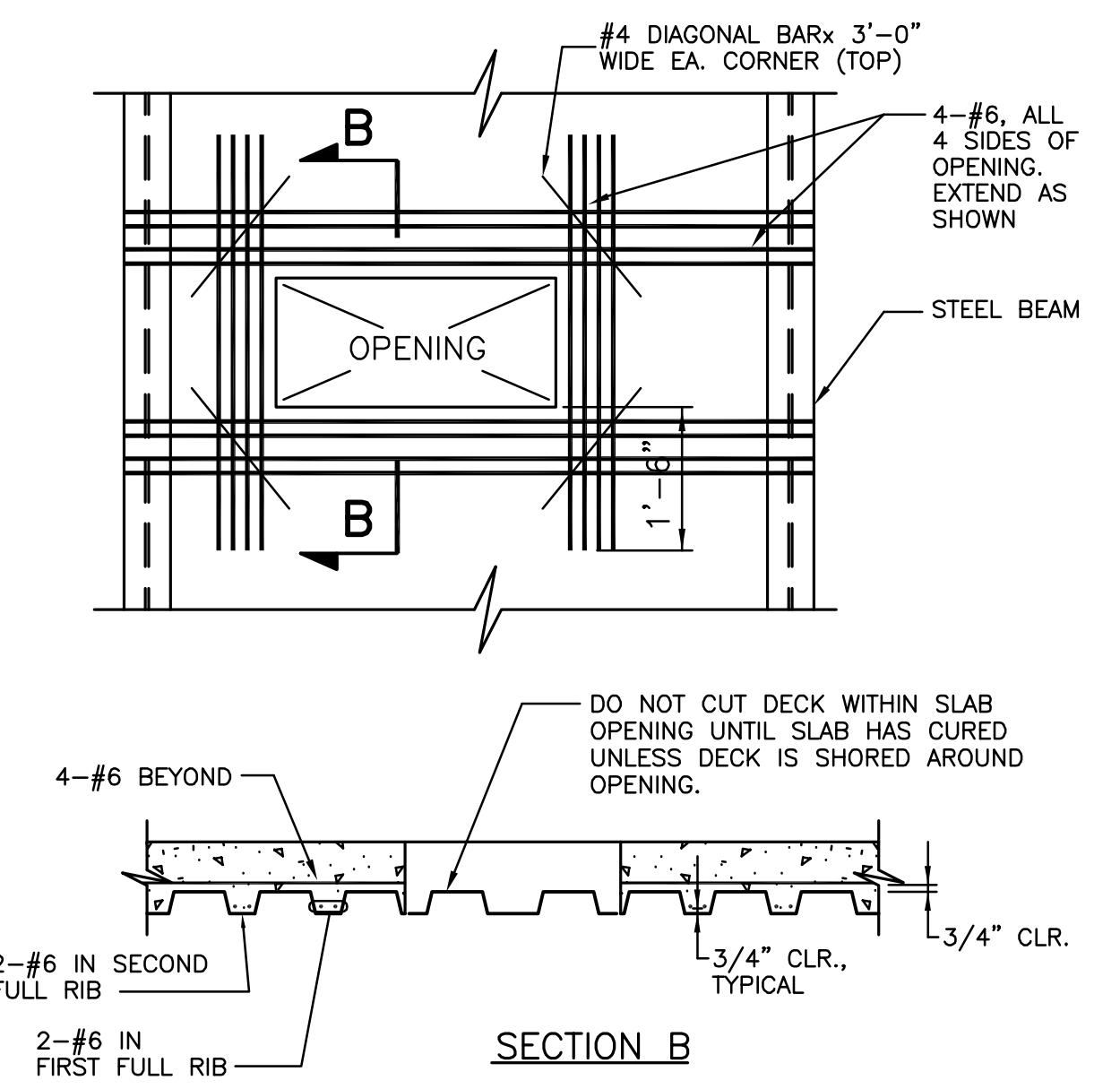
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100%
 CONSTRUCTION
 DOCUMENTS

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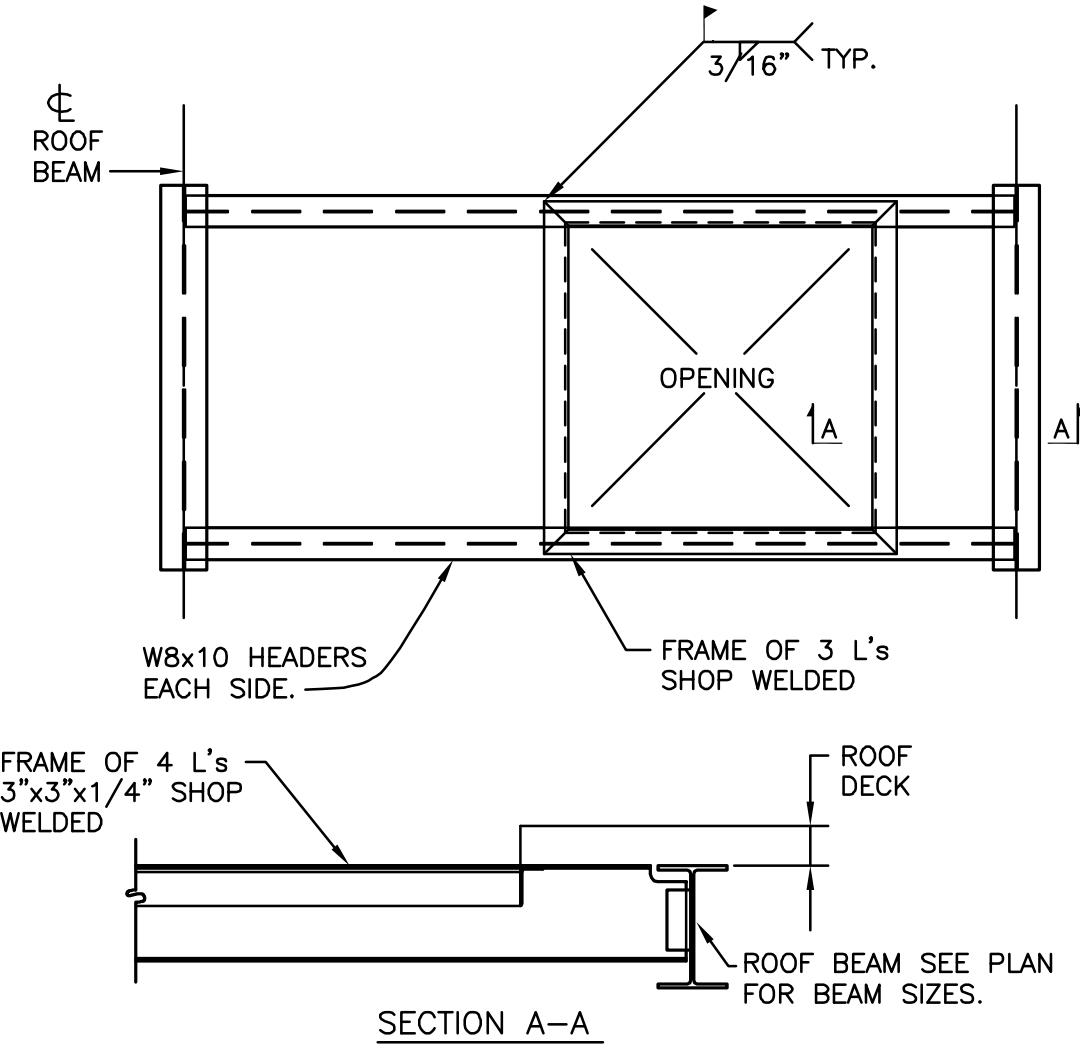
UMB PROJECT NO. 99-311
 WTW PROJECT NO. 703-4091
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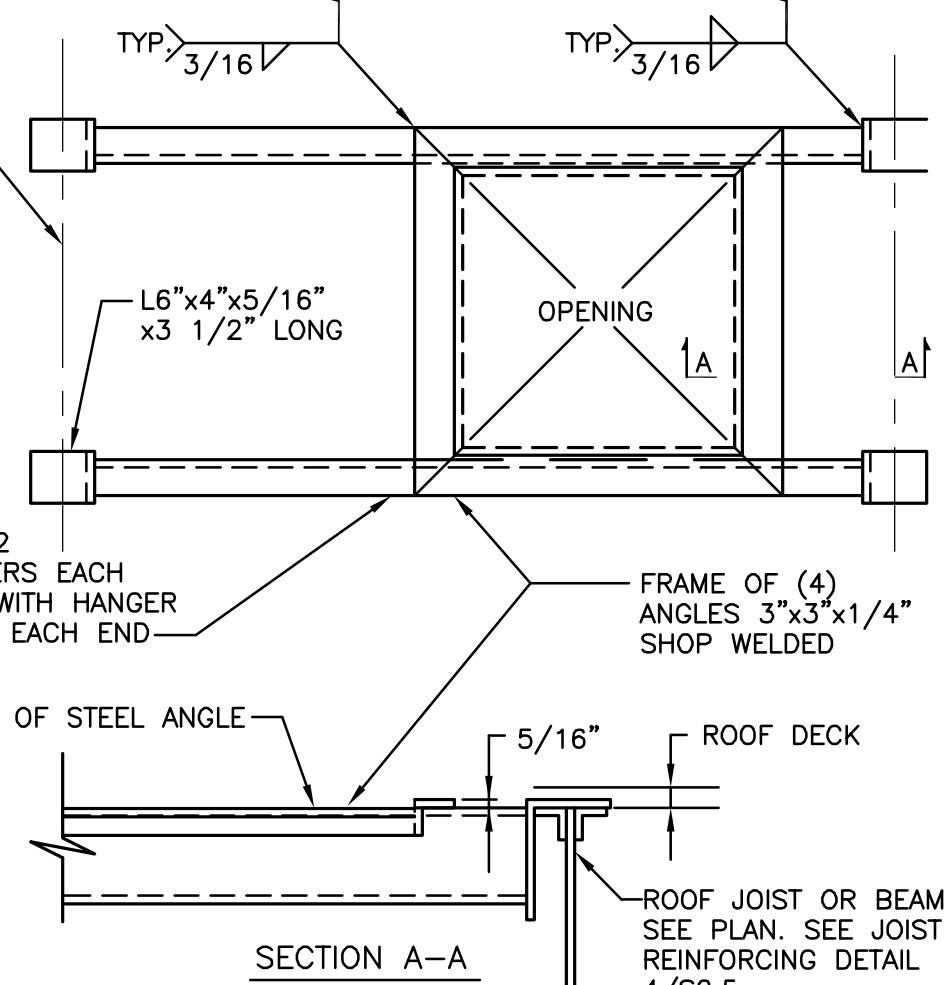


ADDITIONAL BARS AROUND SLAB OPENINGS WITH METAL DECK

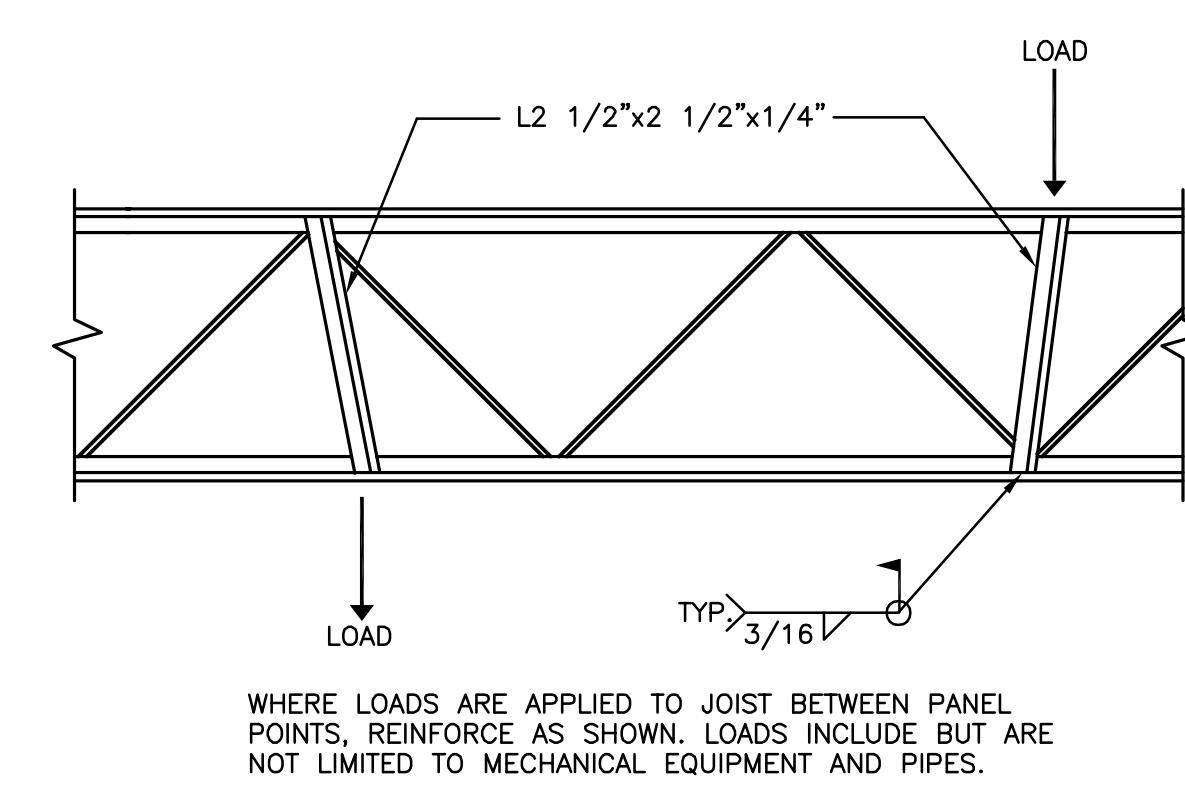
1 DETAIL
S2.5 NOT TO SCALE



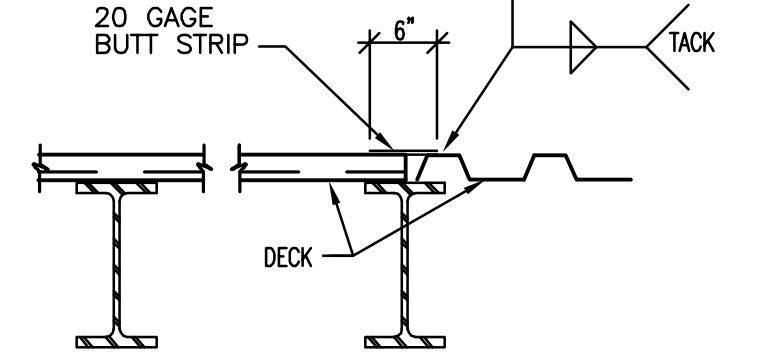
TYPICAL MECHANICAL OPENING



TYPICAL MECHANICAL OPENING

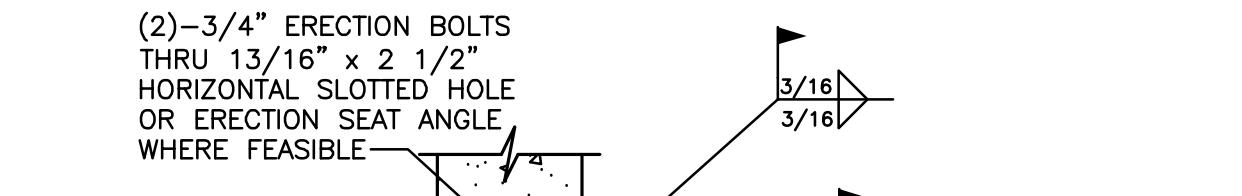


TYPICAL JOIST REINFORCEMENT

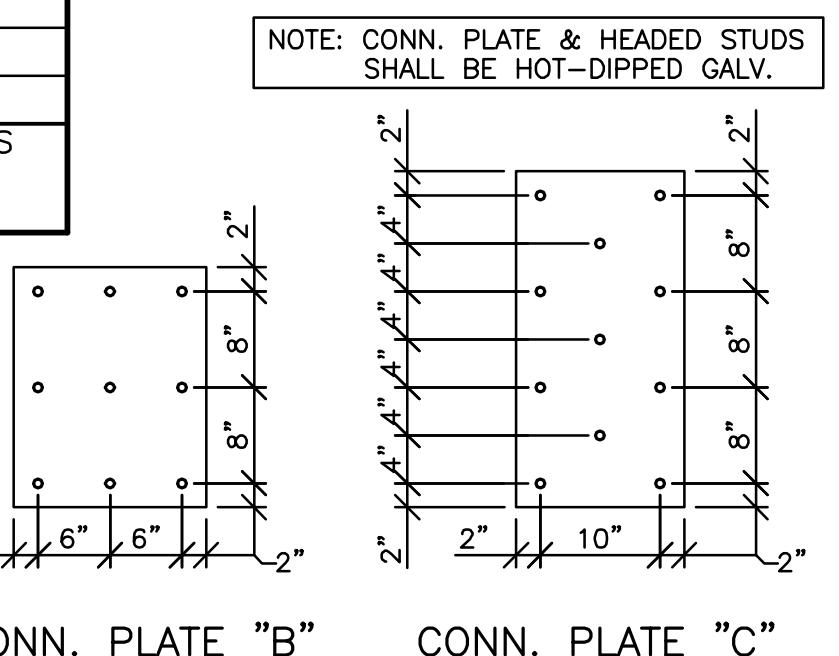


DECK SUPPORT WHERE DECK SPAN CHANGES DIRECTION

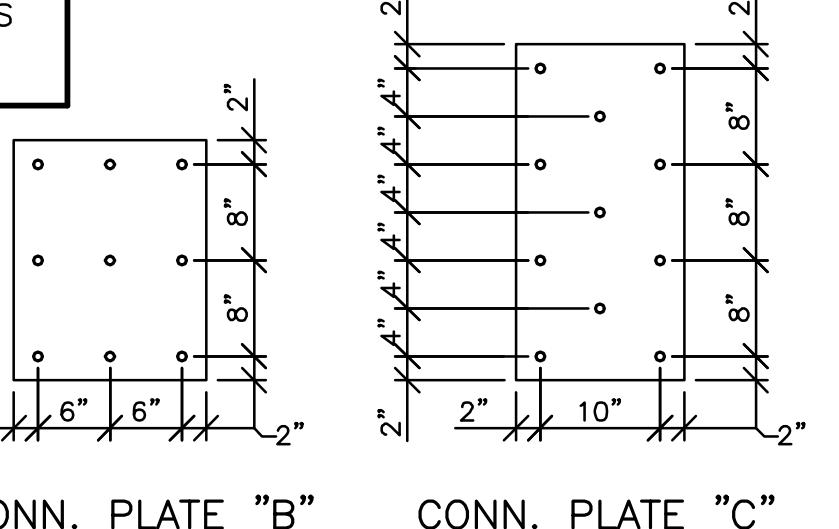
4 DETAIL
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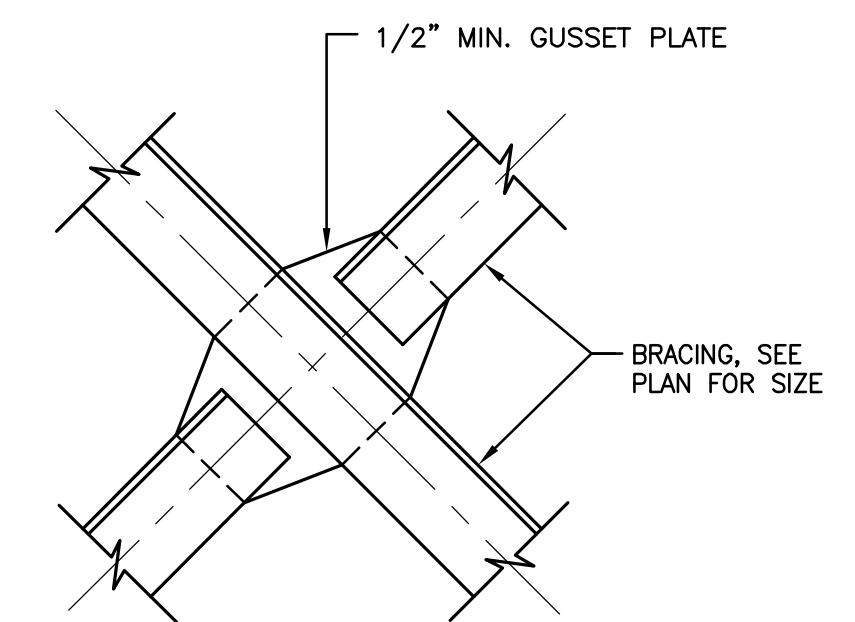
NOTE: CONN. PLATE & HEADED STUDS SHALL BE HOT-DIPPED GALV.



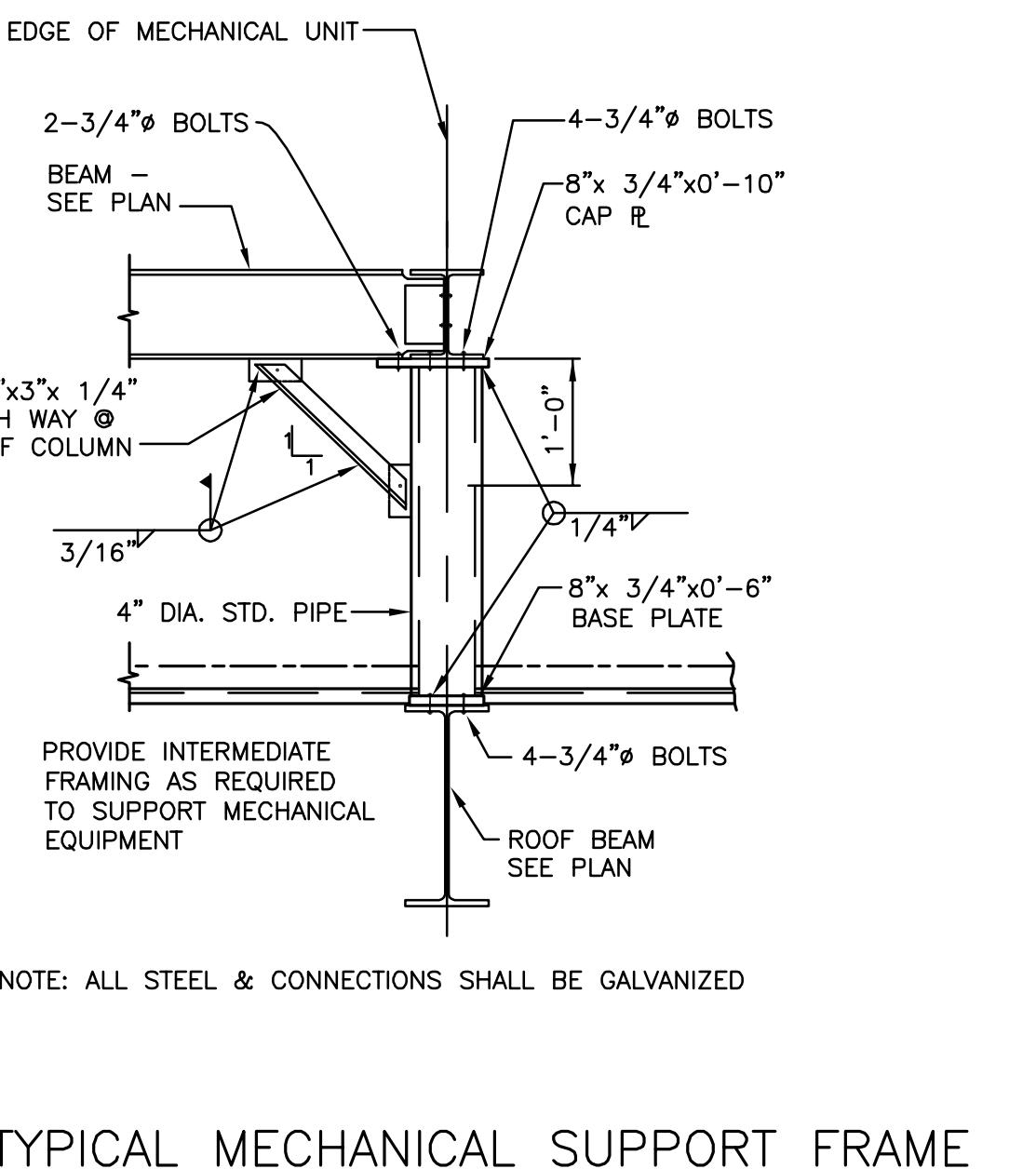
NOTE: CONN. PLATE & HEADED STUDS SHALL BE HOT-DIPPED GALV.



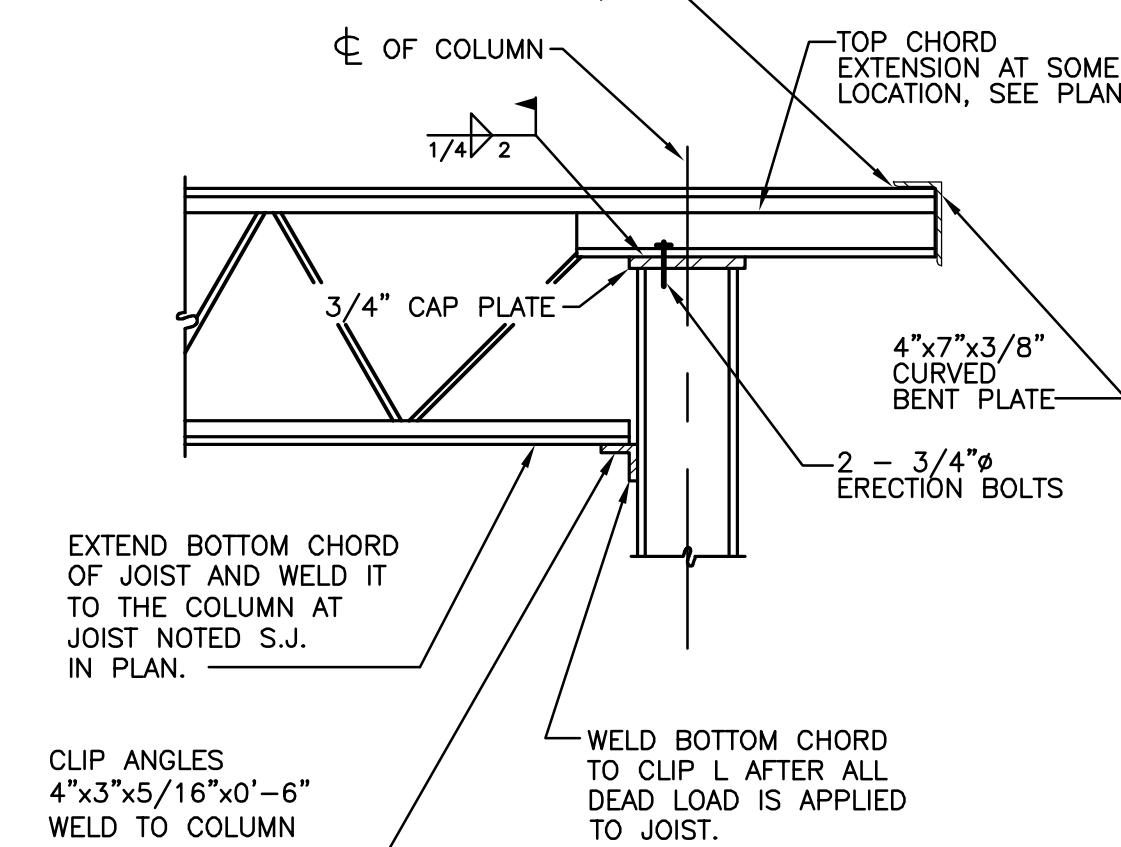
5 DETAIL
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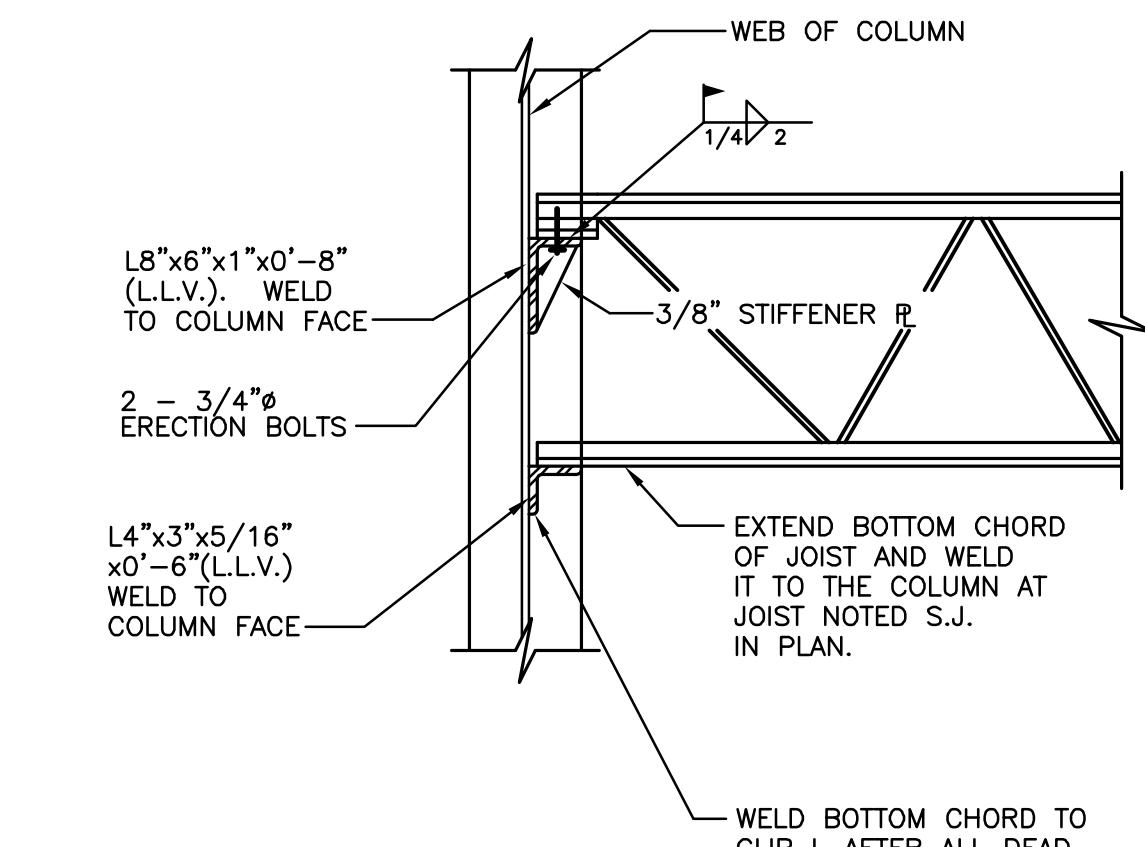
6 DETAIL
S2.5 NOT TO SCALE



TYPICAL MECHANICAL SUPPORT FRAME



TYPICAL STRUT JOIST TO COLUMN CONNECTION



TYPICAL STRUT JOIST TO COLUMN CONNECTION

7 DETAIL
S2.5 NOT TO SCALE

8 SECTION
S2.5 NOT TO SCALE

9 EMBEDDED PLATE CONNECTION DETAIL
S2.5 SCALE: 3/4" = 1'-0"

10 SECTION
S2.5 SCALE: 1 1/2" = 1'-0"

Typical Sections And Details

100% CONSTRUCTION DOCUMENTS

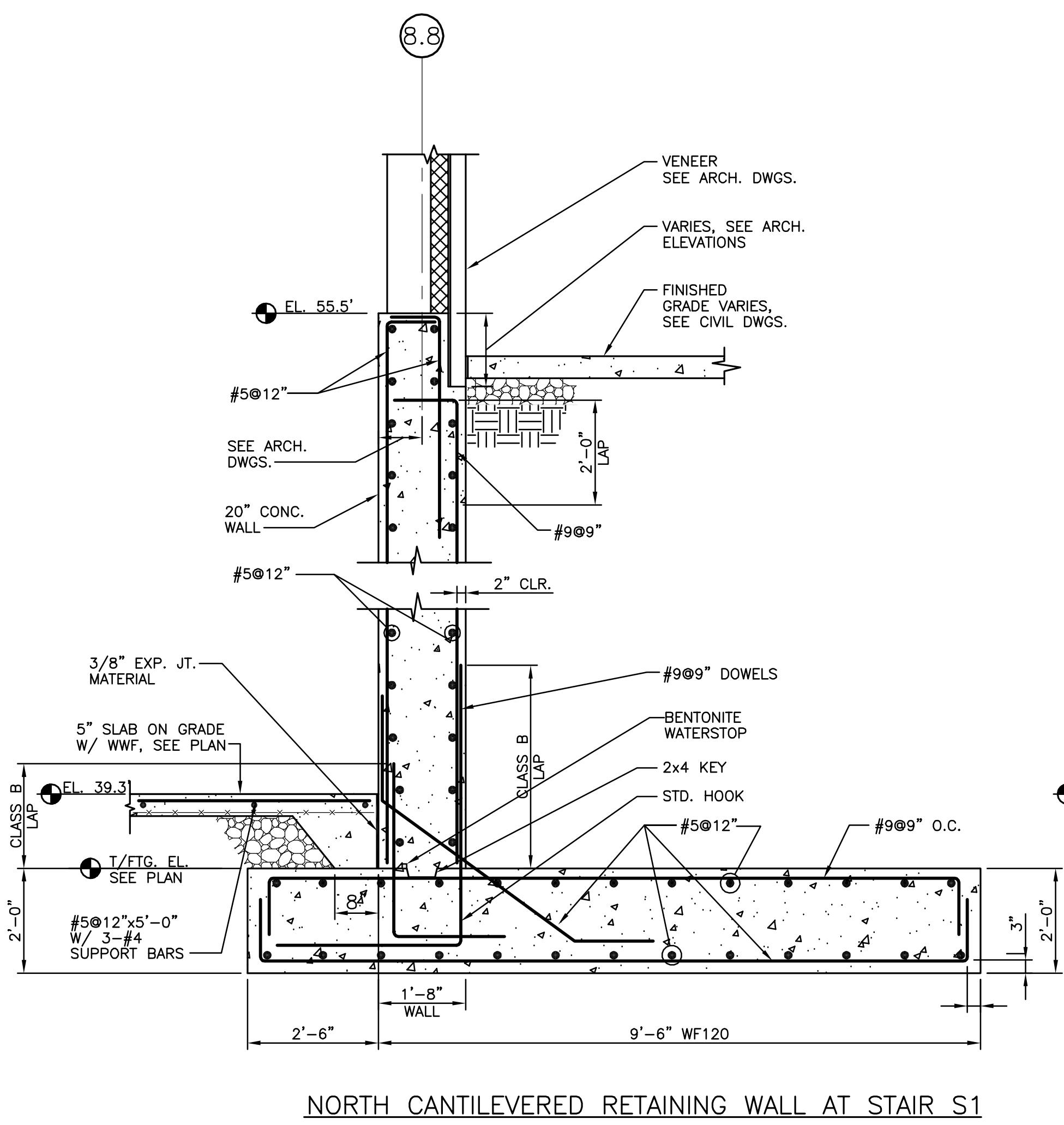
UMB PROJECT NO. 99-311
WTW PROJECT NO. 703-4091
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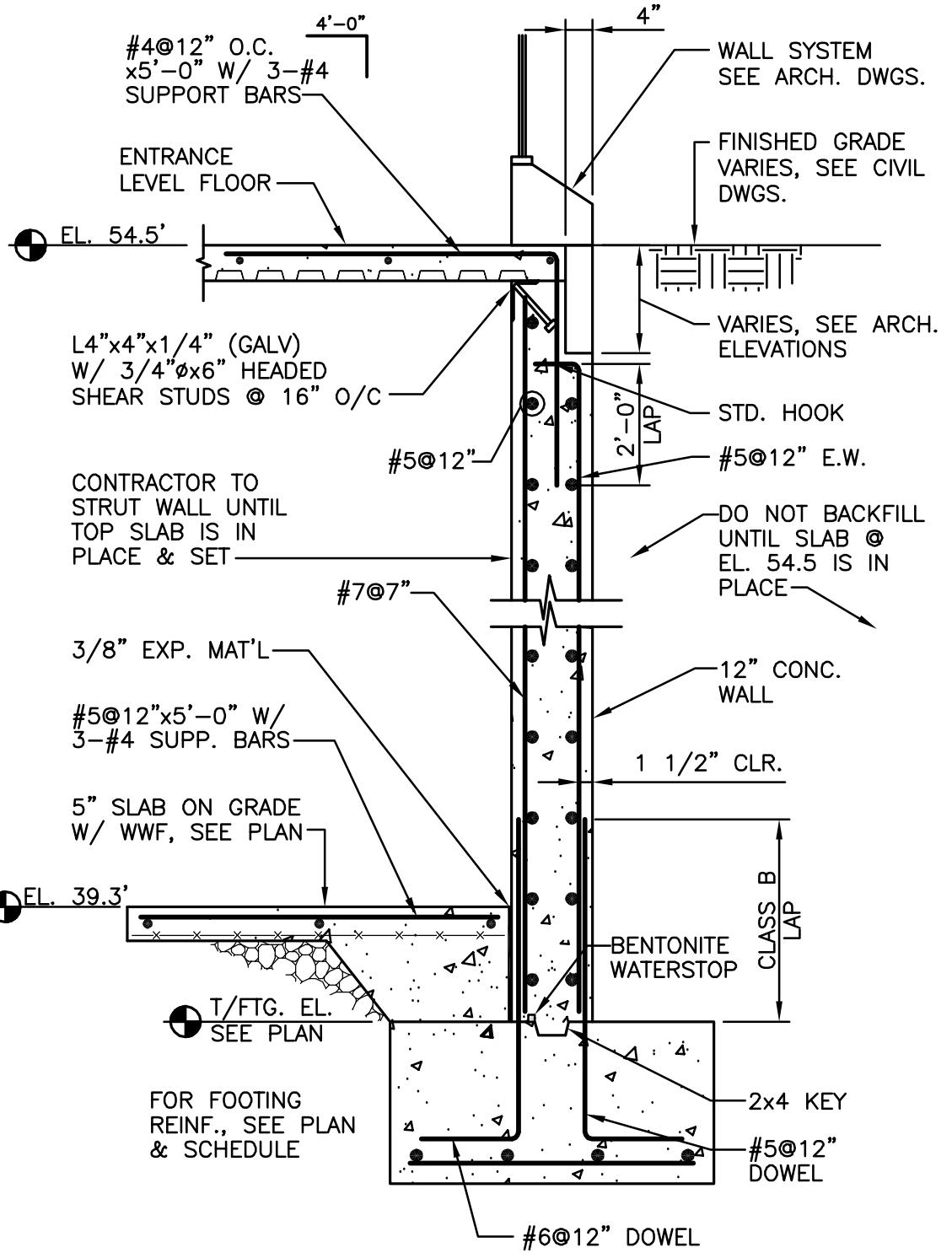
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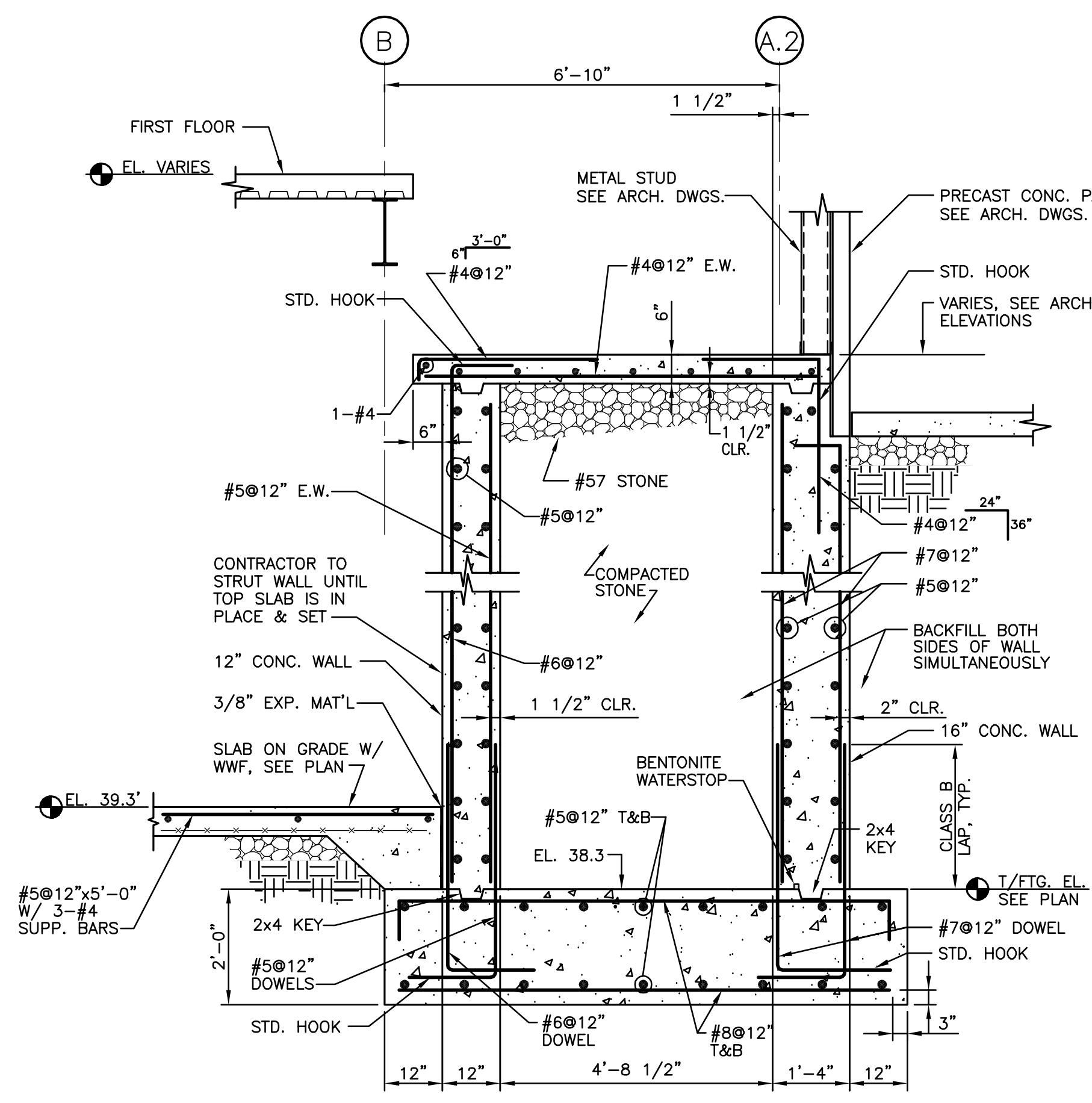




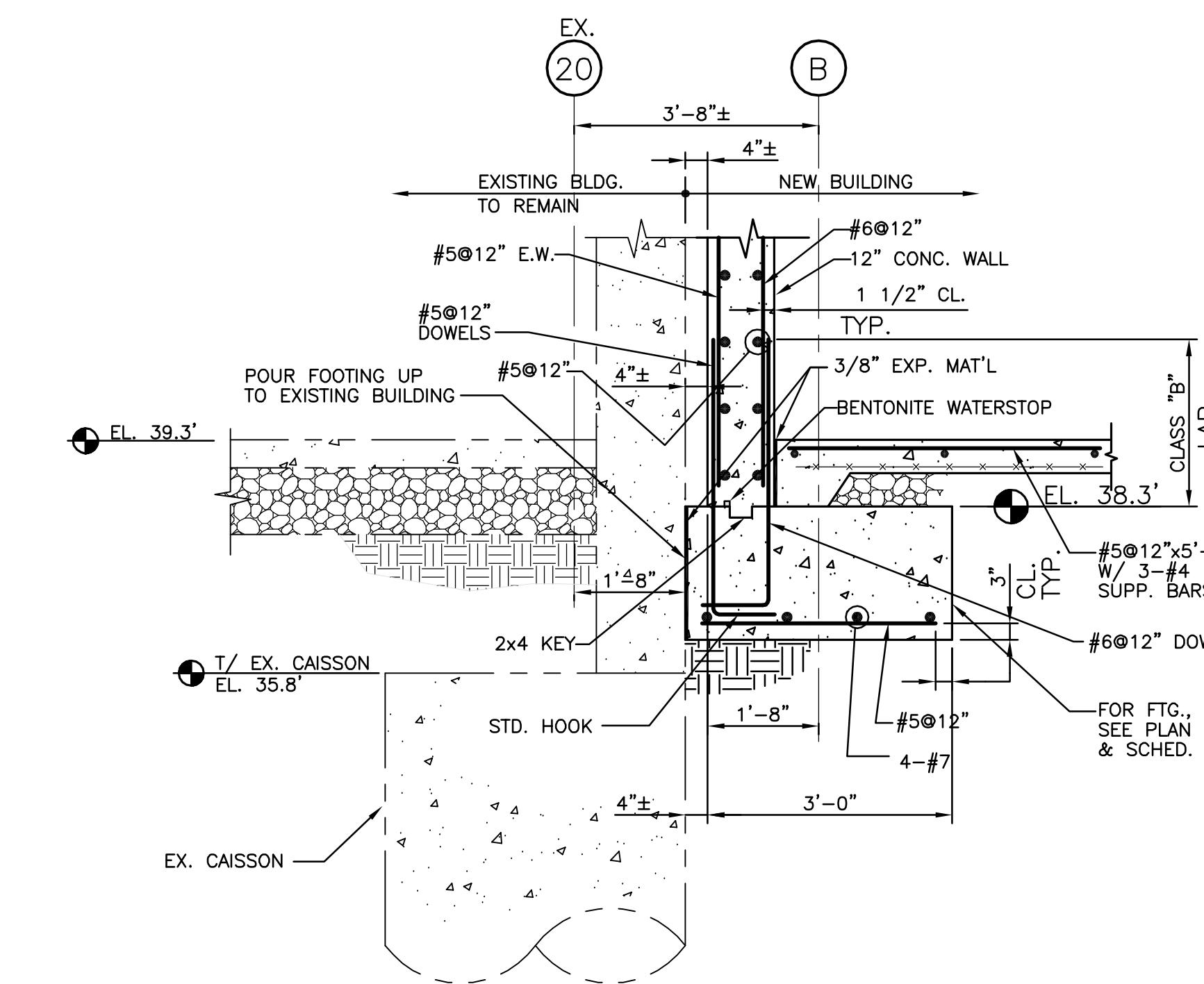
NORTH CANTILEVERED RETAINING WALL AT STAIR S1



BASEMENT WALL THRU
NORTHWEST CORNER



WEST RETAINING WALL @ RAMP



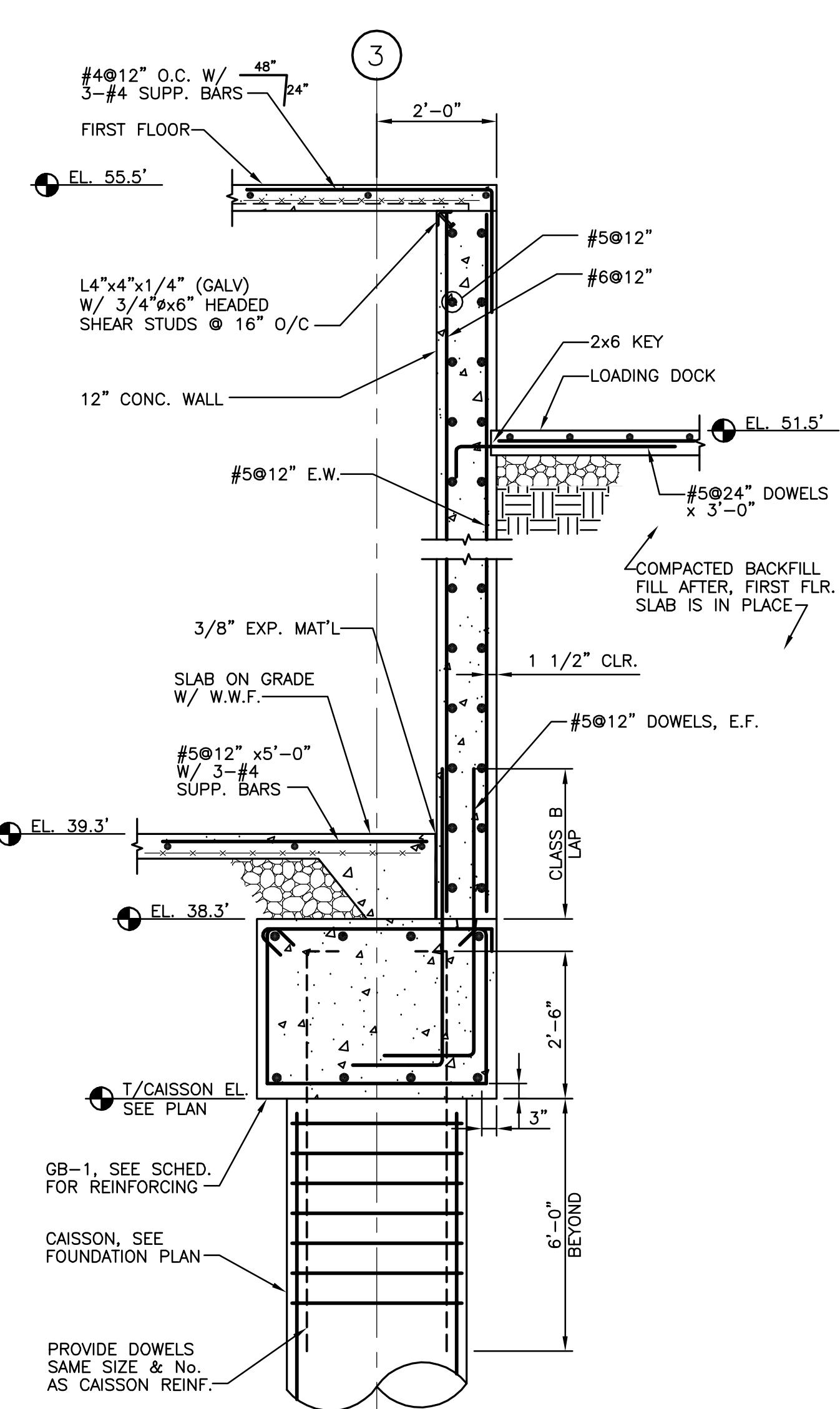
WEST BASEMENT WALL AT
SCHOOL OF NURSING

1 SECTION
S3.1 SCALE: 1/2"=1'-0"

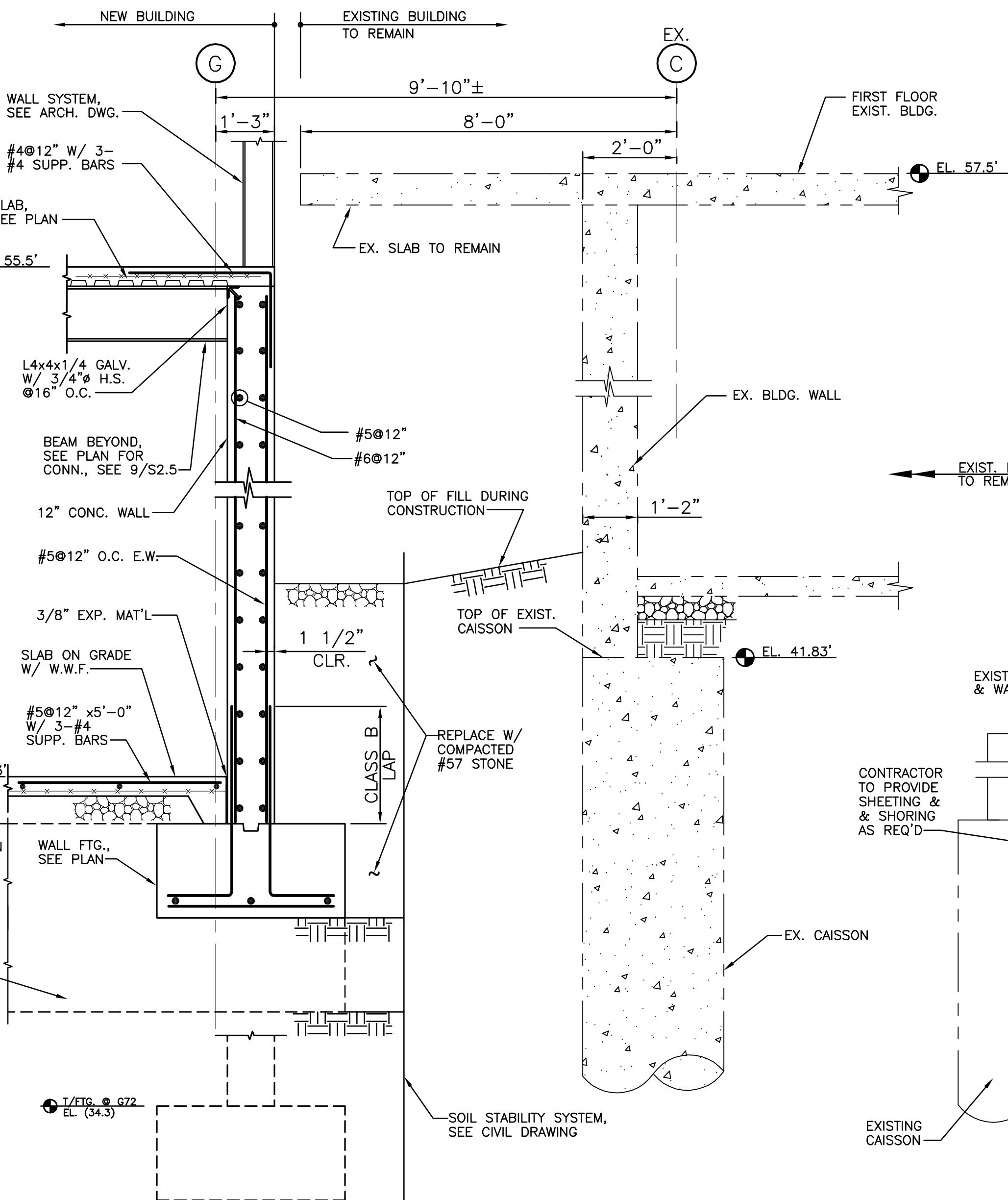
2 SECTION
S3.1 SCALE: 1/2"=1'-0"

3 SECTION
S3.1 SCALE: 1/2"=1'-0"

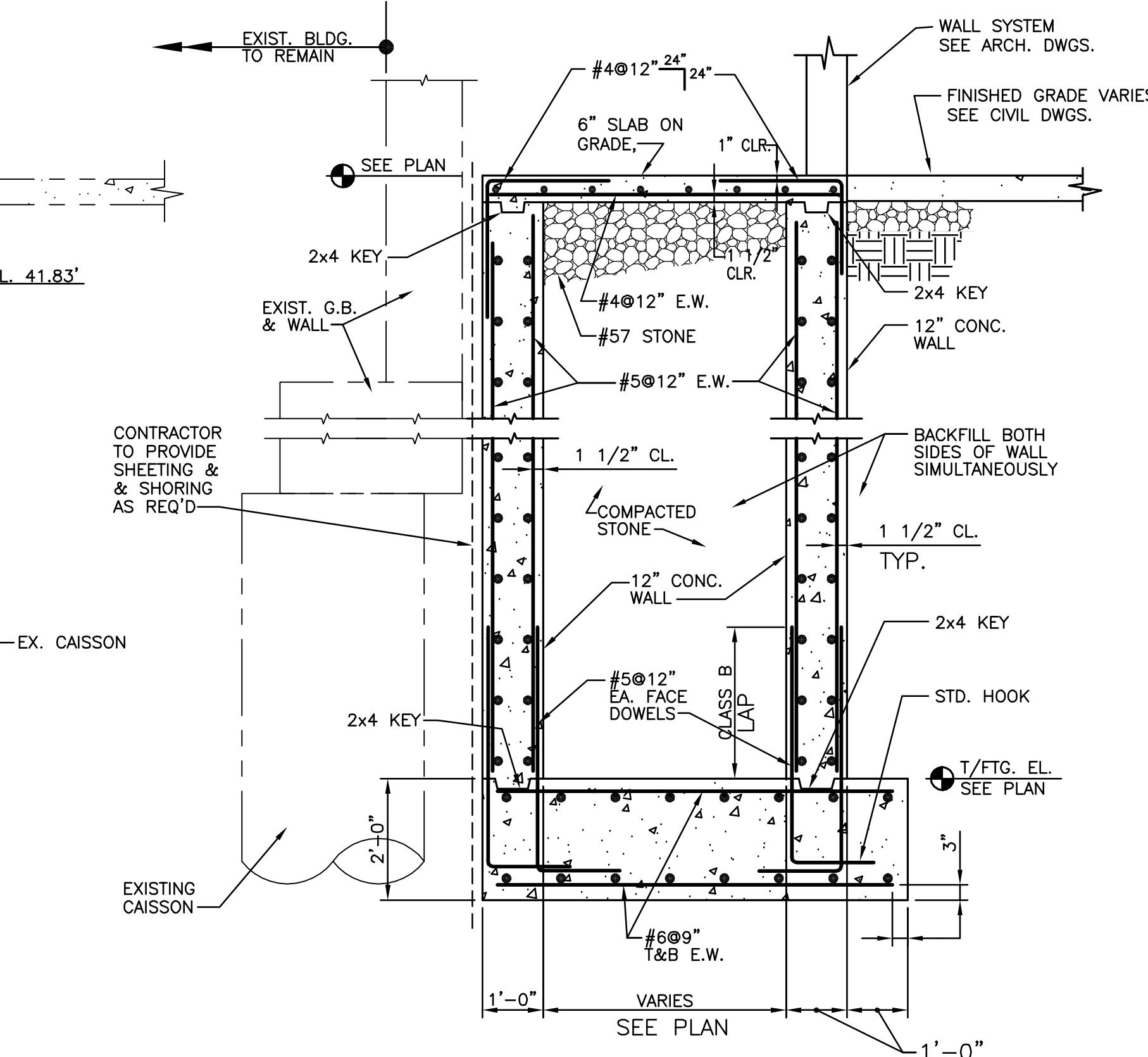
4 SECTION
S3.1 SCALE: 1/2"=1'-0"



SOUTH BASEMENT WALL



EAST BASEMENT WALL
THROUGH HS/HS LIBRARY



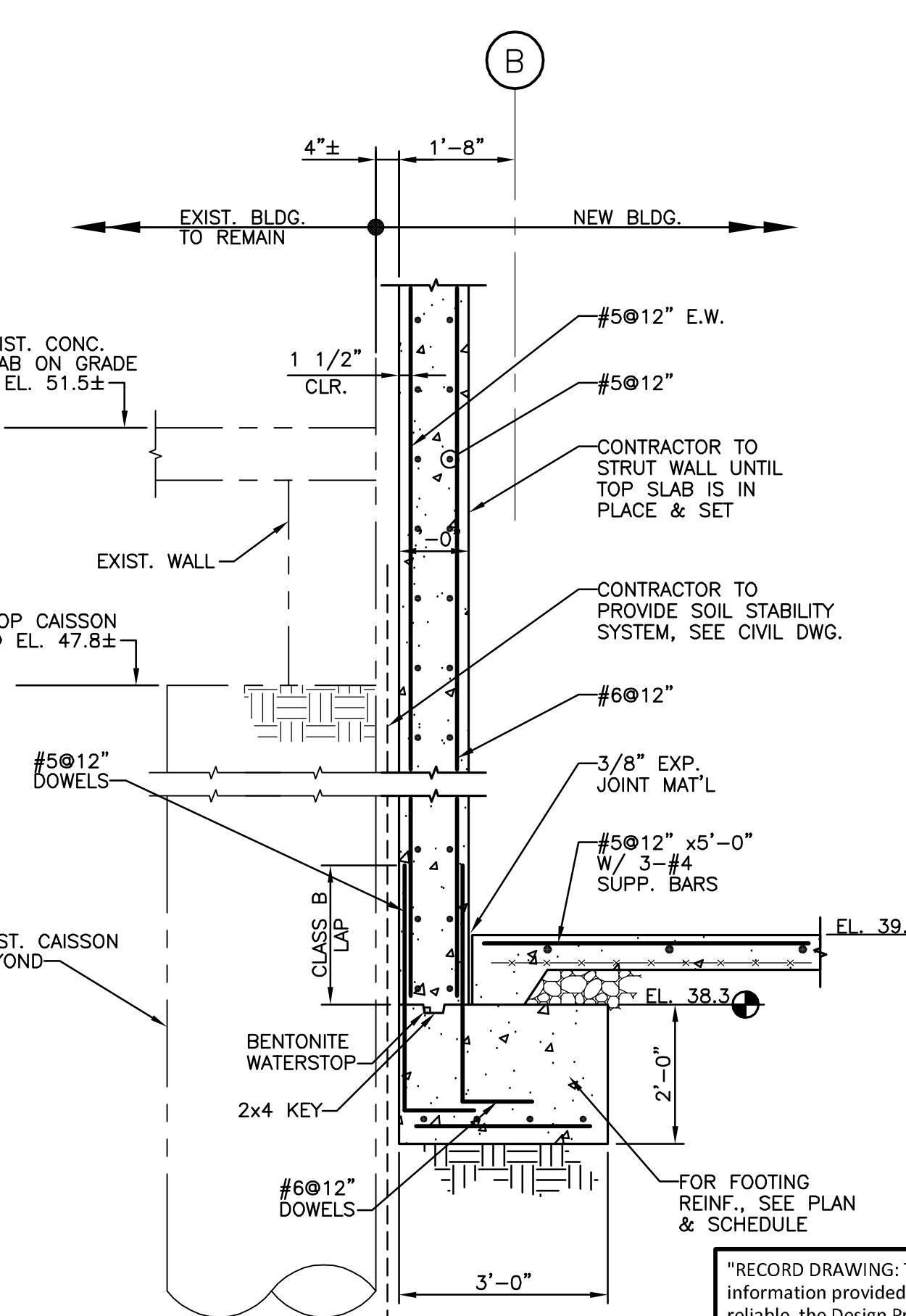
BASEMENT WALL THRU SCHOOL
OF NURSING NEW VESTIBULE

5 SECTION
S3.1 SCALE: 1/2"=1'-0"

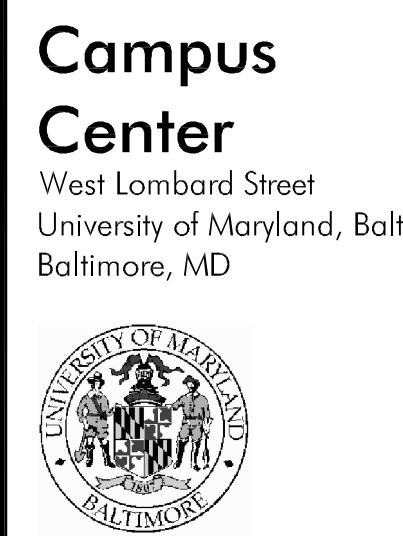
6 SECTION
S3.1 SCALE: 1/2"=1'-0"

7 SECTION
S3.1 SCALE: 1/2"=1'-0"

8 SECTION
S3.1 SCALE: 1/2"=1'-0"



2 01/29/10 RECORD DRAWINGS
 1 04/09/07 BULLETIN #3
 0 02/16/07 ISSUE FOR BID
 Rev. Date: Comment:
 Issued: FEBRUARY 16, 2007



Sections And Details

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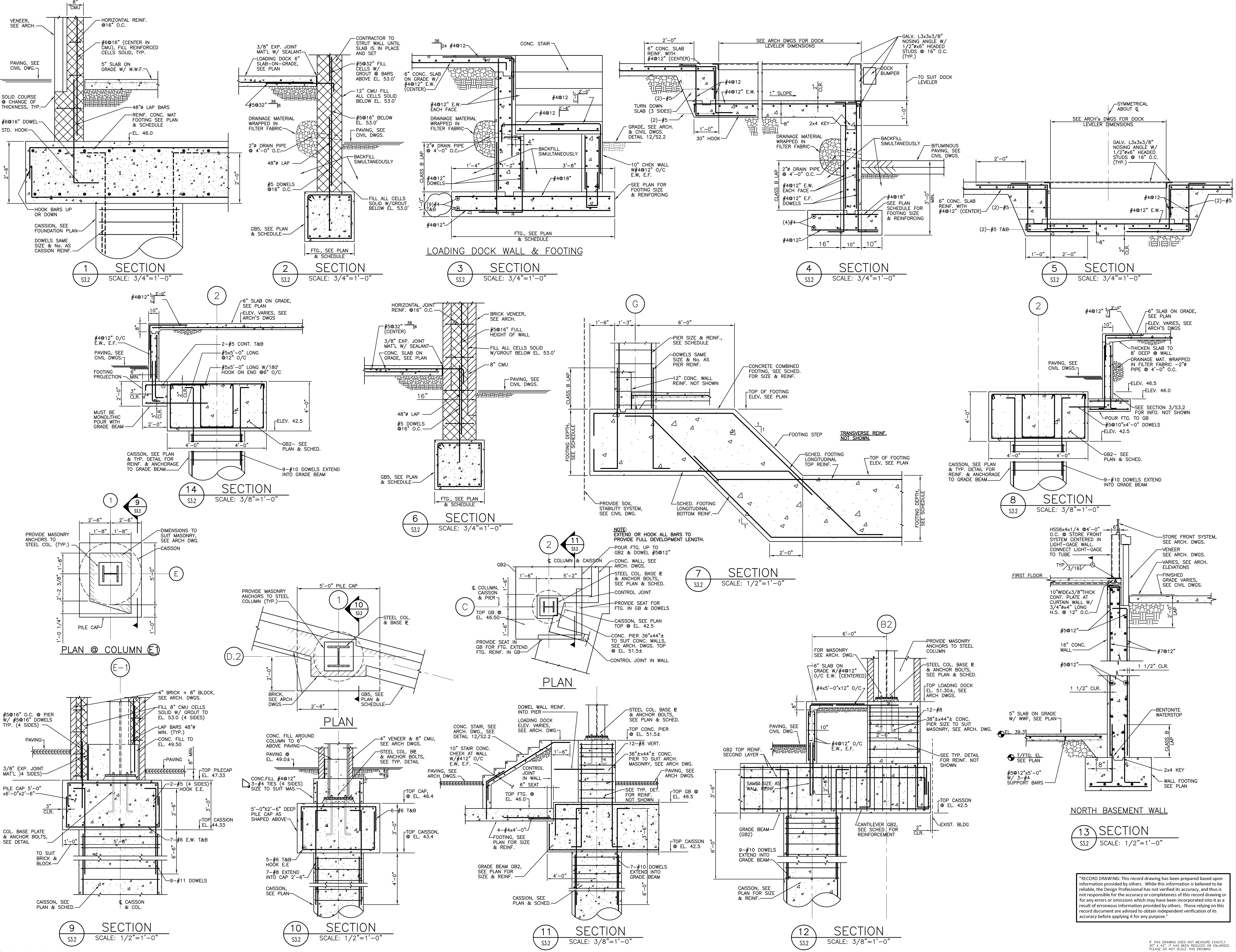
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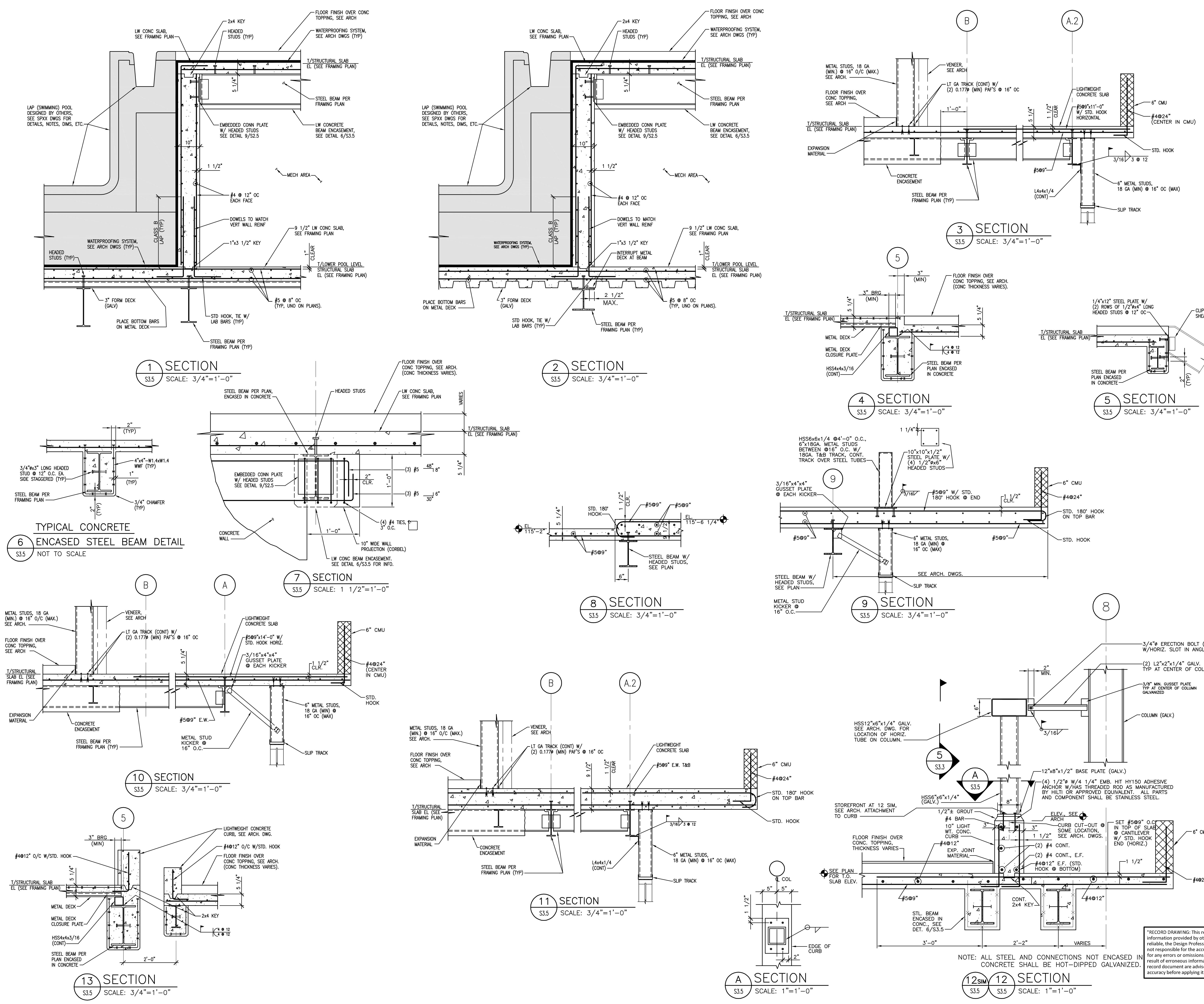
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53.2

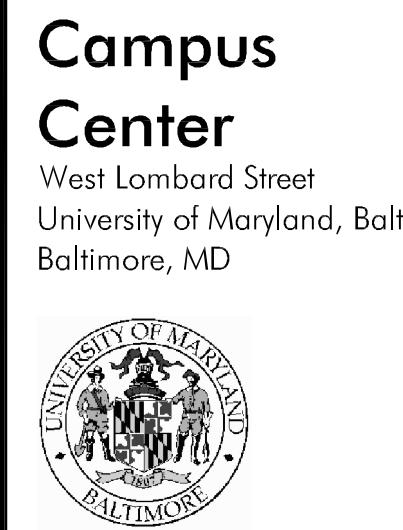
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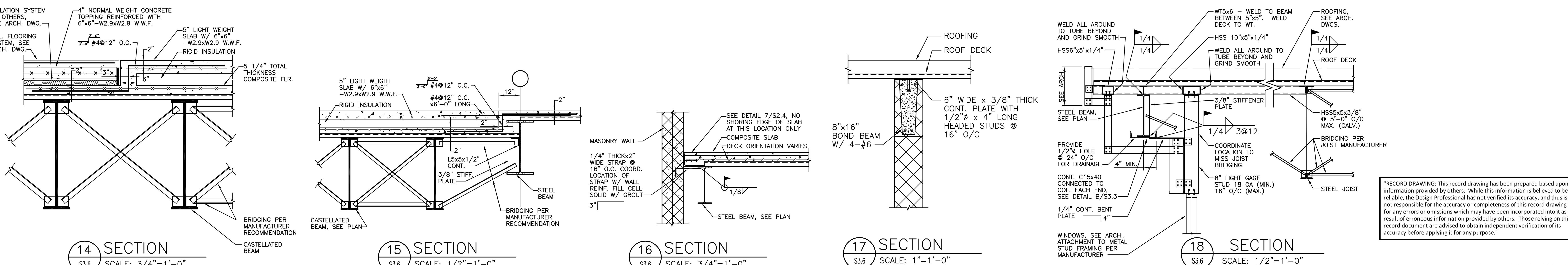
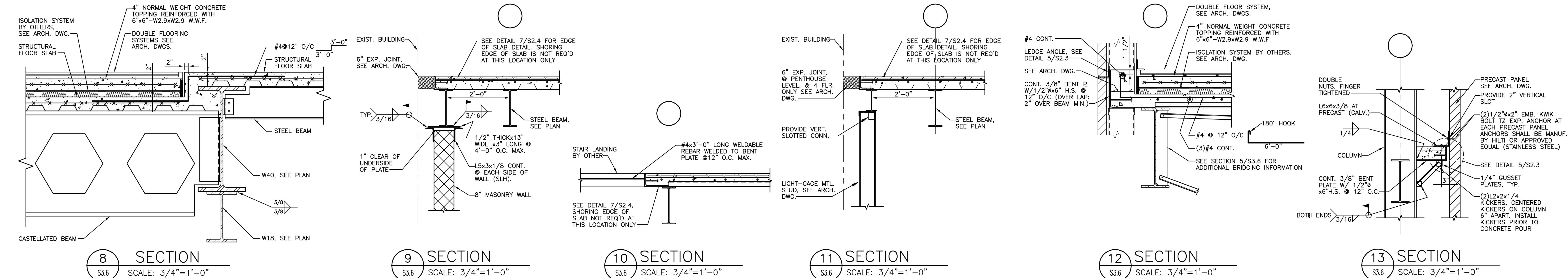
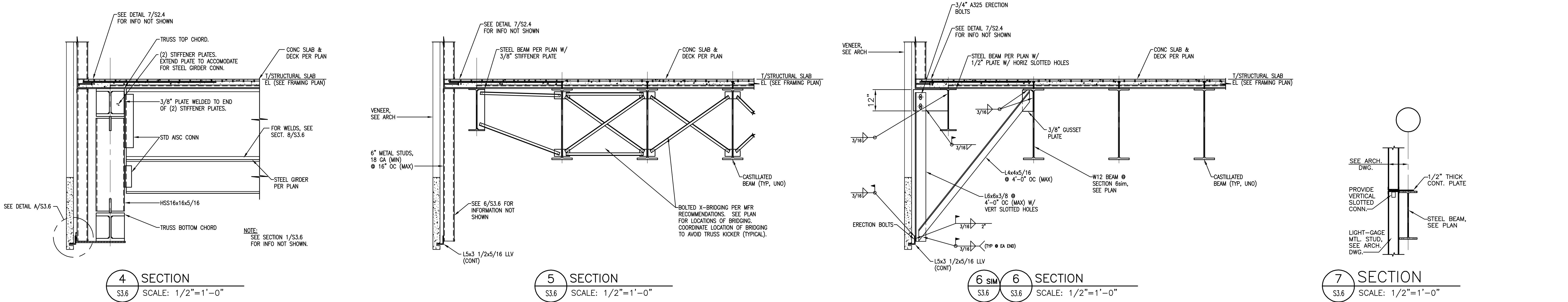
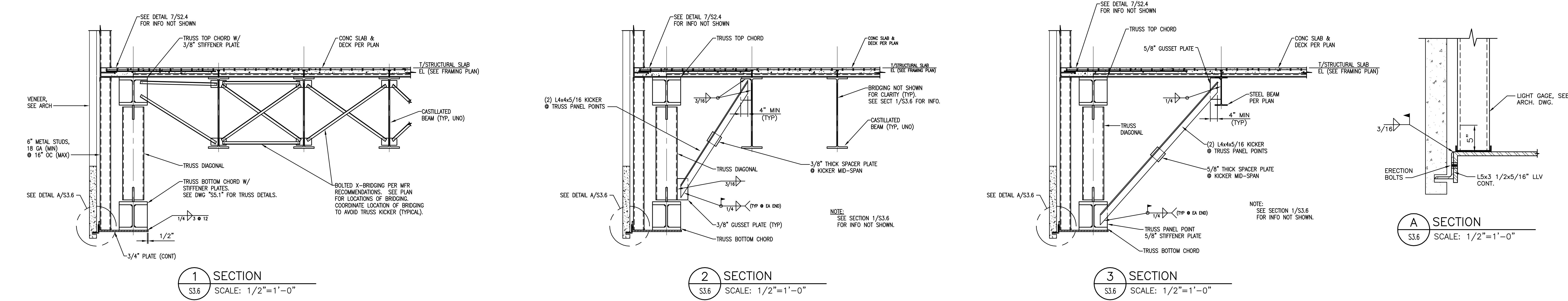
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S3.5



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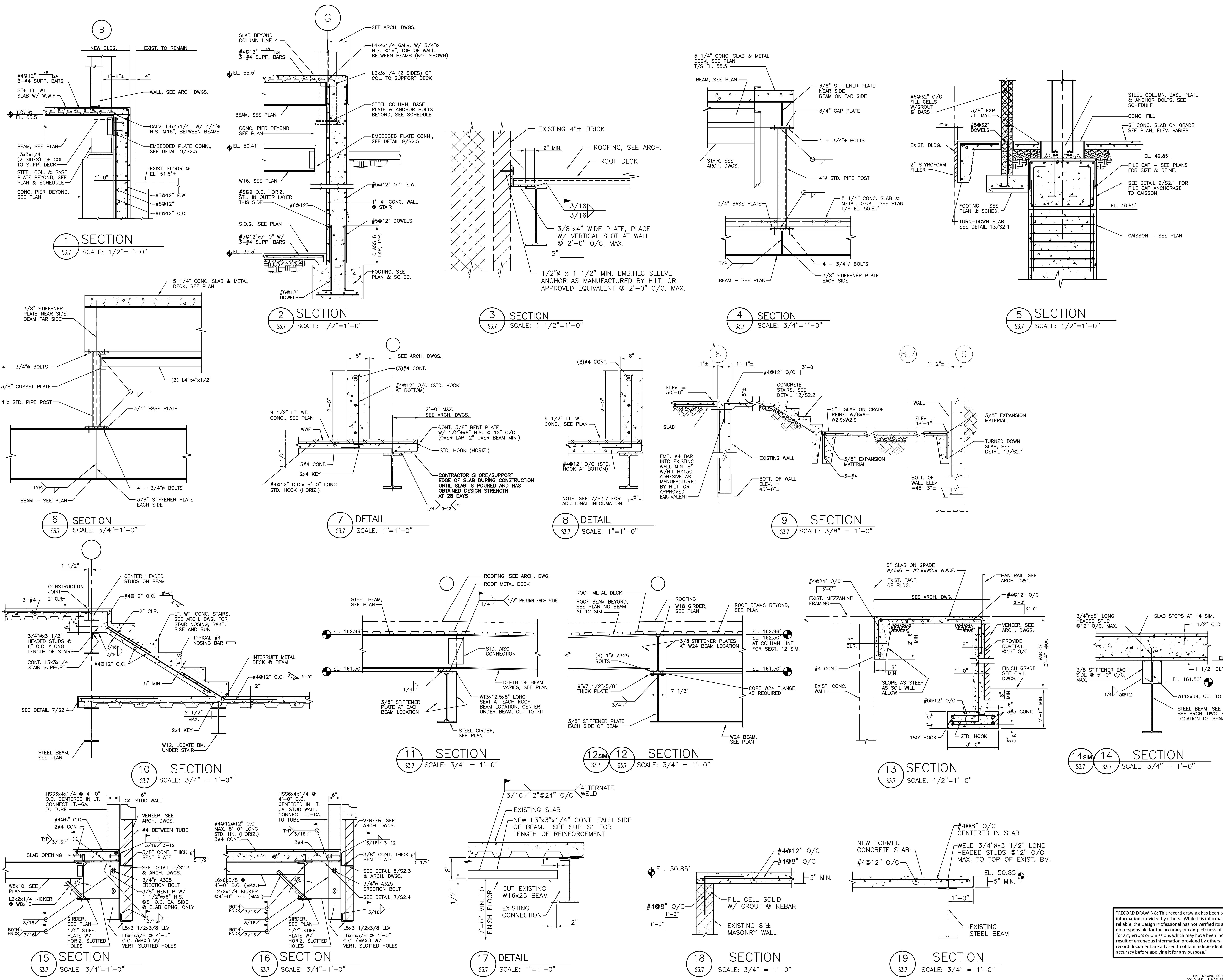


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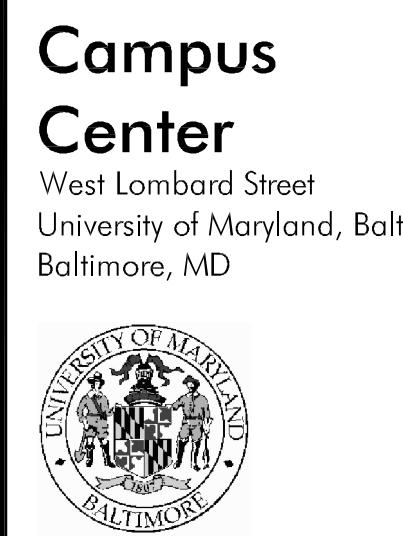
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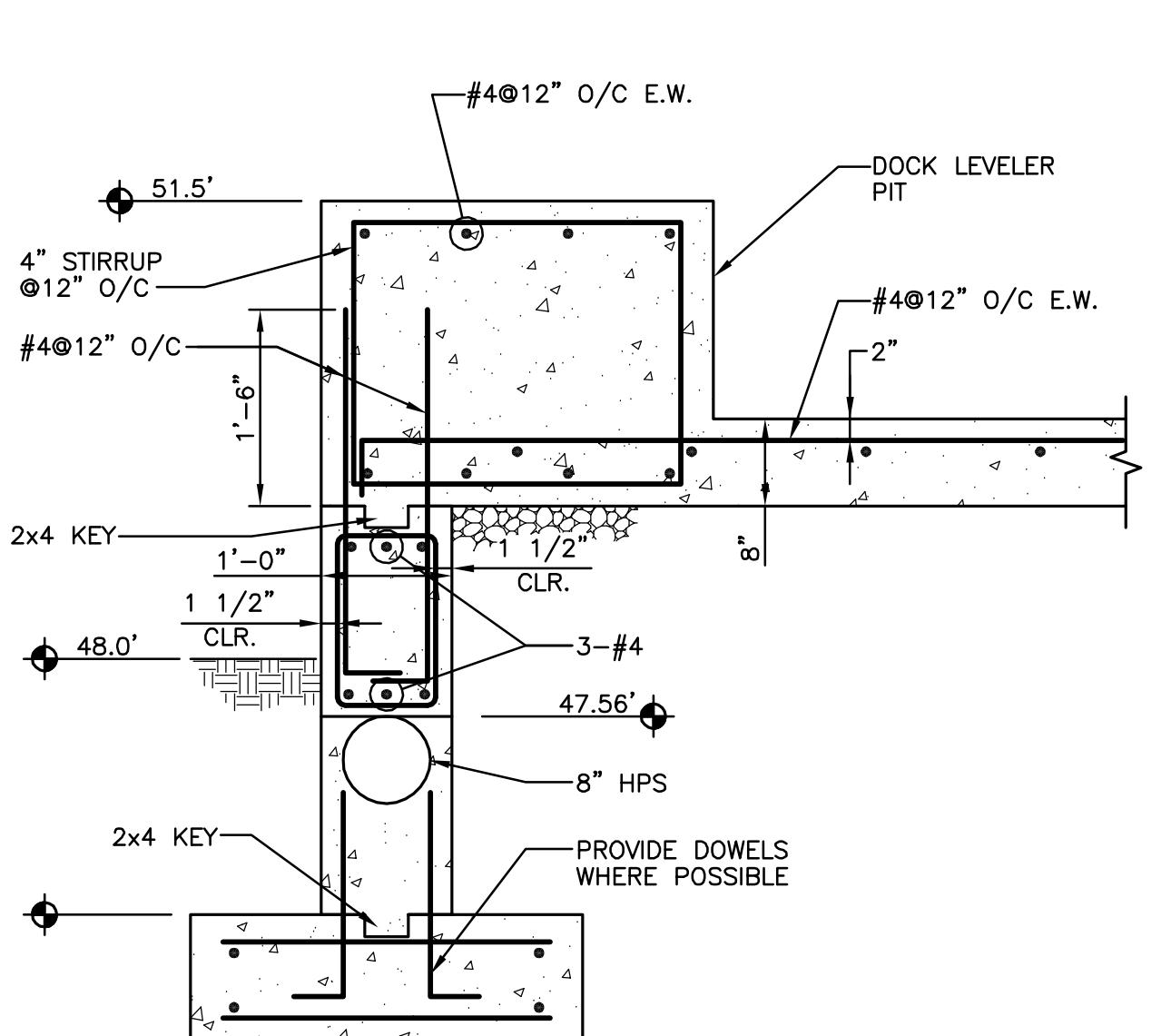


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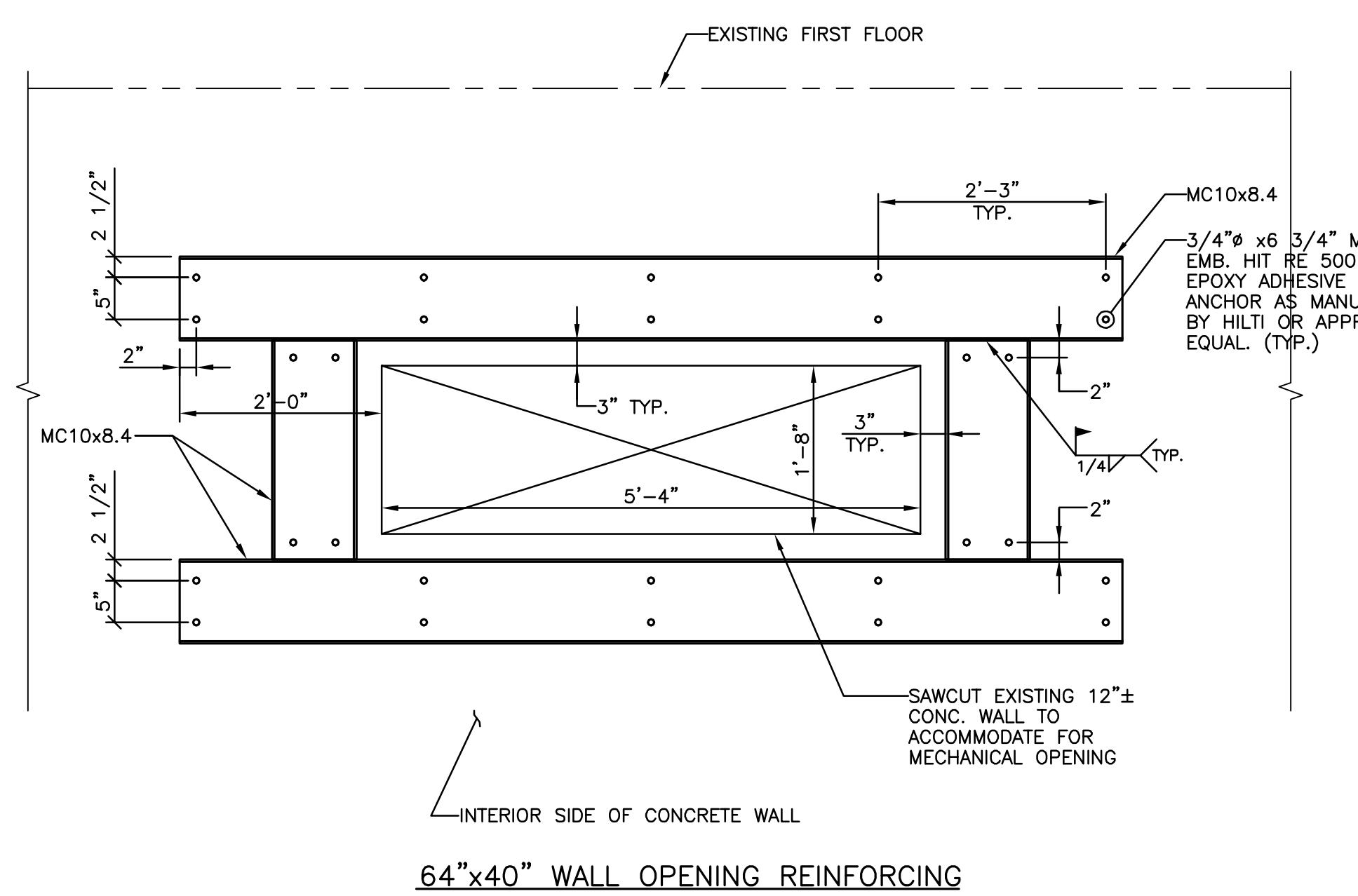
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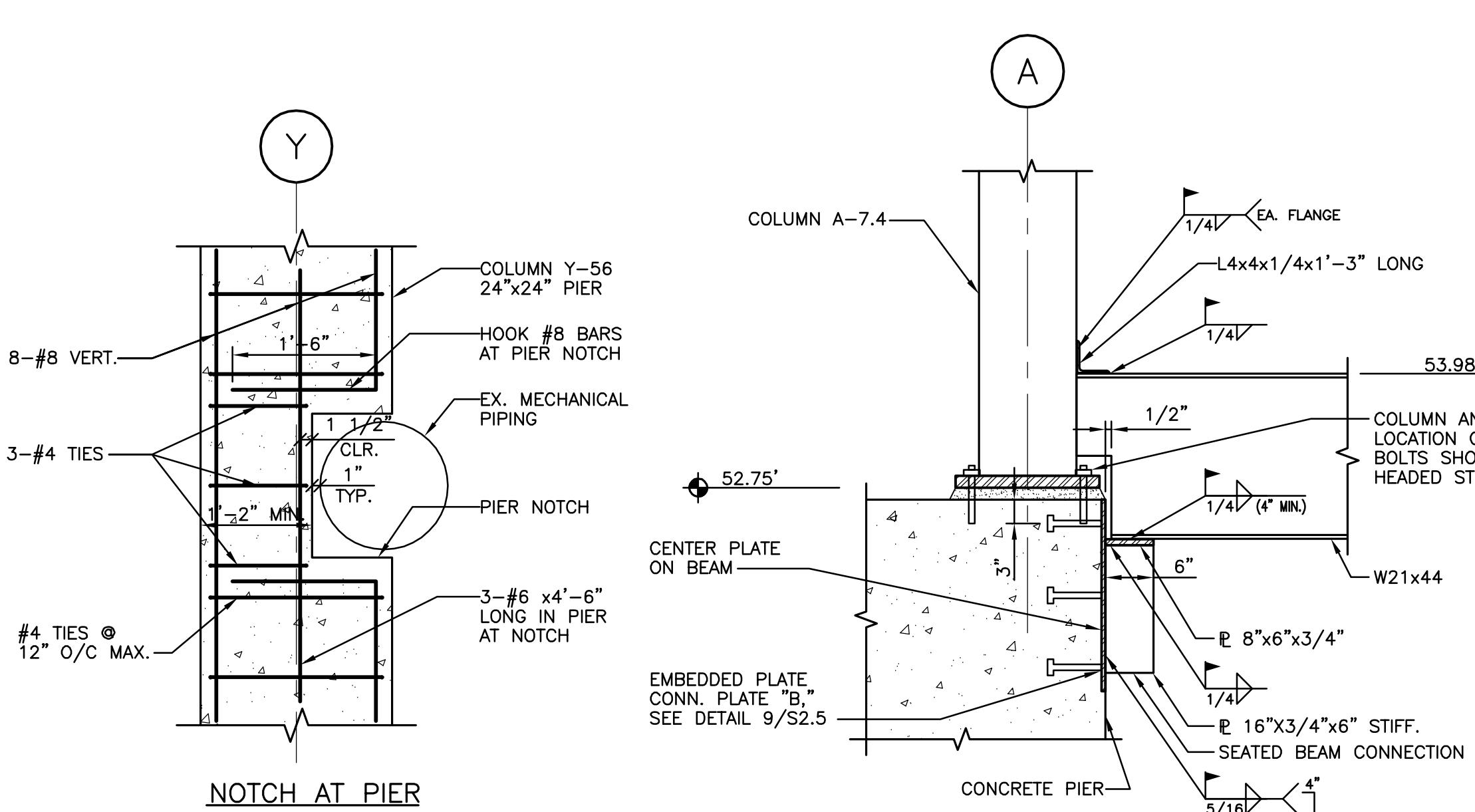
S3.8



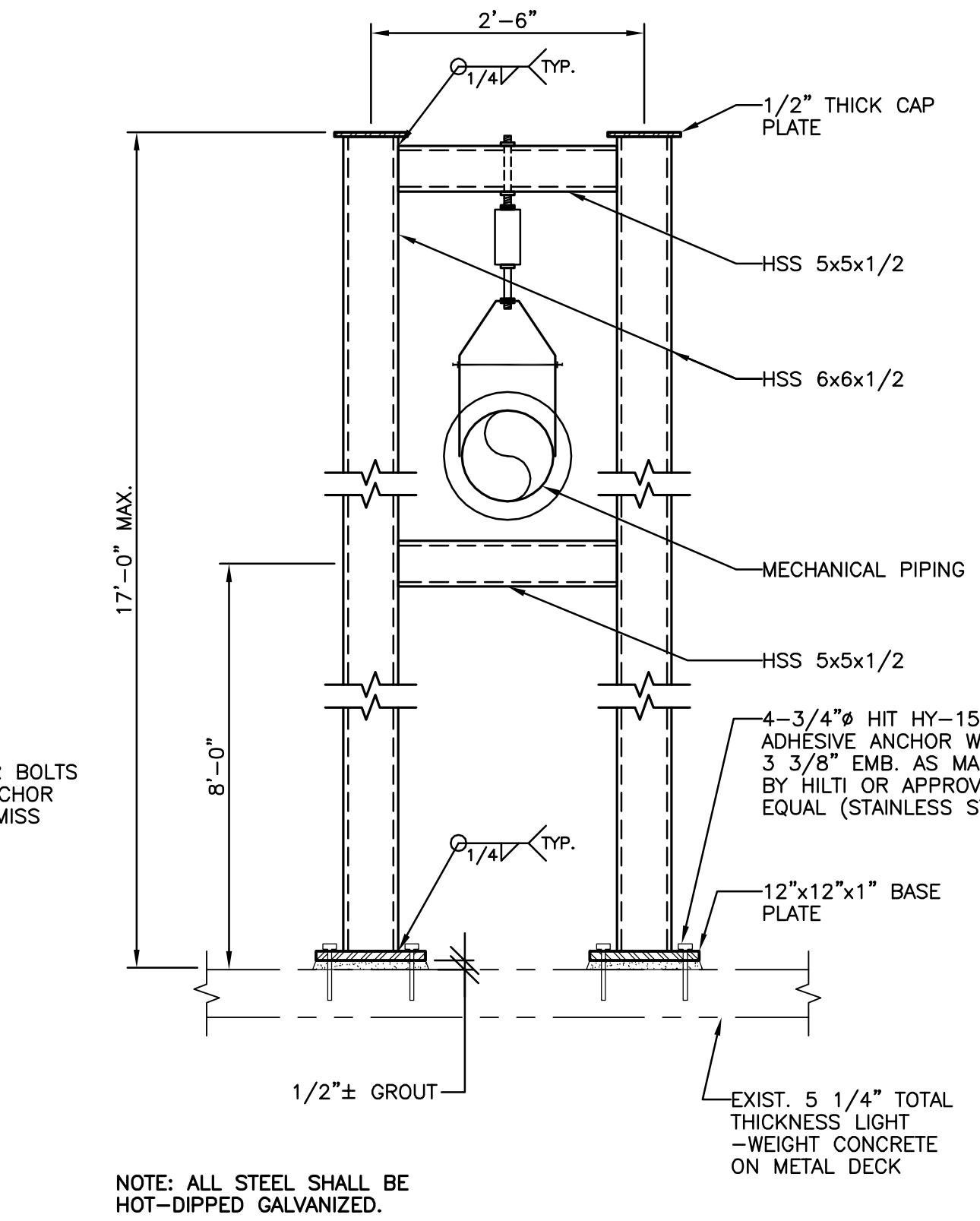
SECTION
 S3.8 SCALE: 3/4" = 1'-0"



DETAIL
 S3.8 SCALE: 3/4" = 1'-0"

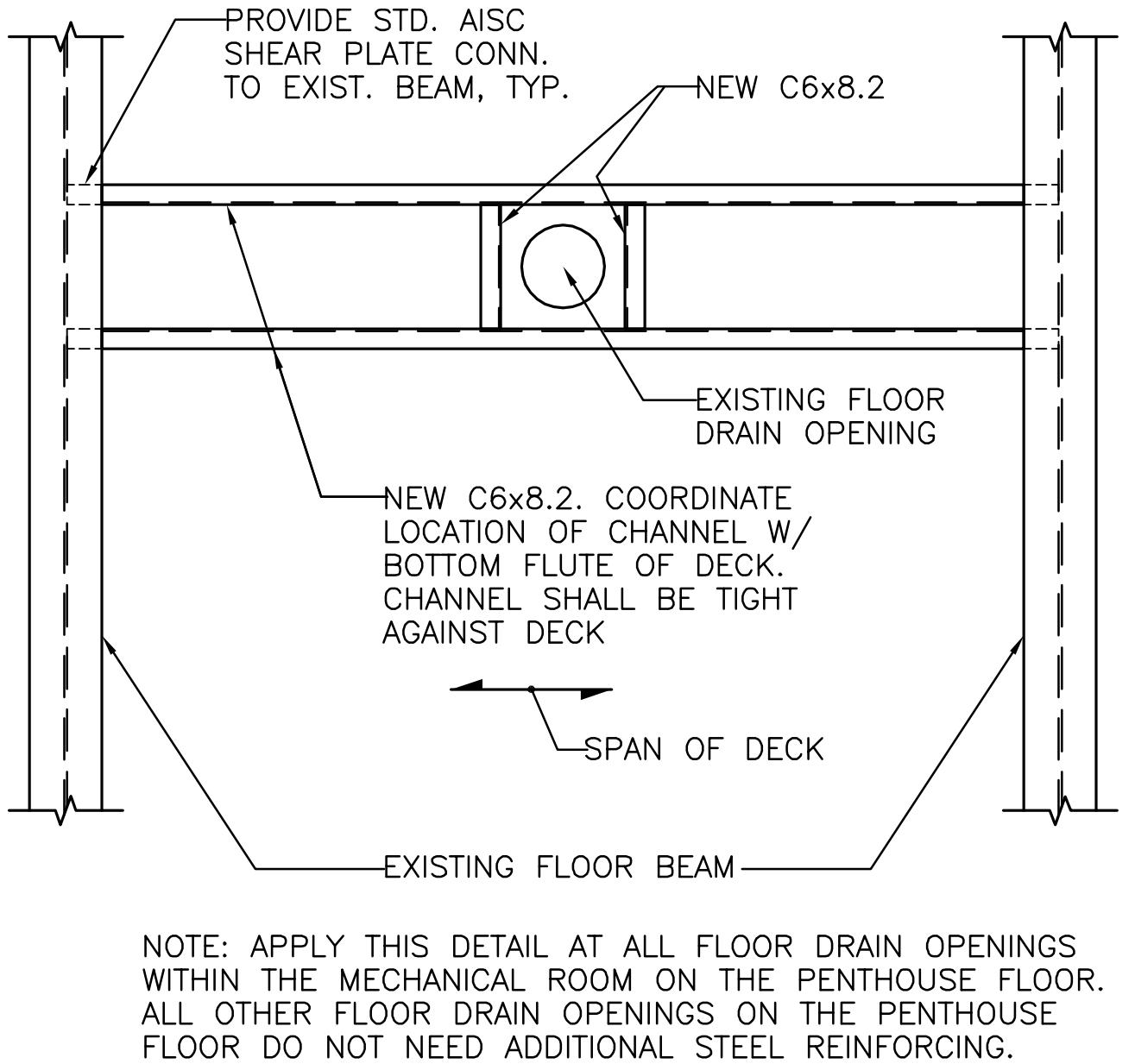


DETAIL
 S3.8 SCALE: 3/4" = 1'-0"

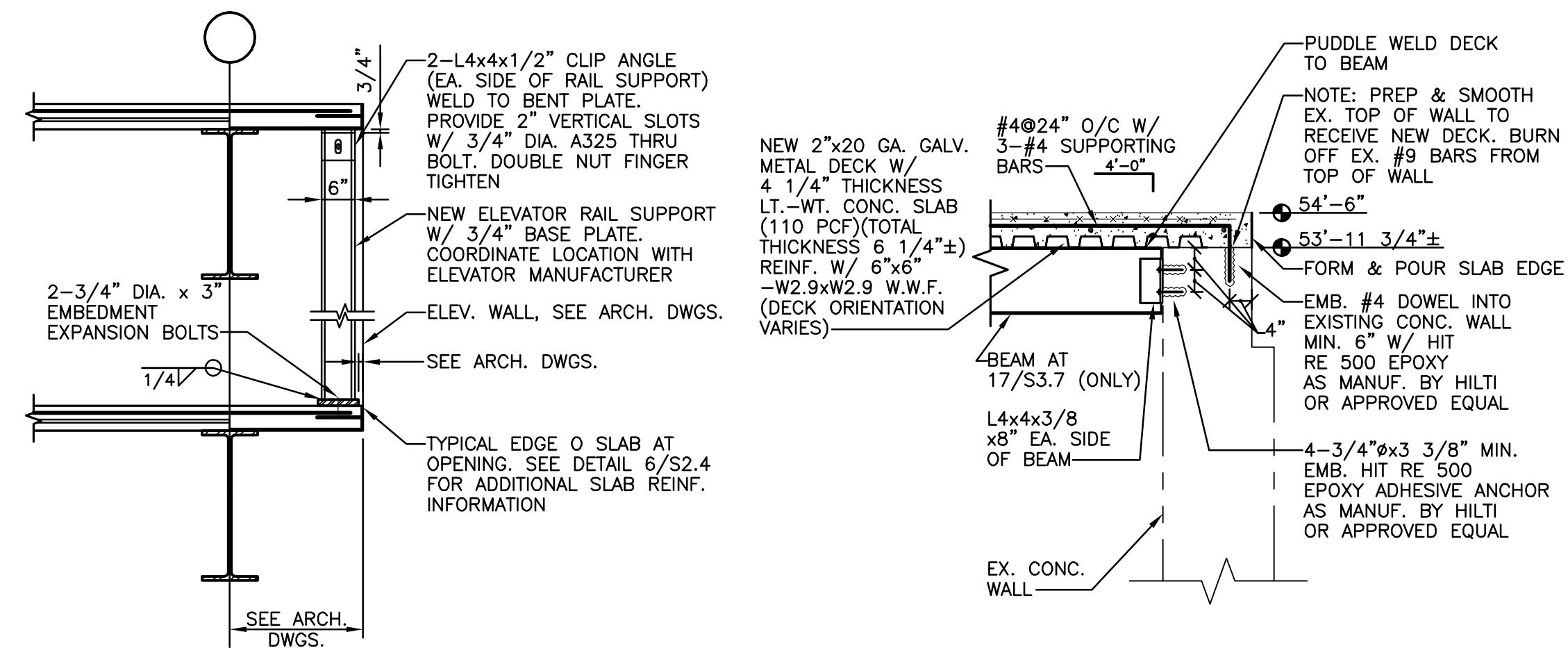


SECTION
 S3.8 SCALE: 3/4" = 1'-0"

TYPICAL PIPE SUPPORT
 S3.8 SCALE: 3/4" = 1'-0"



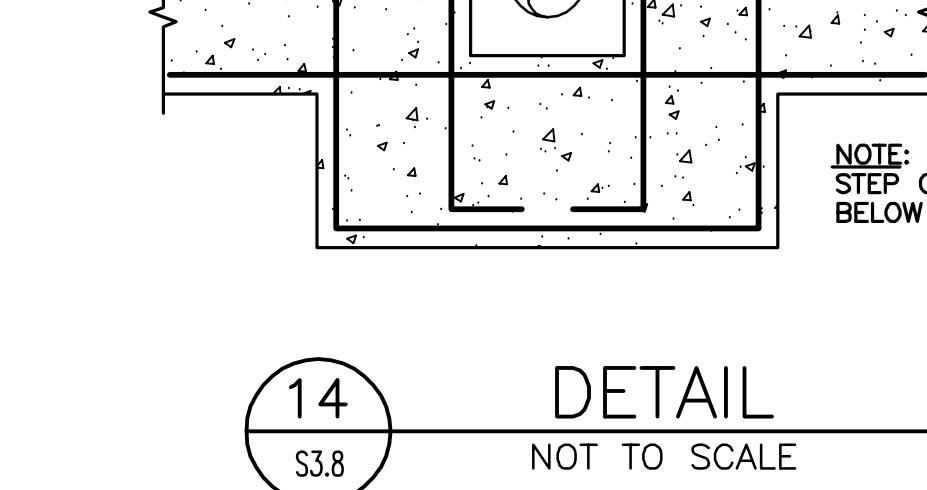
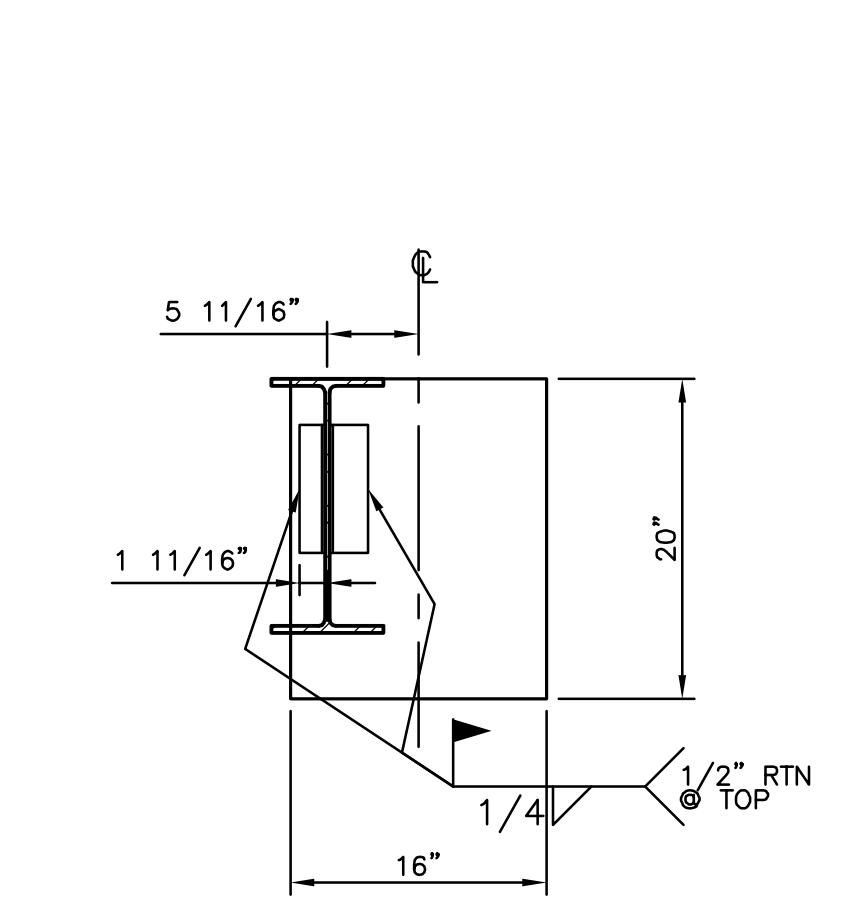
TYPICAL STEEL REINFORCING
 S3.8 NOT TO SCALE



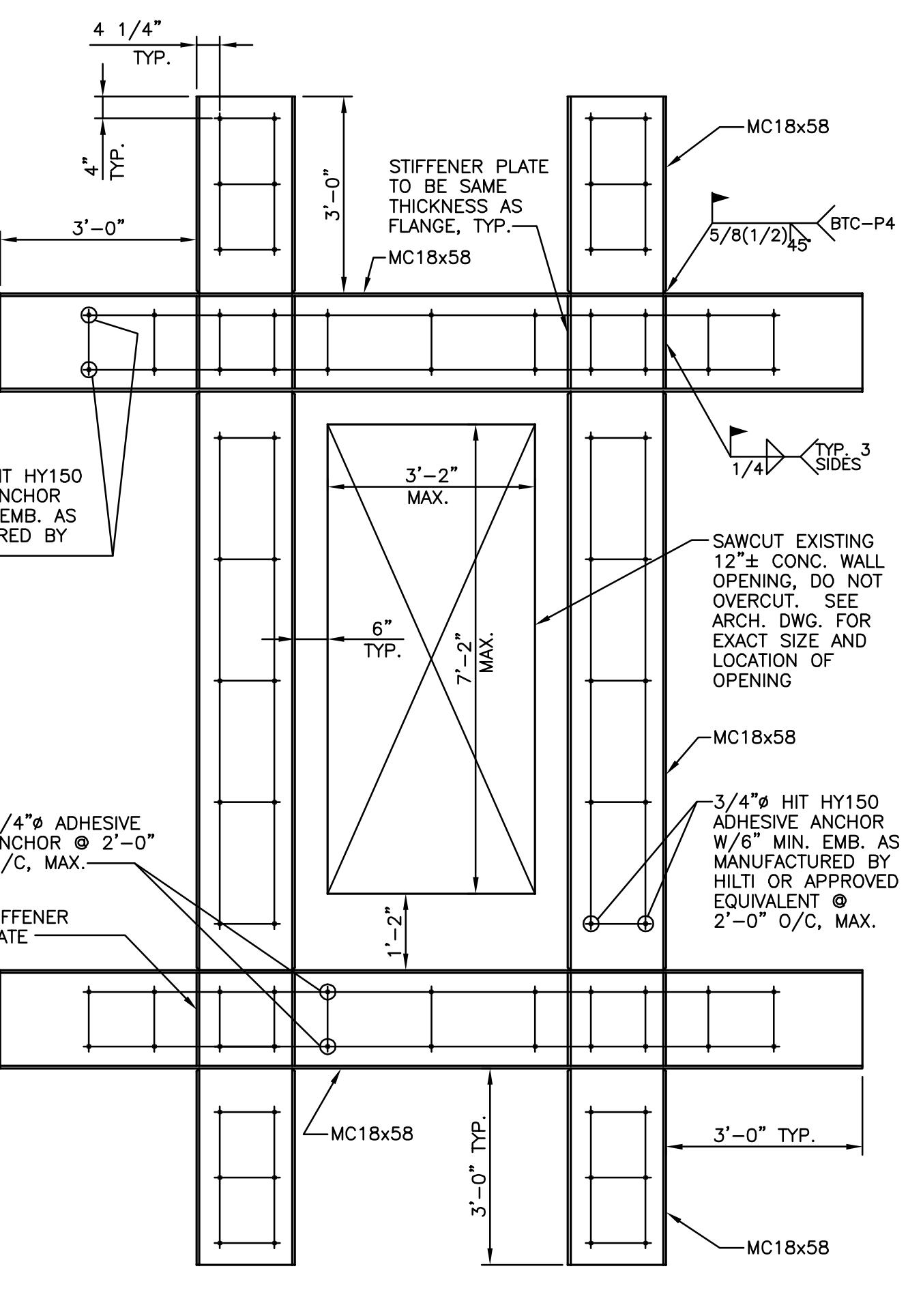
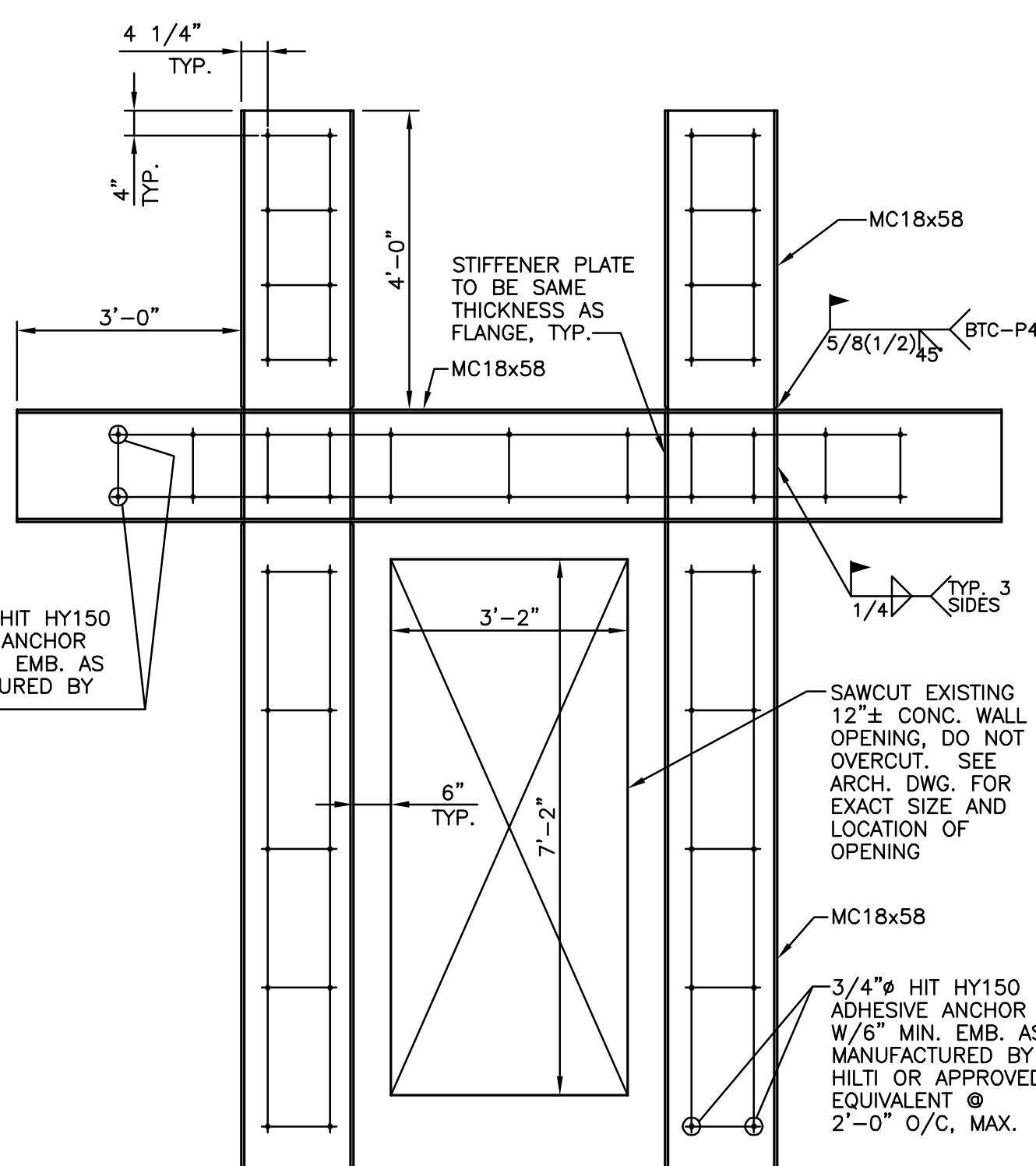
SECTION
 S3.8 SCALE: 3/4" = 1'-0"

SECTION
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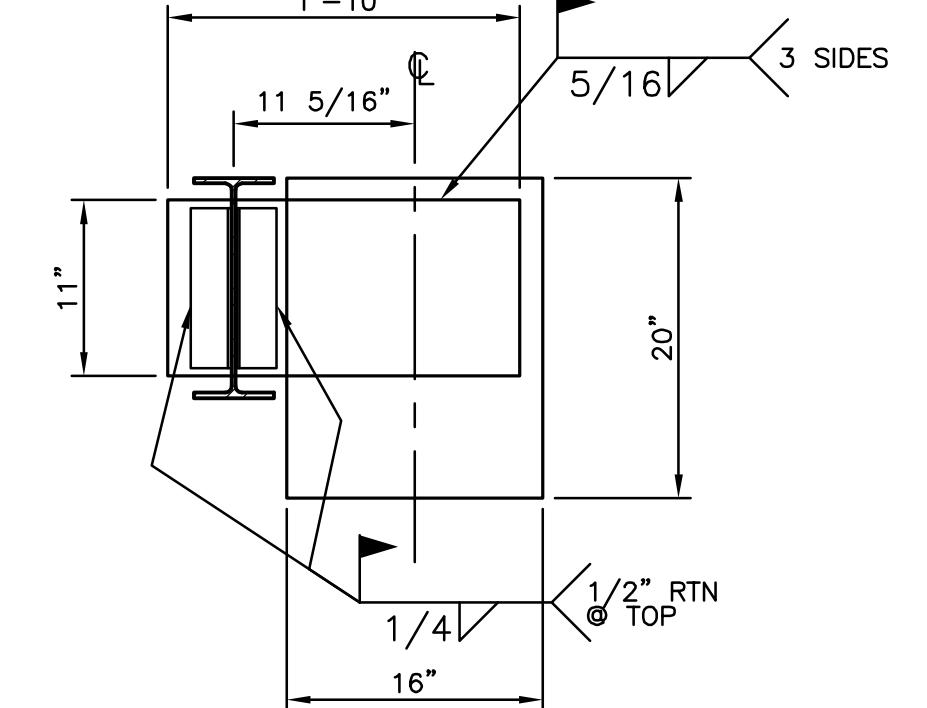
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 S3.8 S3.8 SCALE: 1/2" = 1'-0"



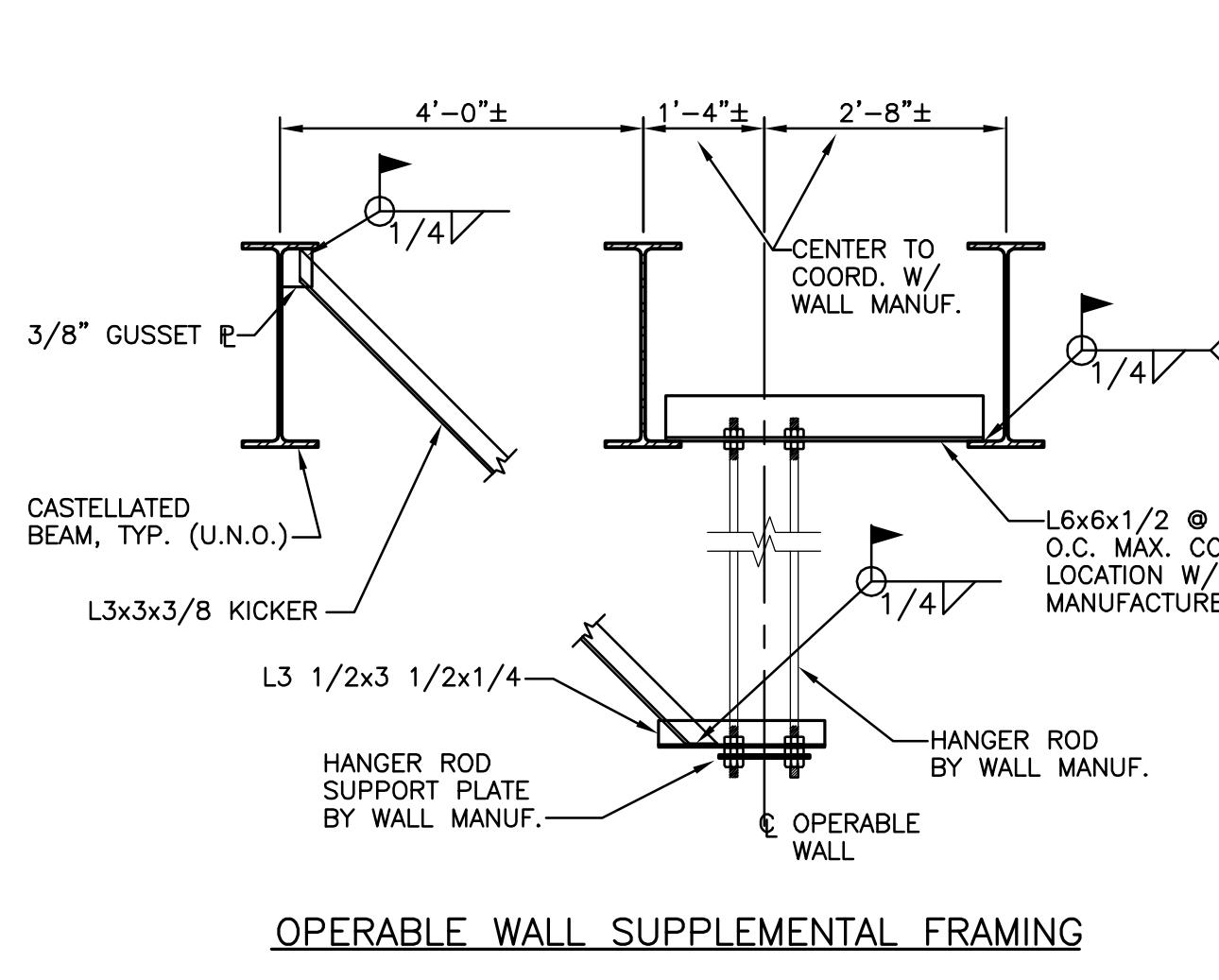
DETAIL
 S3.8 NOT TO SCALE



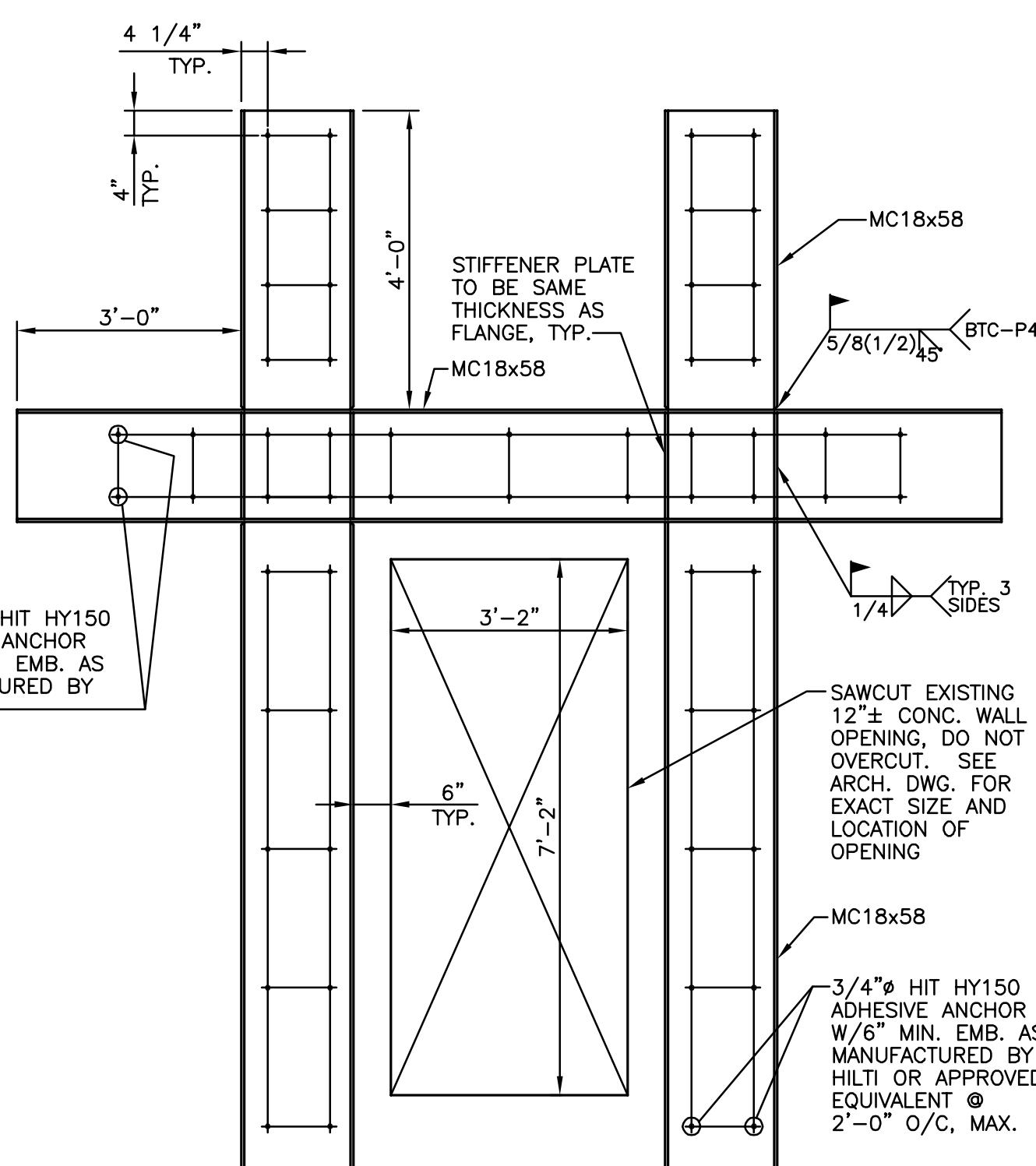
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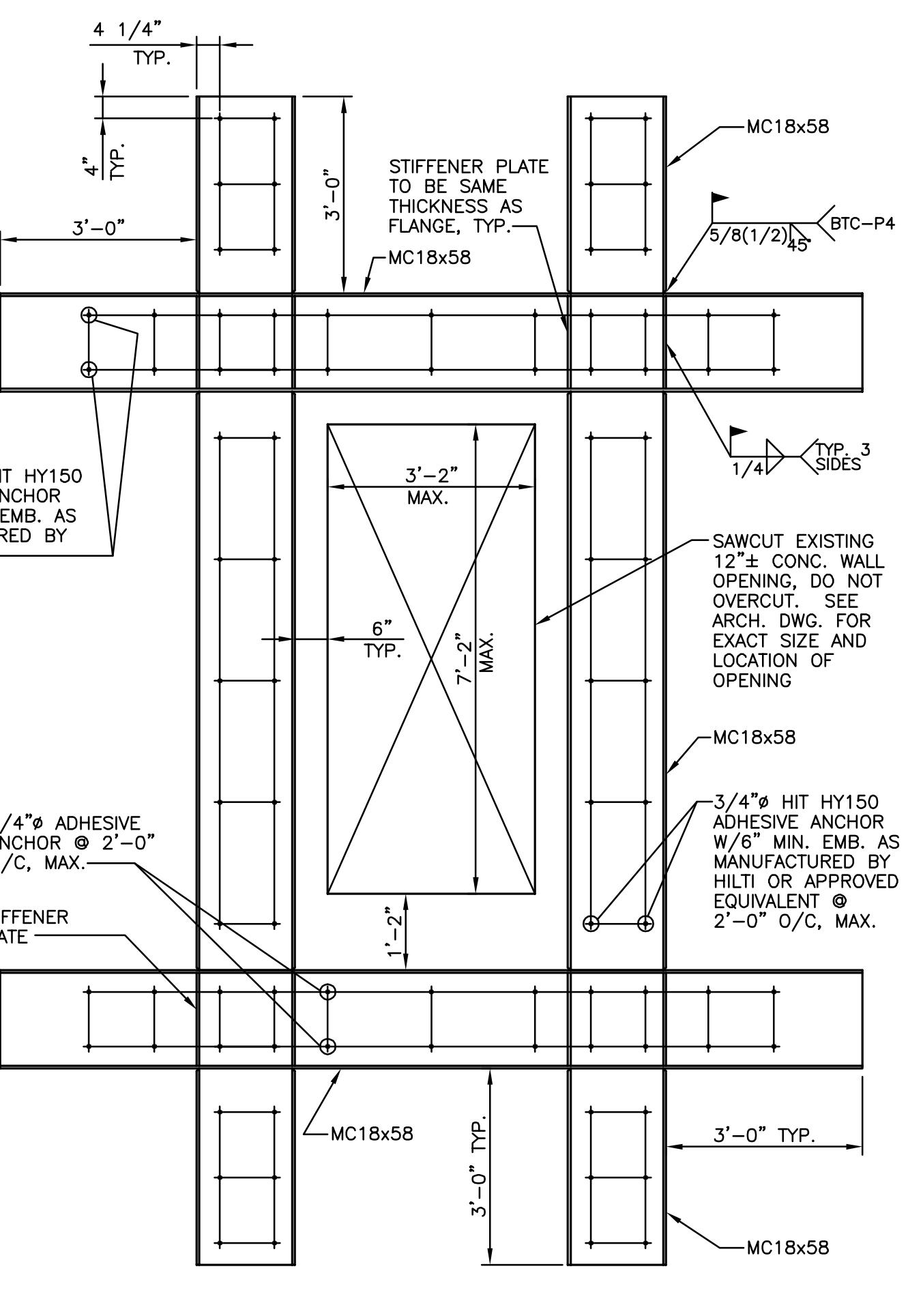
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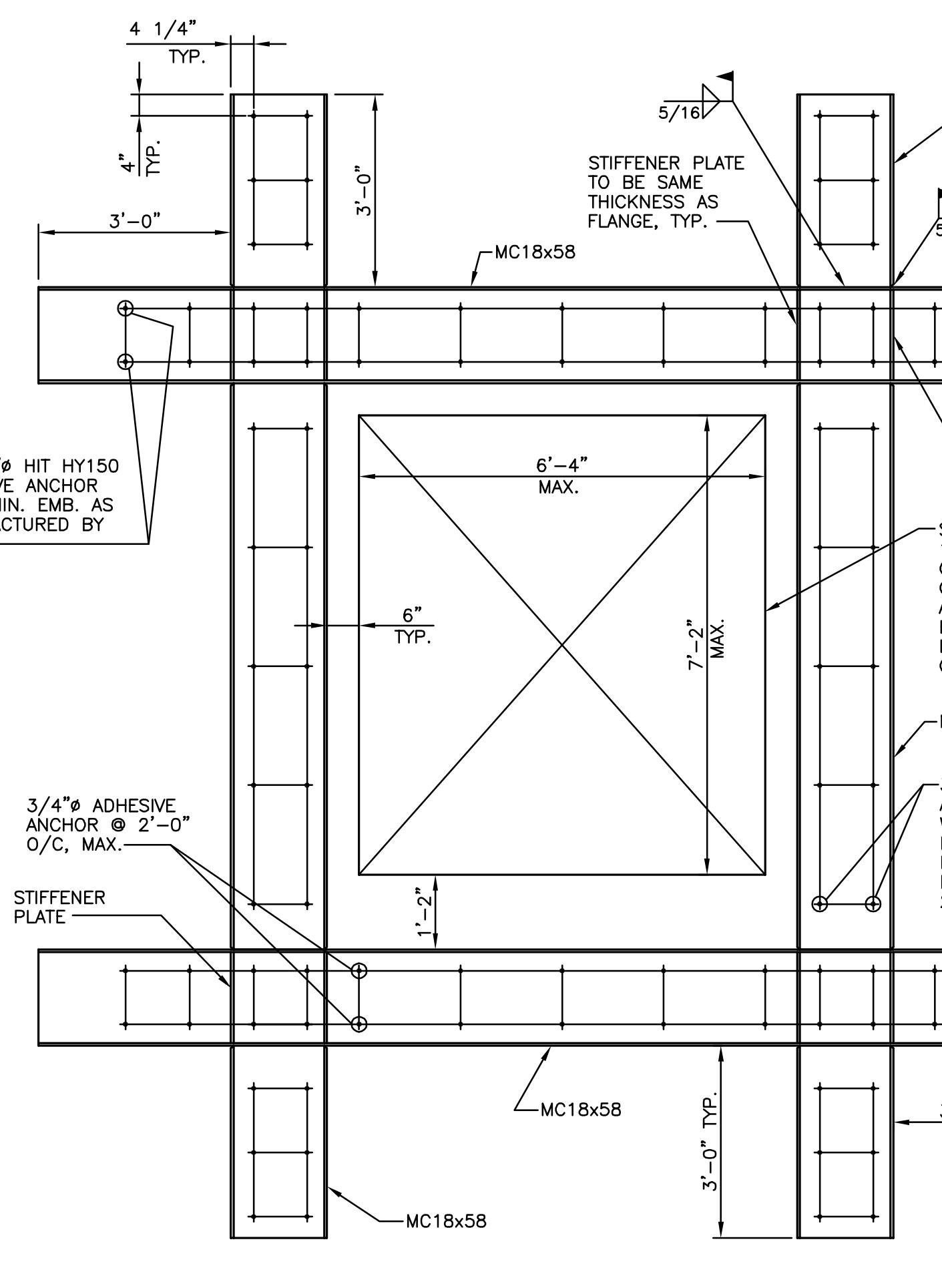
SECTION
 S3.8 NOT TO SCALE



DETAIL
 S3.8 NOT TO SCALE



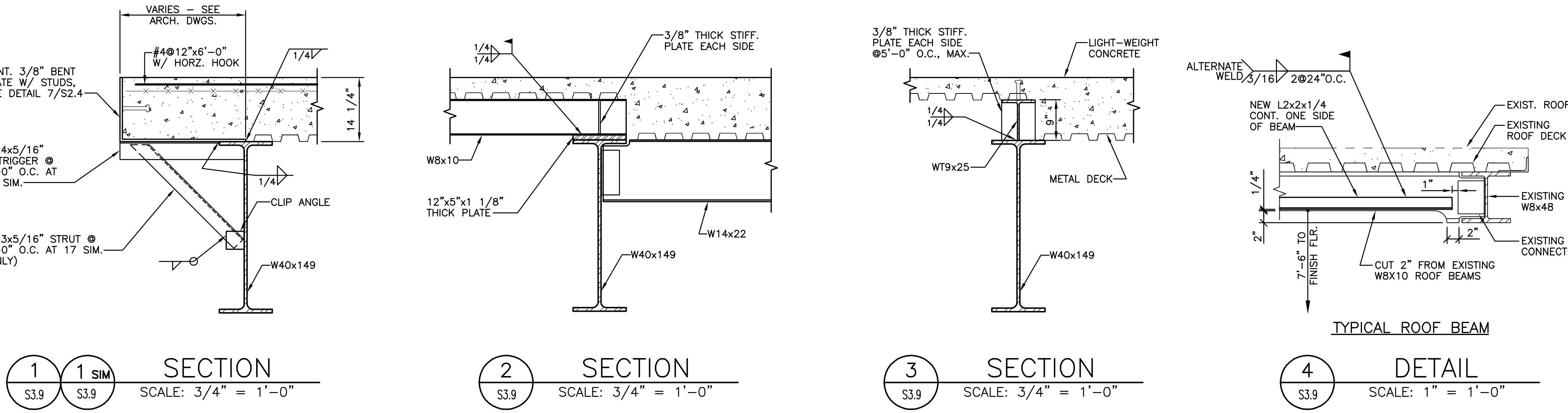
DETAIL
 S3.8 NOT TO SCALE



DETAIL
 S3.8 NOT TO SCALE

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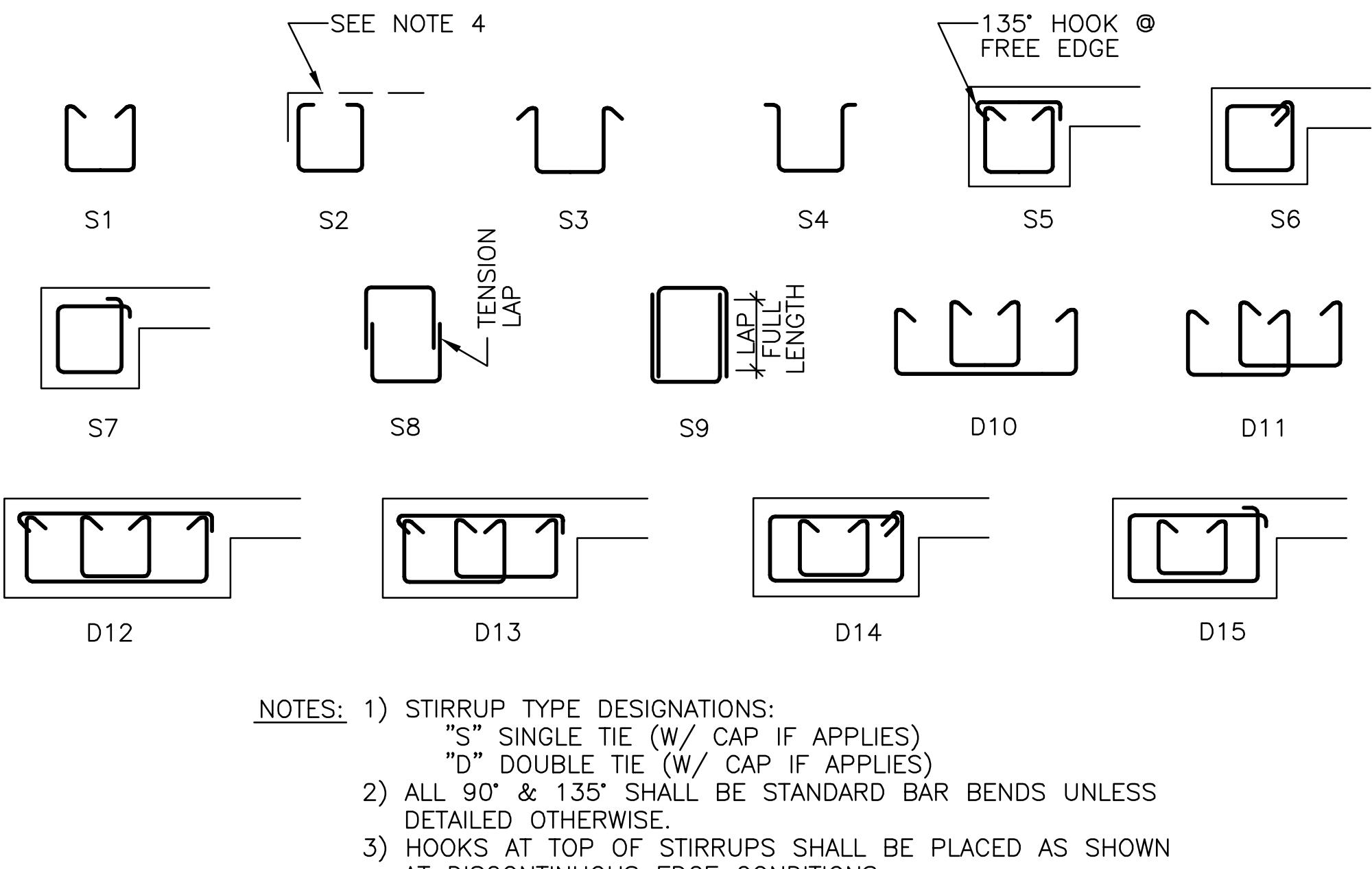
Typical Sections and Details

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S3.9

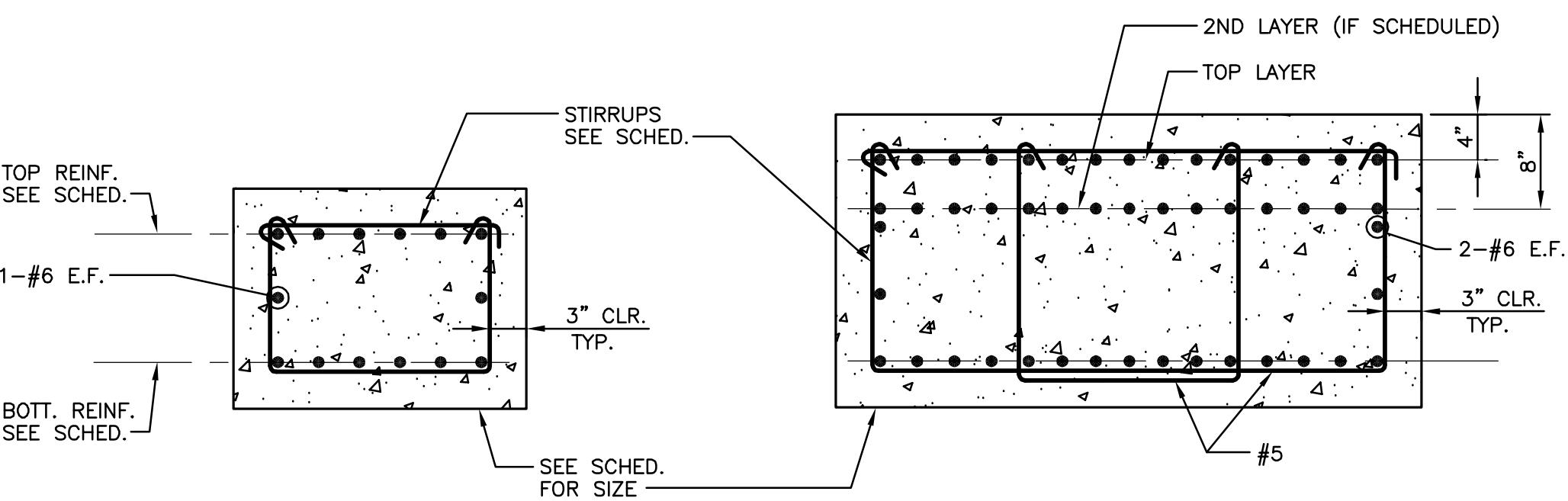


NOTES: 1) STIRRUP TYPE DESIGNATIONS:
 "S" SINGLE TIE (W/ CAP IF APPLIES)
 "D" DOUBLE TIE (W/ CAP IF APPLIES)
 2) ALL 90° & 135° SHALL BE STANDARD BAR BENDS UNLESS
 DETAILED OTHERWISE.
 3) HOOKS AT TOP OF STIRRUPS SHALL BE PLACED AS SHOWN
 AT DISCONTINUOUS EDGE CONDITIONS.
 4) FOR STIRRUP TYPES S1, S2, S3, S4, TOP LEG OF STIRRUP
 IS PROVIDED BY SLAB REINFORCING. WHERE STIRRUP SPACING
 IS CLOSER THAN SLAB REINFORCING, PROVIDE #4 TOP LEG
 AS SHOWN FOR STIRRUP TYPE S5.

— STIRRUP TYPES

S4.1

NOT TO SCALE



TYPICAL GRADE BEAM SECTION

SECTION THRU GB2
 SECTION THRU GB4 SIMILAR

FOOTING SCHEDULE			
MARK	SIZE	REINFORCEMENT	REMARKS
F60	6'-0" x 6'-0" x 24"	(6) #7 E.W. BOTTOM	
F70	7'-0" x 7'-0" x 24"	(8) #7 E.W. BOTTOM	
F110	11'-0" x 11'-0" x 33"	(14) #8 E.W. BOTTOM	
F120	12'-0" x 12'-0" x 36"	(14) #8 E.W. BOTTOM	
F130	13'-0" x 13'-0" x 38"	(17) #9 E.W. BOTTOM	
F140	14'-0" x 14'-0" x 40"	(19) #9 E.W. BOTTOM	
F2817	28'-6" x 16'-6" x 30"	#8@6" E.W., T&B	MAT FOUNDATION
F3212	12'-0" x 32'-0" x 48"	#9@6" E.W., T&B	EXTEND SHORT BARS INTO ELEV. MAT
F3216	16'-0" x 32'-0" x 48"	#9@6" E.W., T&B	
F3512	12'-0" x 35'-0" x 48"	#9@6" E.W., T&B	
WF26	2'-6"x12" CONT.	(3) #4 CONT. W/#4@12" TRANSVERSE	
WF30	3'-0"x24" CONT.	(5) #4 CONT. W/#6@19" TRANSVERSE	
WF30LD	3'-0"x12" CONT.	(4) #4 CONT. W/#4@12" TRANSVERSE	4-#4 CONT. #4@18" TOP SEE 3/S3.2
WF36	3'-6"x12" CONT.	(5) #4 CONT. W/#5@16" TRANSVERSE	
WF40	4'-0"x24" CONT.	(5) #5 CONT. W/#5@12" TRANSVERSE	
WF120	12'-0" x 24" CONT.	(13) #6 CONT. T&B W/#9@9" TOP AND #5@9" BOTTOM - TRANSVERSE	
WF62	6'-2"x12" CONT.	(9) #4 CONT. W/#4@12" TRANSVERSE	

FOOTING SCHEDULE NOTES:

1. FOOTINGS WITH TOP & BOTTOM BARS SHALL HAVE ALL BAR ENDS HOOKED.

CAISSON SCHEDULE			
CAISSON MARK	SHAFT DIA	REINFORCING STEEL	REMARKS
C24	2'-0"	7-#6 W/ #3 TIES @ 12" O.C.	FULL LENGTH VERT. REINF. UNLESS NOTED * ON PLAN.
C36	3'-0"	7-#8 W/ #3 TIES @ 16" O.C.	
C42	3'-6"	7-#9 W/ #4 TIES @ 18" O.C.	
C48	4'-0"	7-#10 W/ #4 TIES @ 18" O.C.	
C54	4'-6"	9-#10 W/ #4 TIES @ 18" O.C.	
C60	5'-0"	9-#11 W/ #4 TIES @ 18" O.C.	

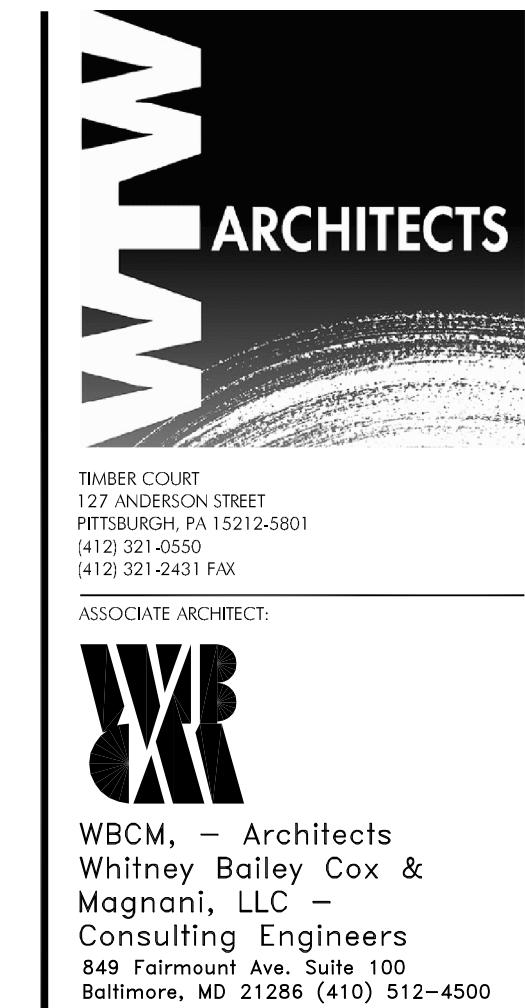
NOTES:

1. END BEARING = 100 KSF (ALLOWABLE)
2. SKIN FRICTION = 0.5 KSF (ALLOWABLE)
3. SEE DETAILED CAISSON DETAILS ON DWGS.
4. CAISSENS ARE CENTERED ON COLUMN LINES UNLESS NOTED OTHERWISE.

MASONRY WALL LINTEL SCHEDULE			
MARK	SIZE	SPAN	REMARKS
P	8" DEEP PRECAST CONCRETE LINTEL REINFORCED WITH 1-#4 TOP & BOTTOM 1-#5 TOP & BOTTOM	UP TO 5'-0" CLEAR	AT 6" THICK WALLS PROVIDE 1-#5 TOP & BOTTOM
L-1	1-L 4'x3 1/2"x5 1/16"	UP TO 4'-0"	1-L FOR EACH 4" THICKNESS OF MASONRY
L-2	1-L 5"x3 1/2"x5 1/16"	4'-0" TO 6'-0"	D.O.
L-3	1-L 6"x3 1/2"x5 1/16"	6'-0" TO 8'-0"	D.O.
L-4	W8x18 + SUSPENDED PLATE	SEE PLAN	W 3/8"

NOTES:

1. END LINTELS SHALL HAVE 6" MINIMUM BEARING AT EACH END.
2. PRECAST LINTELS OVER 4' OPENINGS PIERCED ABOVE INCLUDING DOORS, WINDOWS, DUCTS, LOUVERS, RECESSSES, AND OTHER OPENINGS. UNLESS SHOWN OR NOTED OTHERWISE ON STRUCTURAL OR ARCHITECTURAL DRAWINGS.
3. FOR DIMENSIONS AND LOCATIONS OF OPENINGS, SEE ARCHITECTURAL MECHANICAL AND PLUMBING DRAWINGS.
4. IRON ENDS OF LINTELS IN BUILDING PAPER AT CONTROL JOINTS.
5. SUSPENDED PLATES SHALL BE 5/16"x1" LESS THAN NOMINAL WALL THICKNESS. HANGER PLATES SHALL BE 1/4"x8" LESS THAN NOMINAL WALL THICKNESS OR 4" MINIMUM AND SHALL BE SPACED @ 32" MAXIMUM ON CENTER AND 8" MAXIMUM FROM JAMB. STOP SUSPENDED PLATE 1/2" FROM MASONRY JAMB.
6. ALL EXTERIOR LINTELS SHALL BE HOT DIPPED GALVANIZED.



CONCRETE GRADE BEAM SCHEDULE						
MARK	SIZE (W/HGX) (NOTE 3)	REINFORCING BOTTOM BARS	TOP BARS	SIZE TYPE	SPACING (INCH) (SEE NOTE 1)	REMARKS
GB-1	48 36	(9) #10	(9) #10 TFL	5 SS	BAL012	
GB-2	96 48	(15) #9	(30) #10 TFL	5 D12 9@6" O.C., FROM END OF CANT., BAL012	EXTEND TOP STEEL FROM END OF CANTILEVER TO 6'-0" PAST 2ND SUPPORT. CONTINUE 9#10 FOR GB1	
GB-3	16 24	(3) #7	(3) #6	4 S6	BAL010	
GB-4	48 30	(5) #8	(8) #10	5 D12 6@ 0 CANTILEVER	TOP STEEL FROM END OF CANTILEVER TOP FULL LENGTH	
GB-5	24 24	(5) #9	(5) #9	4 D12 6@ 10 E.E.	BAL012	

CONCRETE BEAM SCHEDULE NOTES:

1. \times INDICATES NO. OF STIRRUPS @ NOTED SPACING.
2. FOR TYP. GRADE BEAM DETAILS, SEE 3/S2.1.

LIGHT GAGE METAL WALL STUD SCHEDULE					
	MAX. HT.	WIDTH	MIN. FLANGE WIDTH	MIN. GAGE	MAX. SPACING
EXTERIOR	16'-8"	6"	2"	12GA.	12"
	16'-8"	8"	2"	14GA.	16"
	22'-0"	10"	2"	16GA.	16"
	22'-0"	12"	2"	18GA.	16"
INTERIOR (BETWEEN BUILDINGS)	22'-0"	6"	1 5/8"	18GA.	16"

NOTE:
 ALL LIGHT GAGE SHALL BE MINIMUM 50 KSI STEEL.

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COLUMN MARK	C-8	D-8 E-8 F-8	B.4-8	E-1	B-1	D.3-1	F.3-2	E.4-2	C-2	B-2	G-3	A.2-5.4	A-9 B.4-9	A-7.4	G-8	C-8.8	C-1	B-8	G-4	B-3	B-4 G-5	B-5	G-6	B-6 B-7	D-3	E.4-3 C-3	C-4 E-4	F-4	C-5 E-5 F-5	C-6 D-6 E-6 F-6	C-7 D-7 E-7 F-7	F-3	A-1 A-1.3	B-1.3	D-4	D-5	A.2-6 A.2-7	Z-5.3 Y-5.6 A-5.3 A-5.6	B.4-8.8	D.6-2
Stair S8 Roof																																								
Roof																																								
Penthouse																																								
Natatorium Roof																																								
5th Floor																																								
Stair S1 Roof																																								
4th Floor																																								
Pool																																								
3rd Floor																																								
2nd Floor																																								
1st Floor																																								
Receiving Area Floor																																								
Garden Level/ Concrete Grade																																								
WORKING LOADS (k)	500	500	230	900	900	450	550	1160	1160	530	900	100	150	445	546	160	600	50	400	500	600	600	533	117	618	450	450	700	800	340	300	30	450	600	190	20				
BASEPLATE	BP-8	BP-8	BP-7	BP-9	BP-9	BP-8	BP-9	BP-10	BP-8	BP-9	BP-3	BP-1	BP-3	BP-2	BP-8	BP-4	BP-10A	BP-9	BP-10A	BP-10A	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9	BP-9			
COLUMN MARK	C-8	B.4-8	E-1	B-1	D.3-1	F.3-2	E.4-2	C-2	B-2	G-3	A.2-5.4	A-9 B.4-9	A-7.4	G-8	C-8.8	C-1	B-8	G-4	B-3	B-4 G-5	B-5	G-6	B-6 B-7	D-3	E.4-3 C-3	C-4 E-4	F-4	C-5 E-5 F-5	C-6 D-6 E-6 F-6	C-7 D-7 E-7 F-7	F-3	A-1 A-1.3	B-1.3	D-4	D-5	A.2-6 A.2-7	Z-5.3 Y-5.6 A-5.3 A-5.6	B.4-8.8	D.6-2	

MARK ORDER: BY COORDINATES

COLUMN SCHEDULE

NOTES:
1. , DENOTES COLUMN SPLICING, SEE DETAIL 10/S2.4.

2. +, DENOTES STEEL TO CONCRETE PIER CONNECTION, SEE SHEET S4.4.

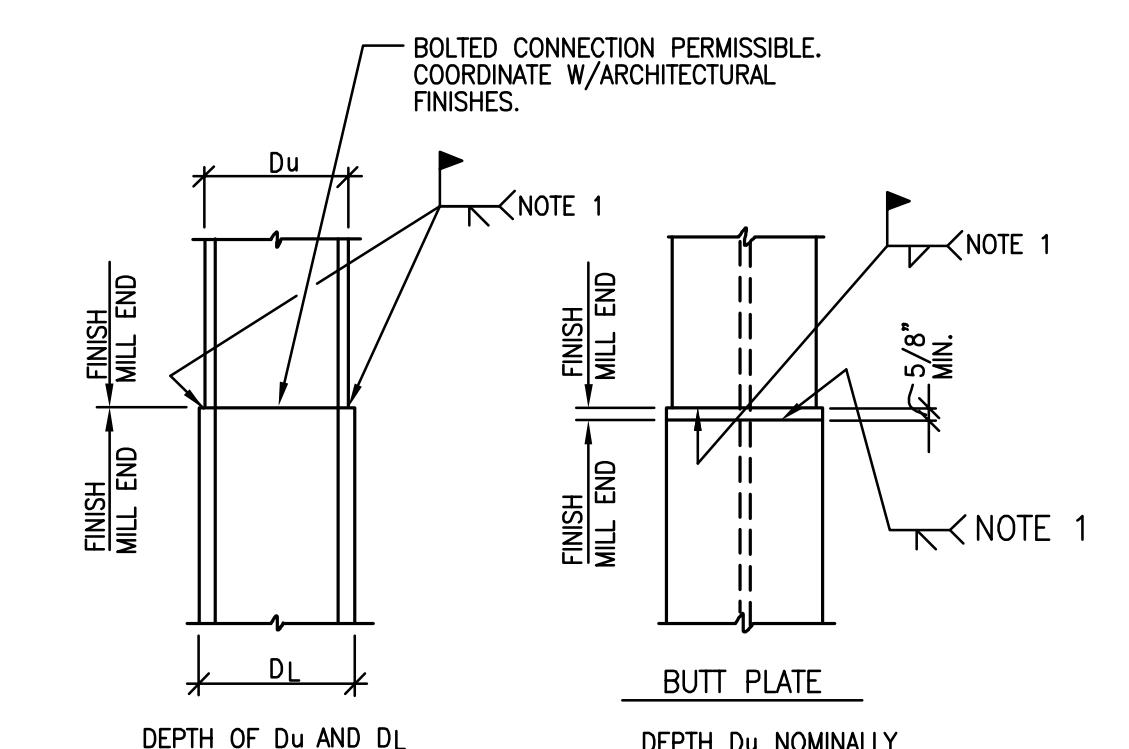
3. SEE S4.3 FOR LATERAL FRAMES.

4. FOR CONCRETE PIER SIZE AND REINFORCEMENT, SEE PIER SCHEDULE ON S4.5.

5. COLUMN WORKING LOADS SHOWN IN SCHEDULE ARE IN KIPS AND ARE NOT FACTORED LOADS.

BASE PLATE & ANCHOR BOLT SCHEDULE

MARK	SIZE	ANCHOR BOLT
BP-1	18"x18"x1"	(4)-1"Ø A.B.
BP-2	14"x14"x1"	(4)-1"Ø A.B.
BP-3	16"x16"x1 1/4"	(4)-1"Ø A.B.
BP-4	22"x14"x1"	(4)-3/4"Ø BOLT
BP-5	18"x28"x2"	(4)-1"Ø A.B.
BP-7	22"x22"x1 1/4"	(4)-1"Ø A.B.
BP-8	22"x22"x1 3/4"	(4)-1"Ø A.B.
BP-9	24"x24"x2"	(4)-1 1/4"Ø A.B.
BP-10	26"x26"x2 1/2"	(4)-1 1/4"Ø A.B.
BP-10A	26"x26"x2 1/2"	(8)-1 1/2"Ø A.B.



NOTES: 1) USE FILLET WELDS OR PARTIAL PENETRATION WHENEVER POSSIBLE.
2) PROVIDE BEVEL EDGES OF THINNER WF FOR BUTT WELD.
3) DESIGN COLUMN CONNECTION TO FULL CAPACITY OF SMALLER COLUMNS.

COLUMN SPLICING - WELDED

1
S4.2
NOT TO SCALE

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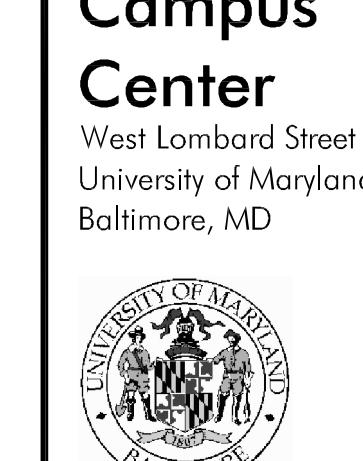
100%

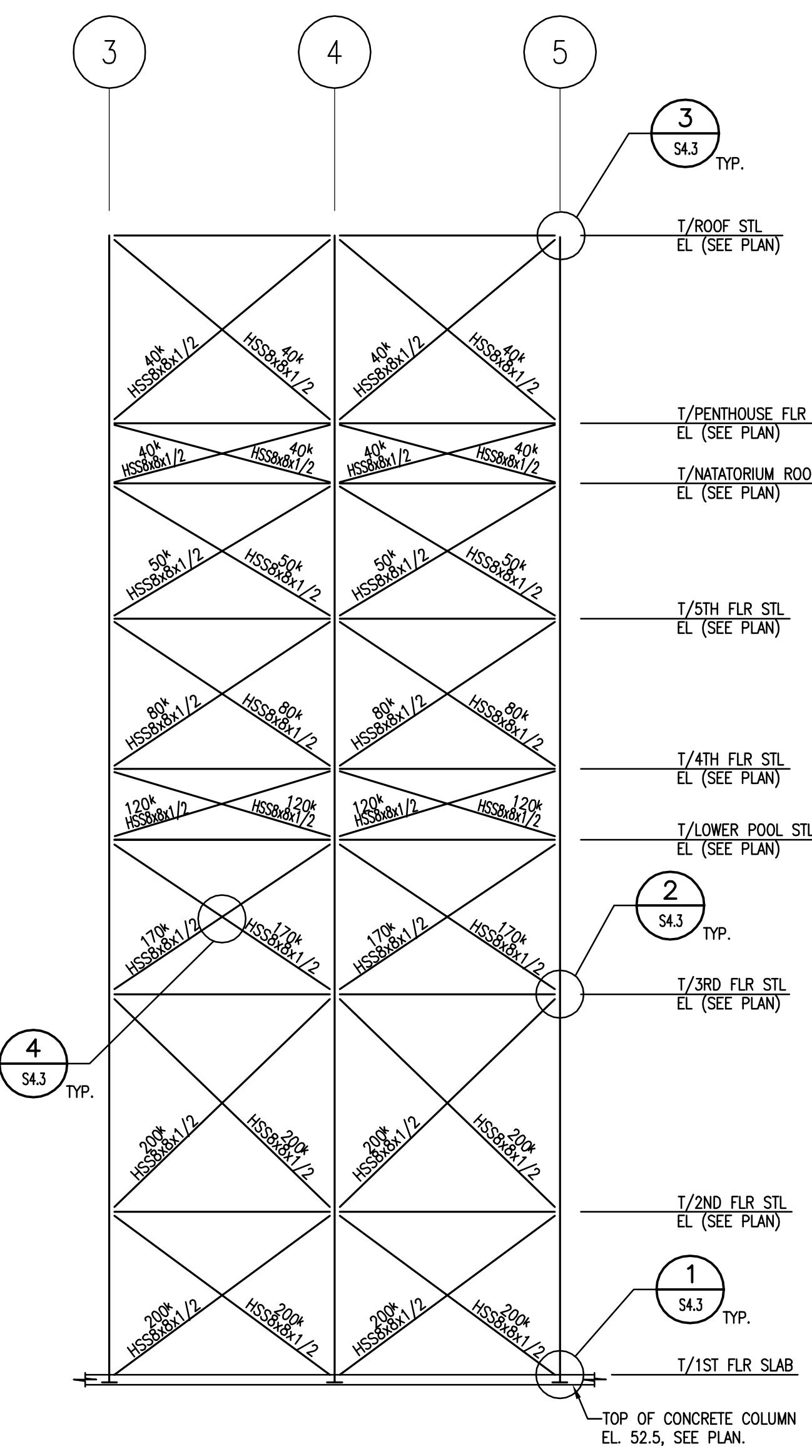
CONSTRUCTION
DOCUMENTS

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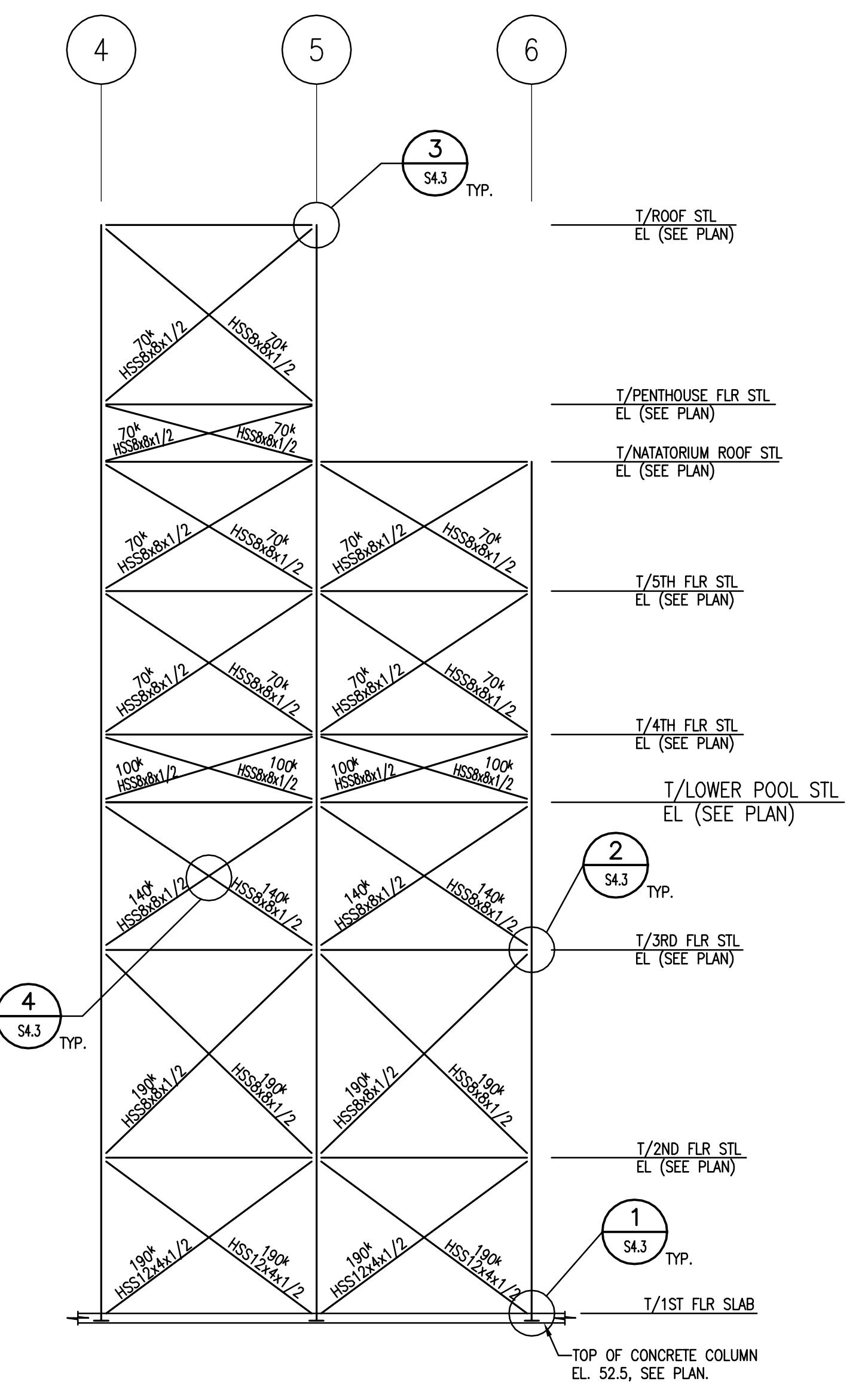
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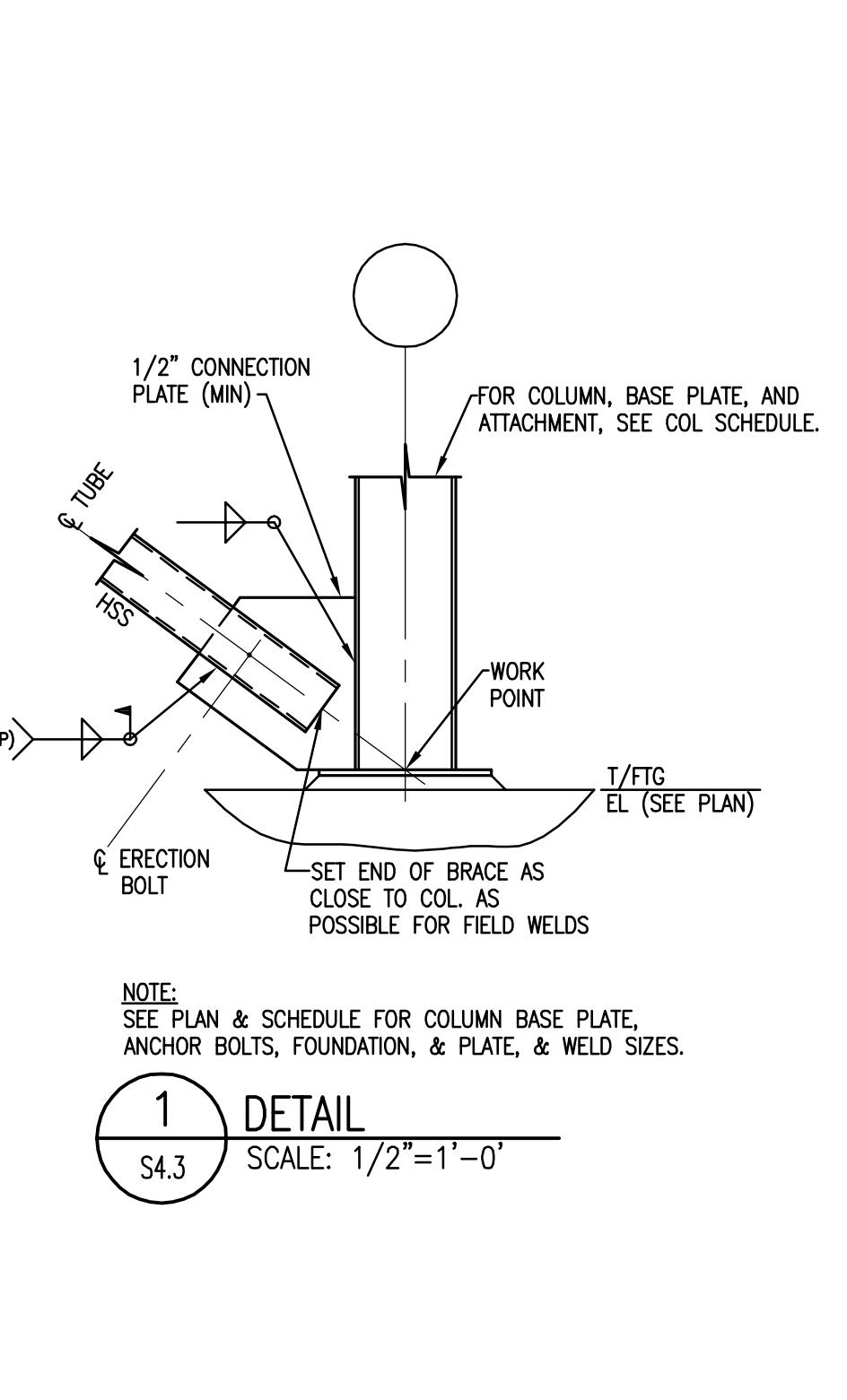
NOTES:
 1. COLUMN SPLICES SHALL BE DESIGNED FOR TENSION AND COMPRESSION LOADING.
 2. PRIOR TO FABRICATION, SUBMIT CONNECTION DESIGN CALCULATIONS TO ARCHITECT OF RECORD, SIGNED, AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MARYLAND.
 3. SEE COLUMN SCHEDULES FOR COLUMN AND BEAM SIZES.
 4. FORCES SHOWN ON DRAWINGS ARE SERVICE LOAD TENSION FORCES IN KIPS. 33% STRESS INCREASES IN CONNECTION DESIGN ARE NOT PERMITTED. ALL FORCES SHOWN ON ELEVATION DO NOT OCCUR SIMULTANEOUSLY.
 5. CONNECTIONS SHOULD BE DESIGNED FOR 2 DIAGONALS IN TENSION FOR LOADS FROM THE RIGHT DIRECTION AND/OR 2 DIAGONALS FOR LOADS FROM THE LEFT DIRECTION.

FRAME 1 ON COLUMN LINE (B)
 NOT TO SCALE

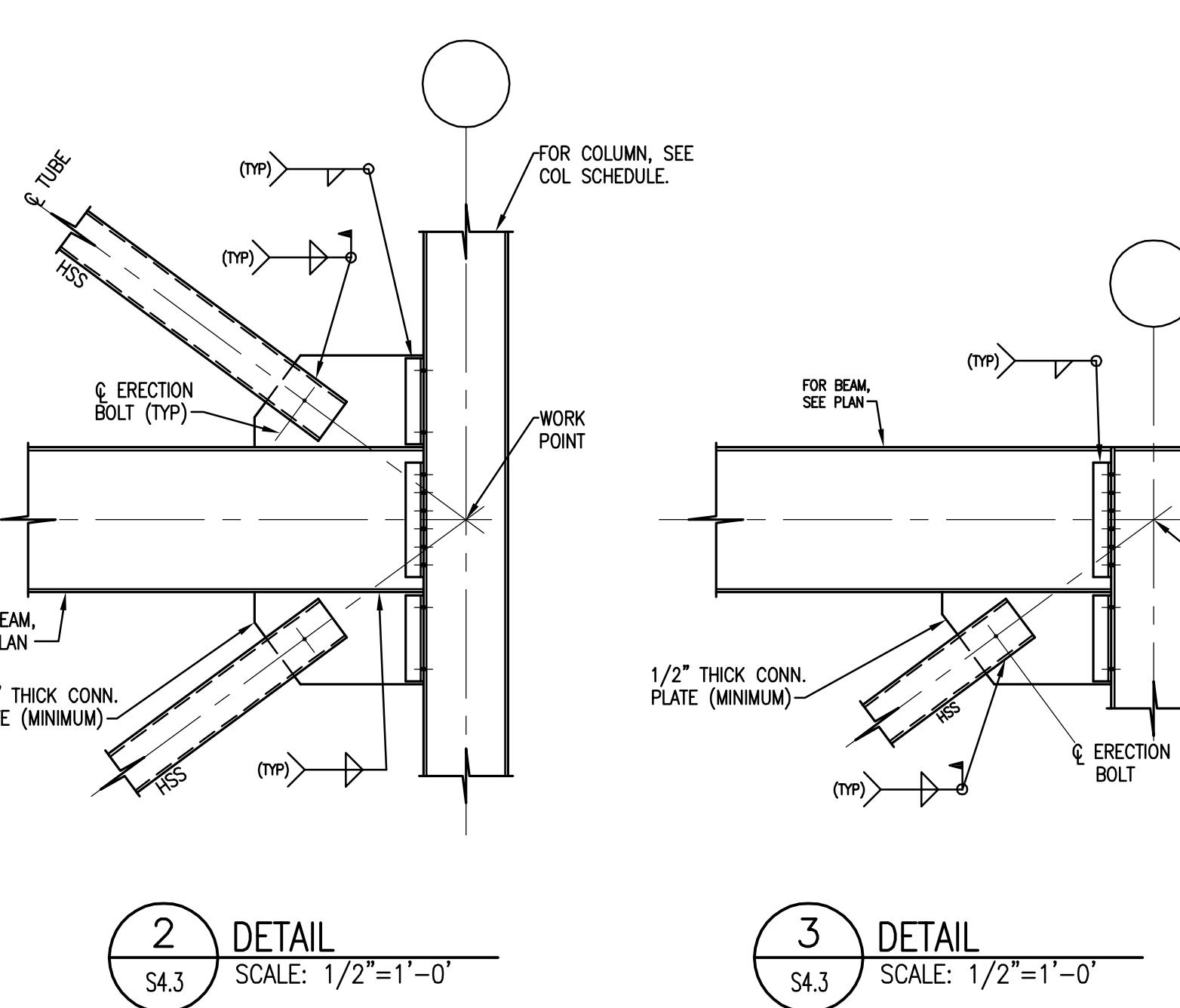


NOTES:
 1. COLUMN SPLICES SHALL BE DESIGNED FOR TENSION AND COMPRESSION LOADING.
 2. PRIOR TO FABRICATION, SUBMIT CONNECTION DESIGN CALCULATIONS TO ARCHITECT OF RECORD, SIGNED, AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MARYLAND.
 3. SEE COLUMN SCHEDULES FOR COLUMN AND BEAM SIZES.
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 5. CONNECTIONS SHOULD BE DESIGNED FOR 2 DIAGONALS IN TENSION FOR LOADS FROM THE RIGHT DIRECTION AND/OR 2 DIAGONALS FOR LOADS FROM THE LEFT DIRECTION.

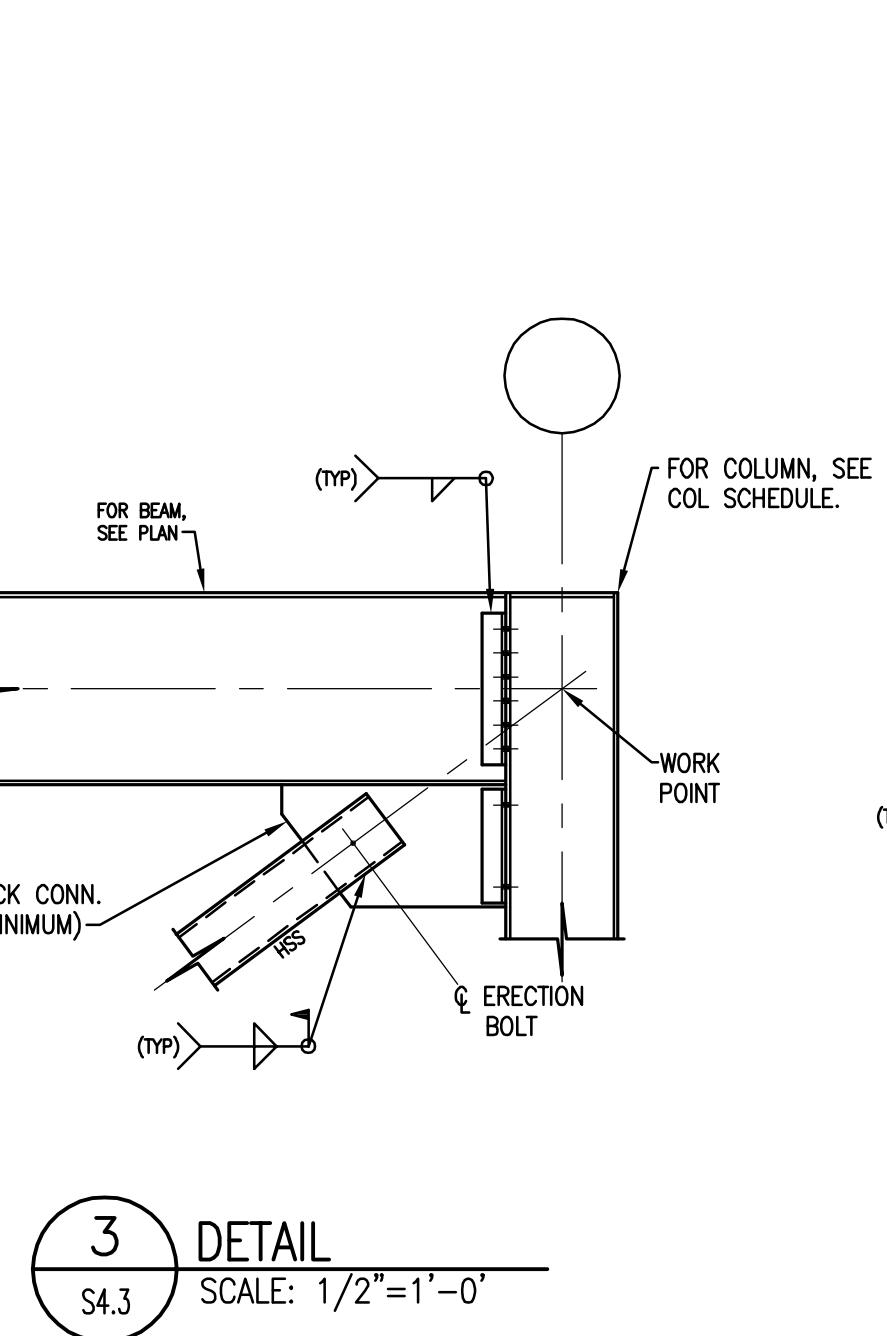
FRAME 2 ON COLUMN LINE (G)
 NOT TO SCALE



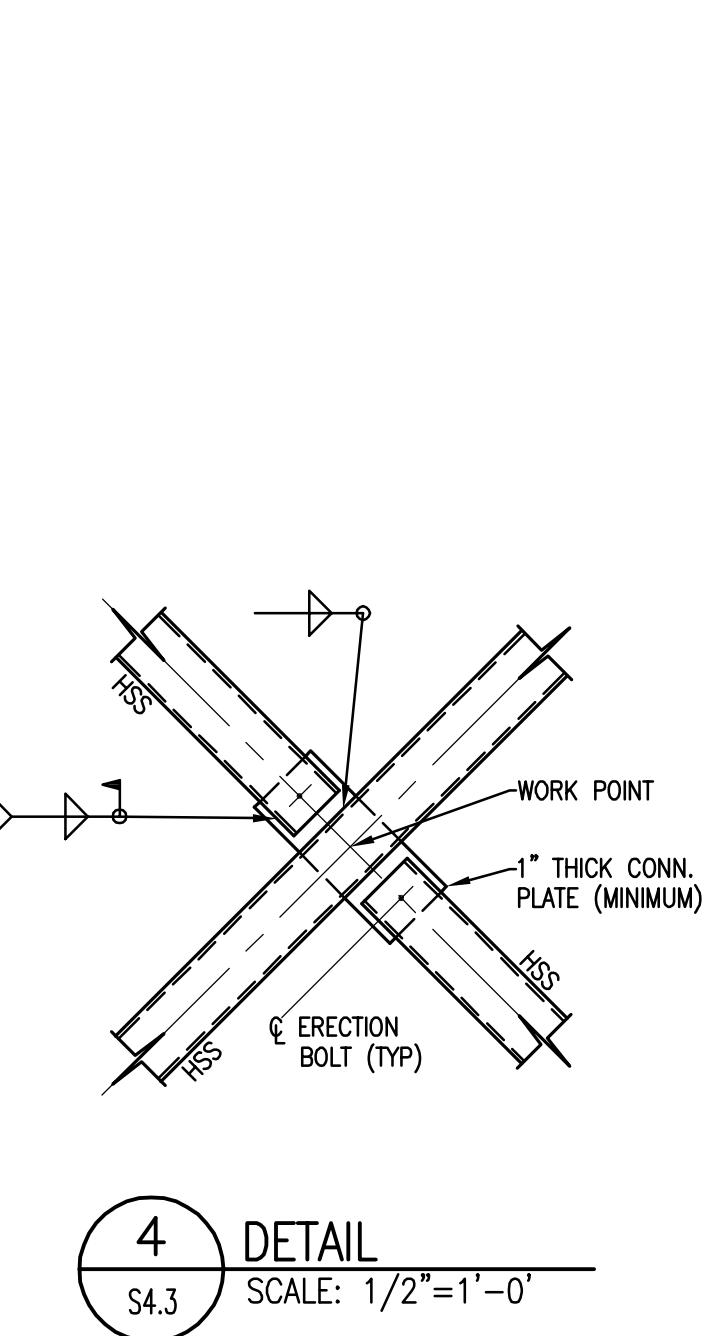
1 DETAIL
 S4.3 SCALE: 1/2"=1'-0"



2 DETAIL
 S4.3 SCALE: 1/2"=1'-0"



3 DETAIL
 S4.3 SCALE: 1/2"=1'-0"



4 DETAIL
 S4.3 SCALE: 1/2"=1'-0"

2 01/29/10 RECORD DRAWINGS
 1 04/09/07 BULLETIN #1
 0 02/16/07 ISSUE FOR BID
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X-Braced Frames And Details

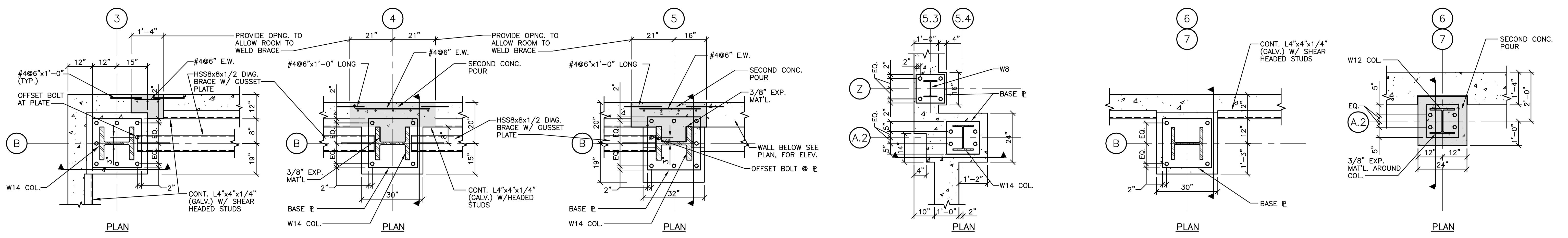
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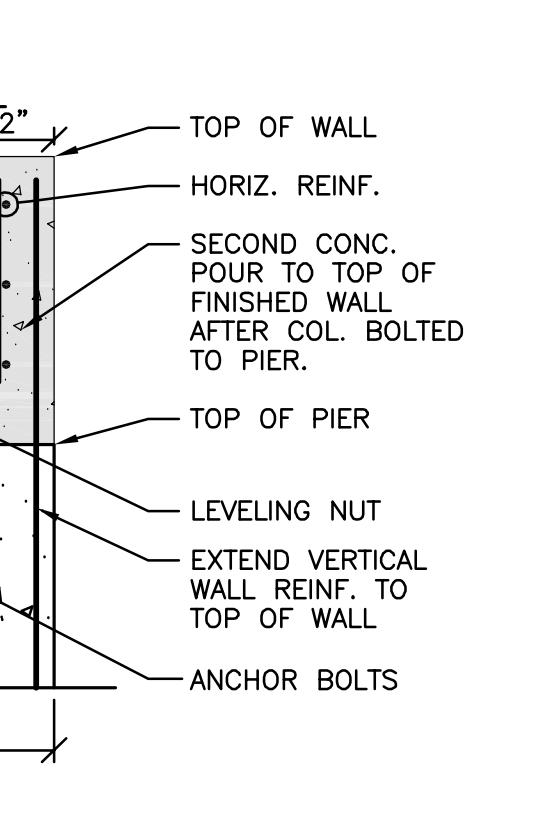
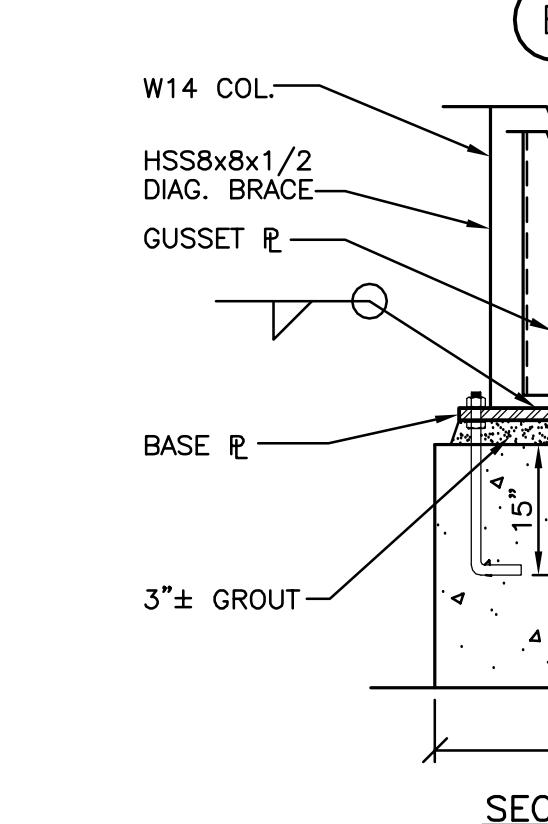
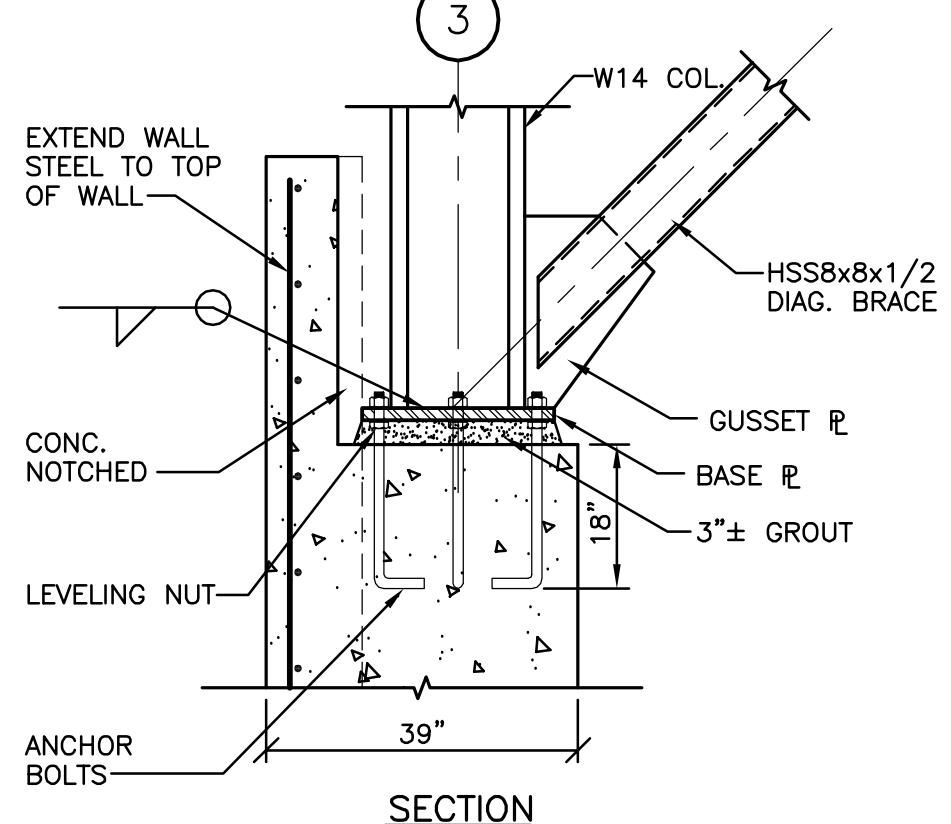
54.3



PLAN

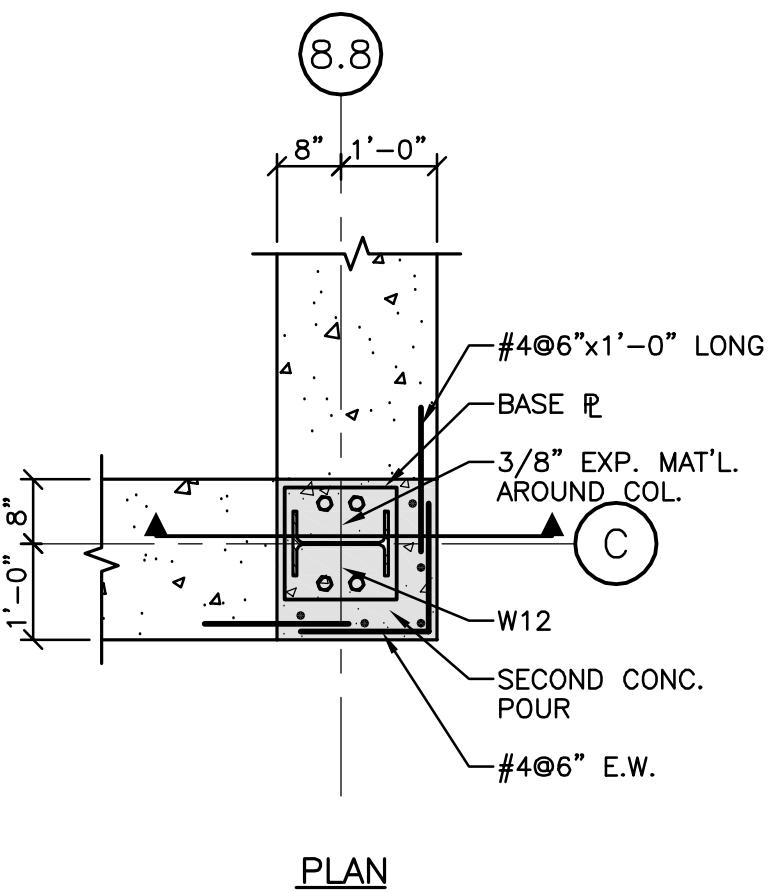
PLAN

PLAN



COLUMN BASE DETAIL — B-3

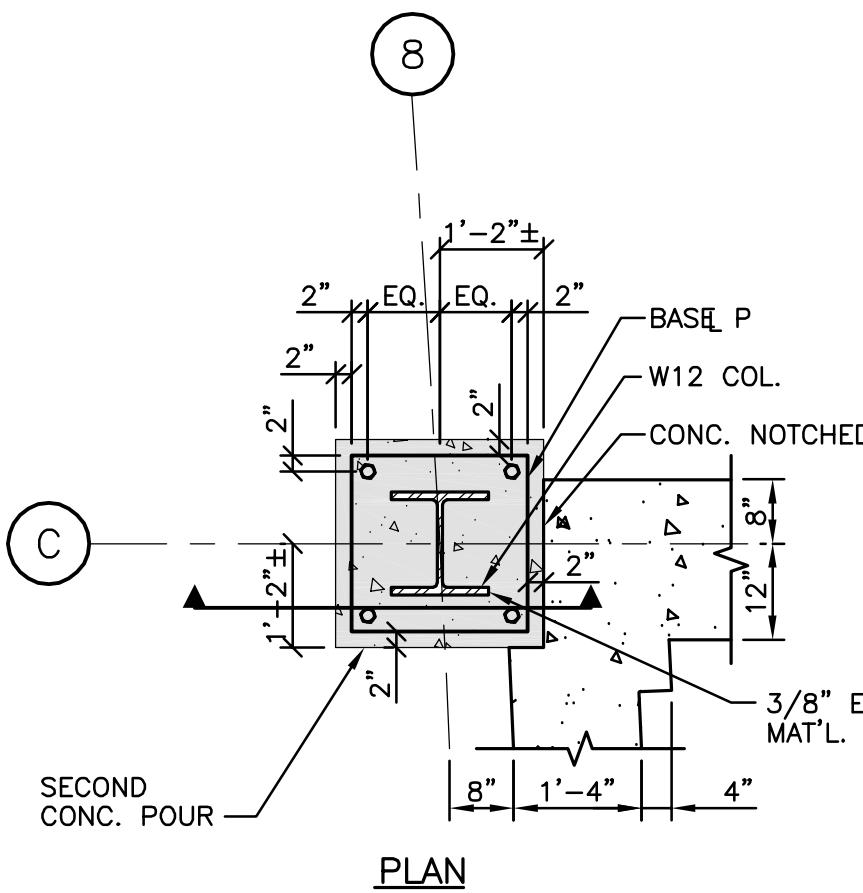
1 DETAIL
 S4.4 SCALE: 1/2"=1'-0"



PLAN

COLUMN BASE DETAIL — B-4

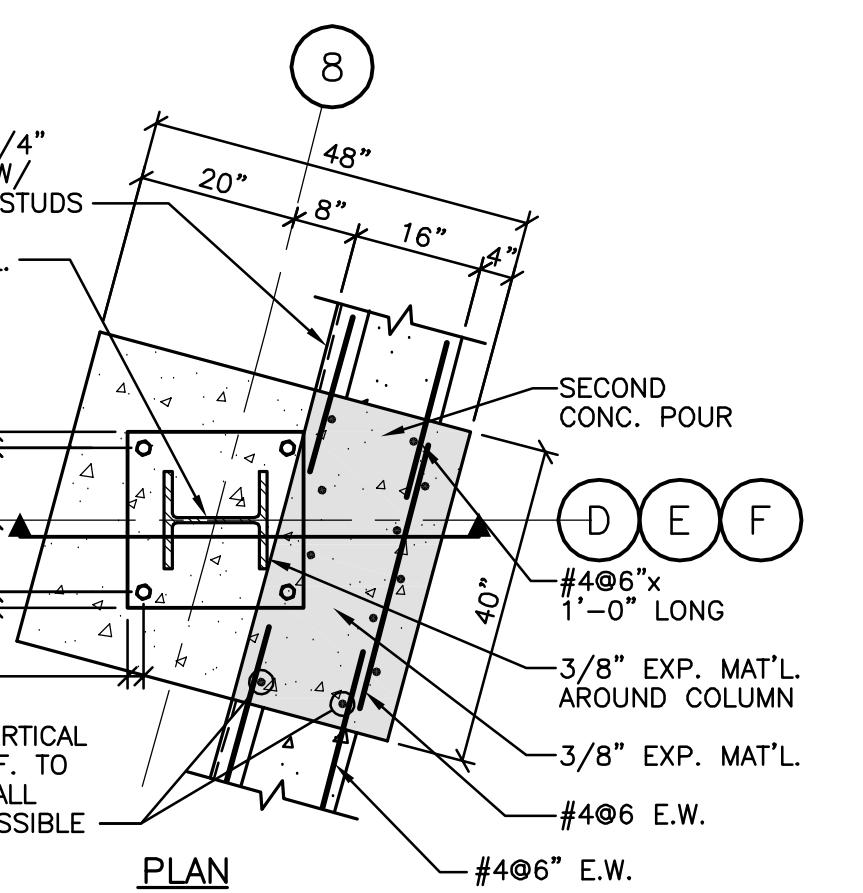
2 DETAIL
 S4.4 SCALE: 1/2"=1'-0"



PLAN

COLUMN BASE DETAIL — B-5

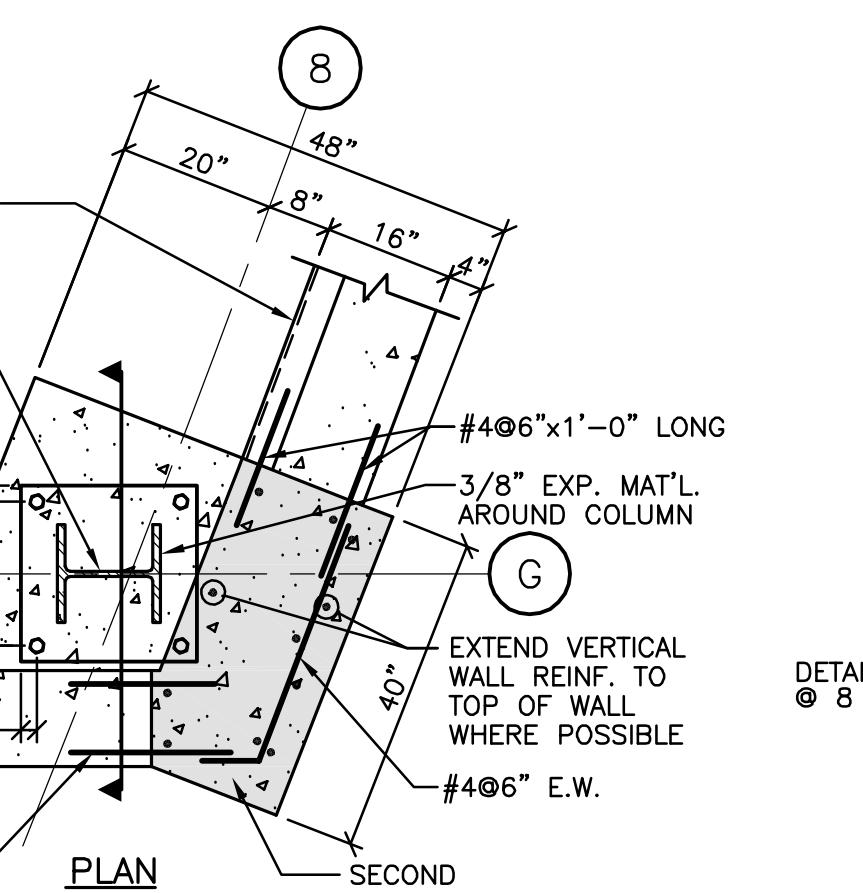
3 DETAIL
 S4.4 SCALE: 1/2"=1'-0"



PLAN

COLUMN BASE DETAIL — A-5.4

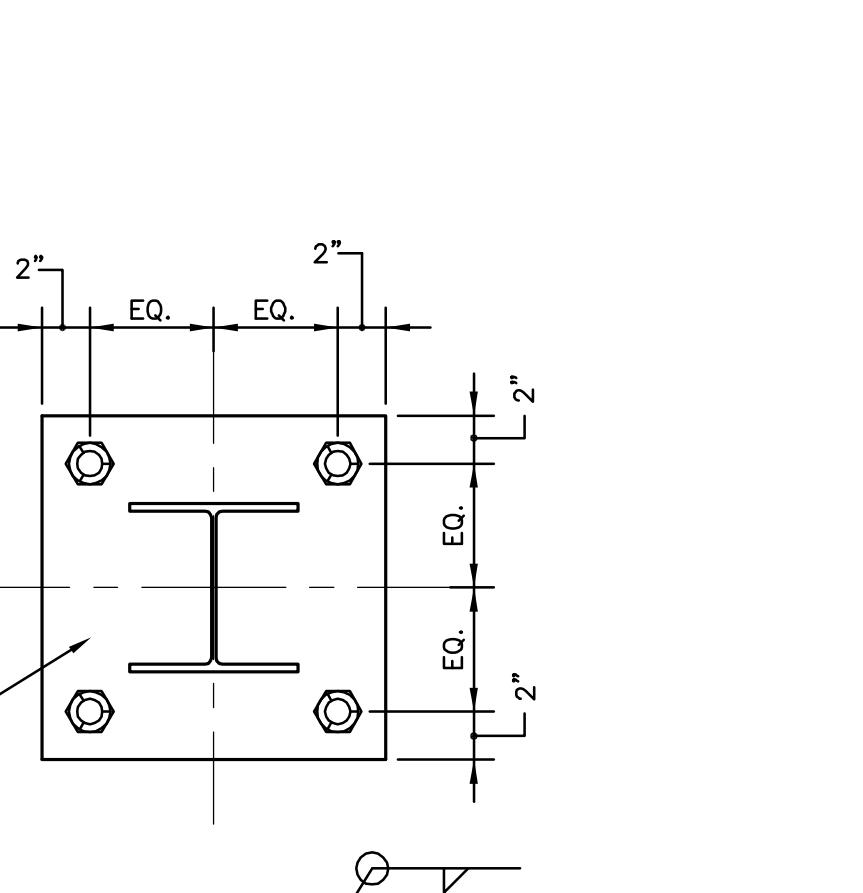
4 DETAIL
 S4.4 SCALE: 1/2"=1'-0"



PLAN

COLUMN BASE DETAIL — B-6, B-7

5 DETAIL
 S4.4 SCALE: 1/2"=1'-0"



PLAN

COLUMN BASE DETAIL — A.2-6, A.2-7

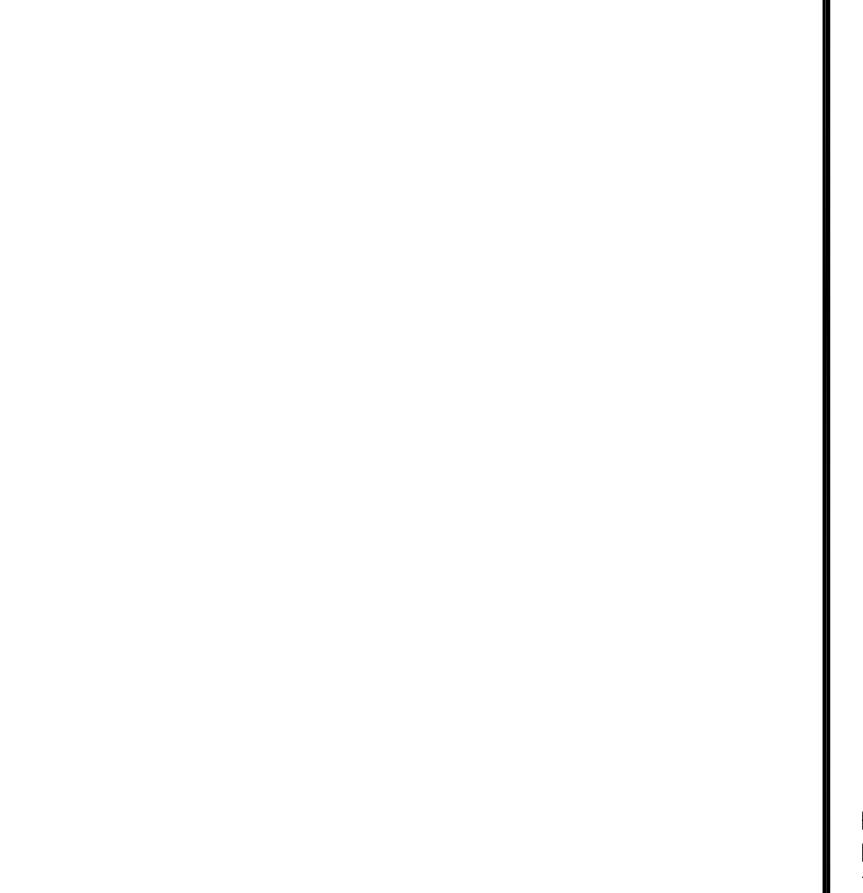
6 DETAIL
 S4.4 SCALE: 1/2"=1'-0"



PLAN

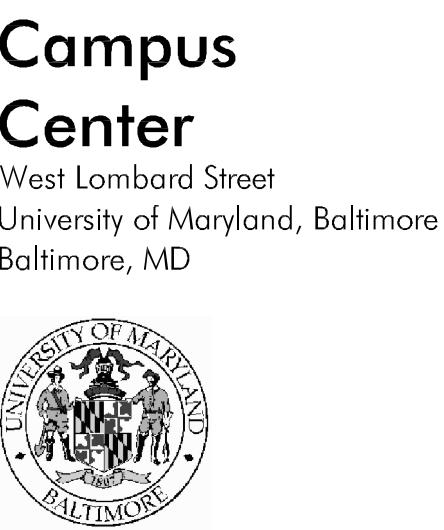
COLUMN BASE DETAIL — A-7.4

7 DETAIL
 S4.4 SCALE: 1/2"=1'-0"

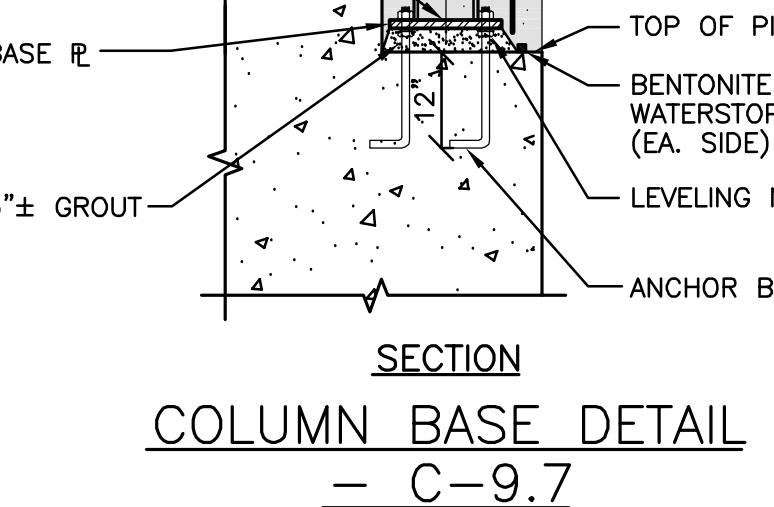


PLAN

2 01/29/10 RECORD DRAWINGS
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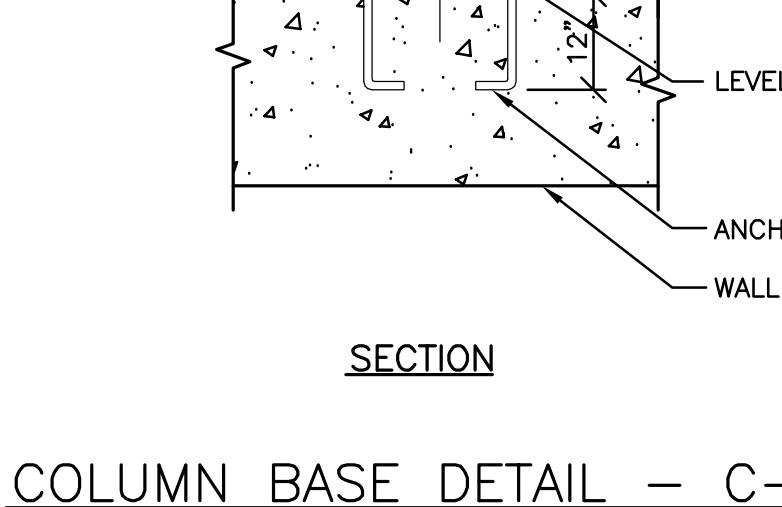


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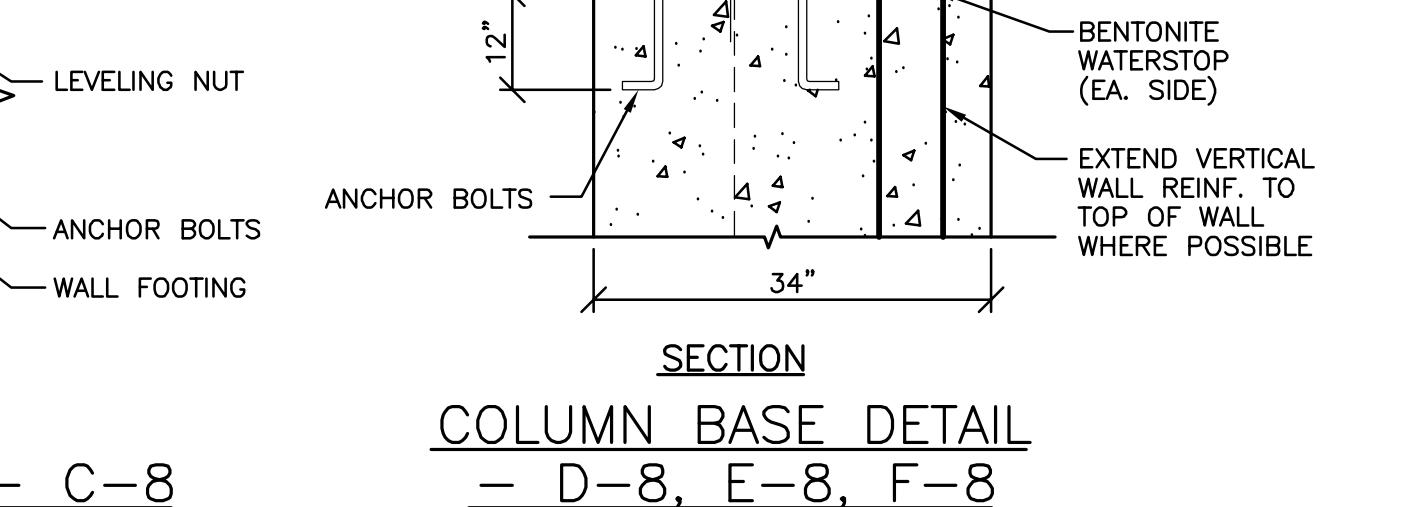
COLUMN BASE DETAIL — C-9.7

8 DETAIL
 S4.4 SCALE: 1/2"=1'-0"



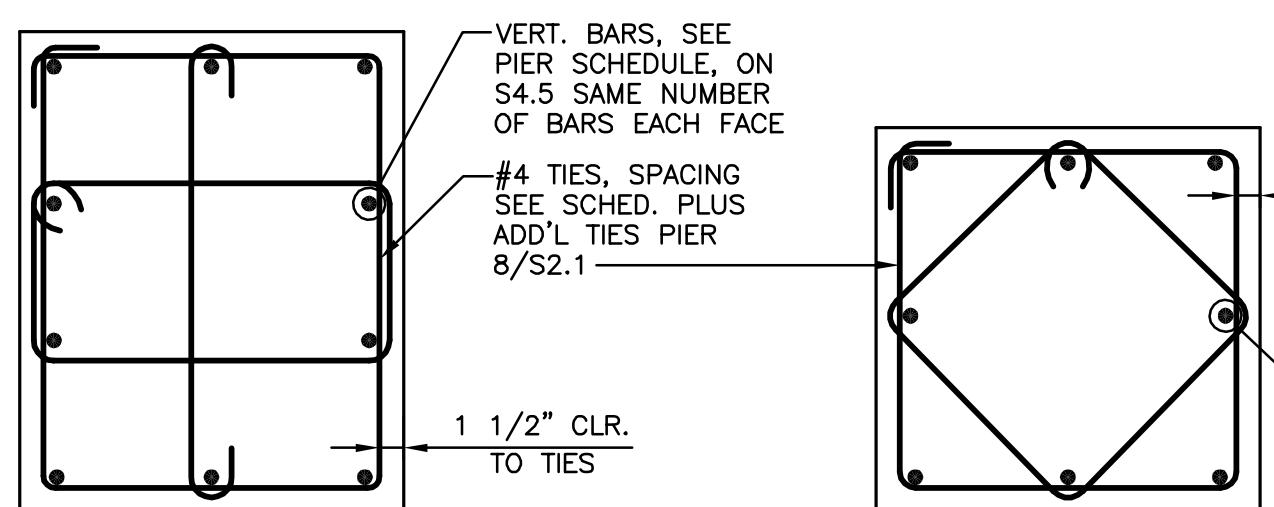
COLUMN BASE DETAIL — C-8

9 DETAIL
 S4.4 SCALE: 1/2"=1'-0"



COLUMN BASE DETAIL — D-8, E-8, F-8

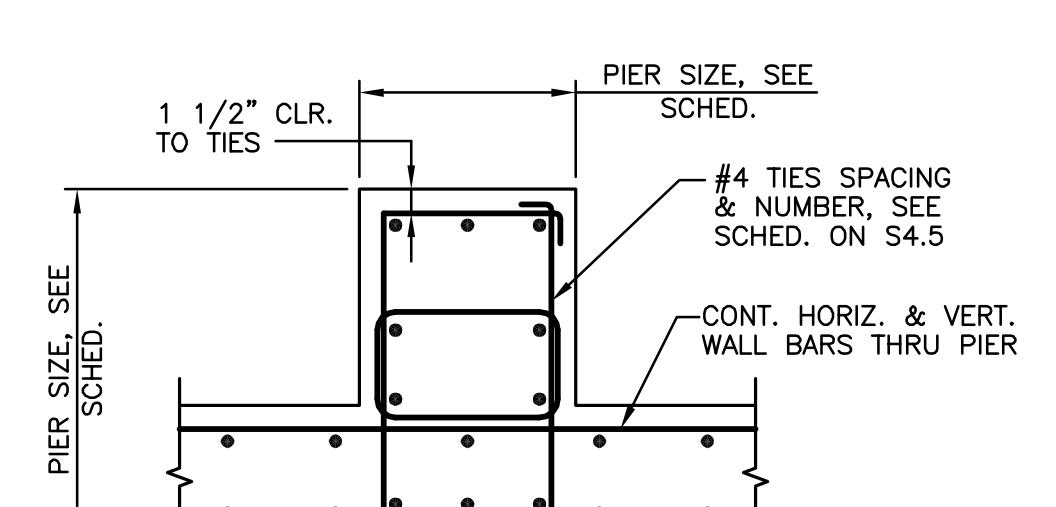
10 DETAIL
 S4.4 SCALE: 1/2"=1'-0"



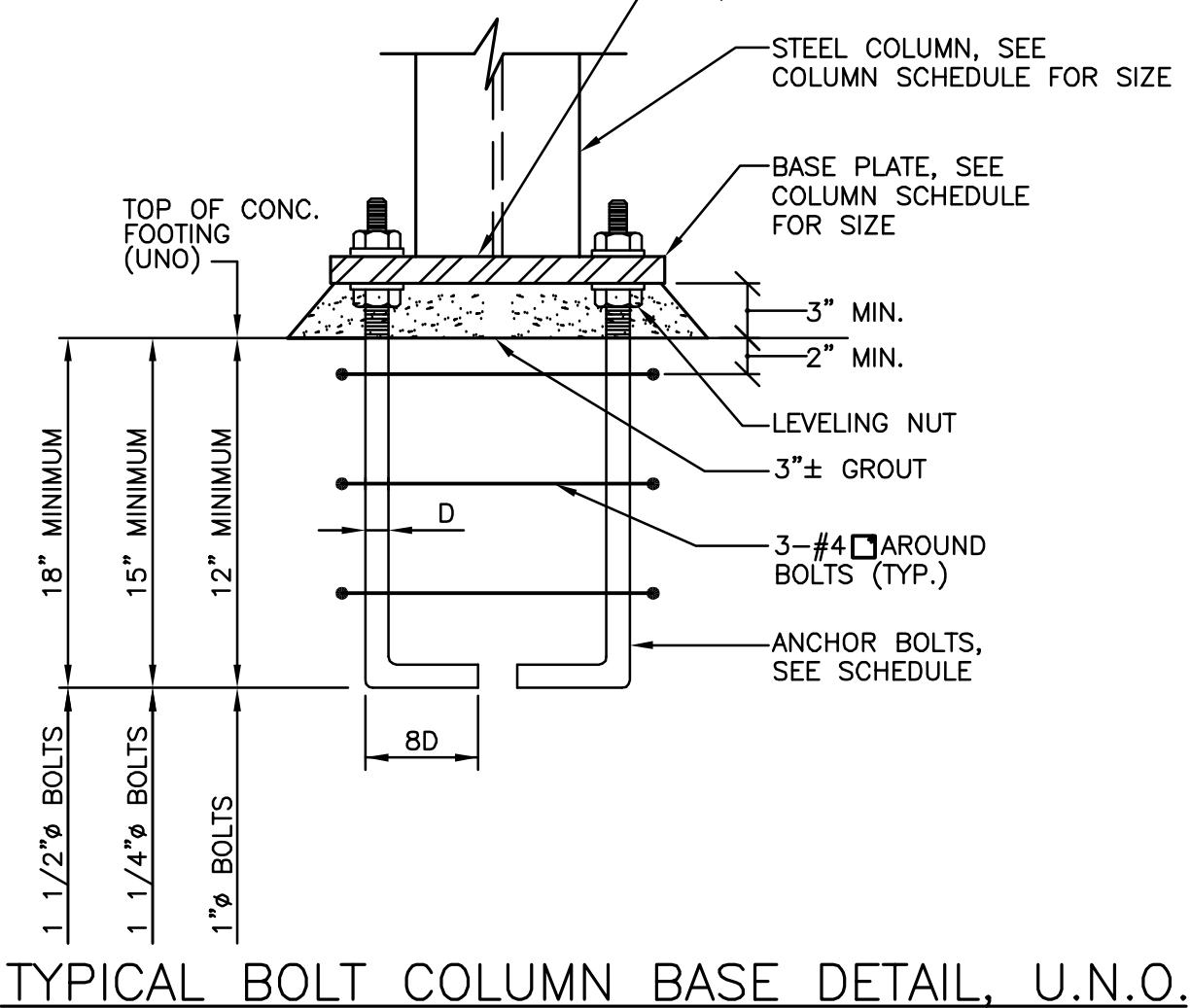
RECTANGULAR

SQUARE

14 CONCRETE PIERS DETAIL
 S4.4 NOT TO SCALE



15 DETAIL
 S4.4 SCALE: 3/4"=1'-0"



11 DETAIL
 S4.4 SCALE: 1/2"=1'-0"

12 DETAIL
 S4.4 NOT TO SCALE

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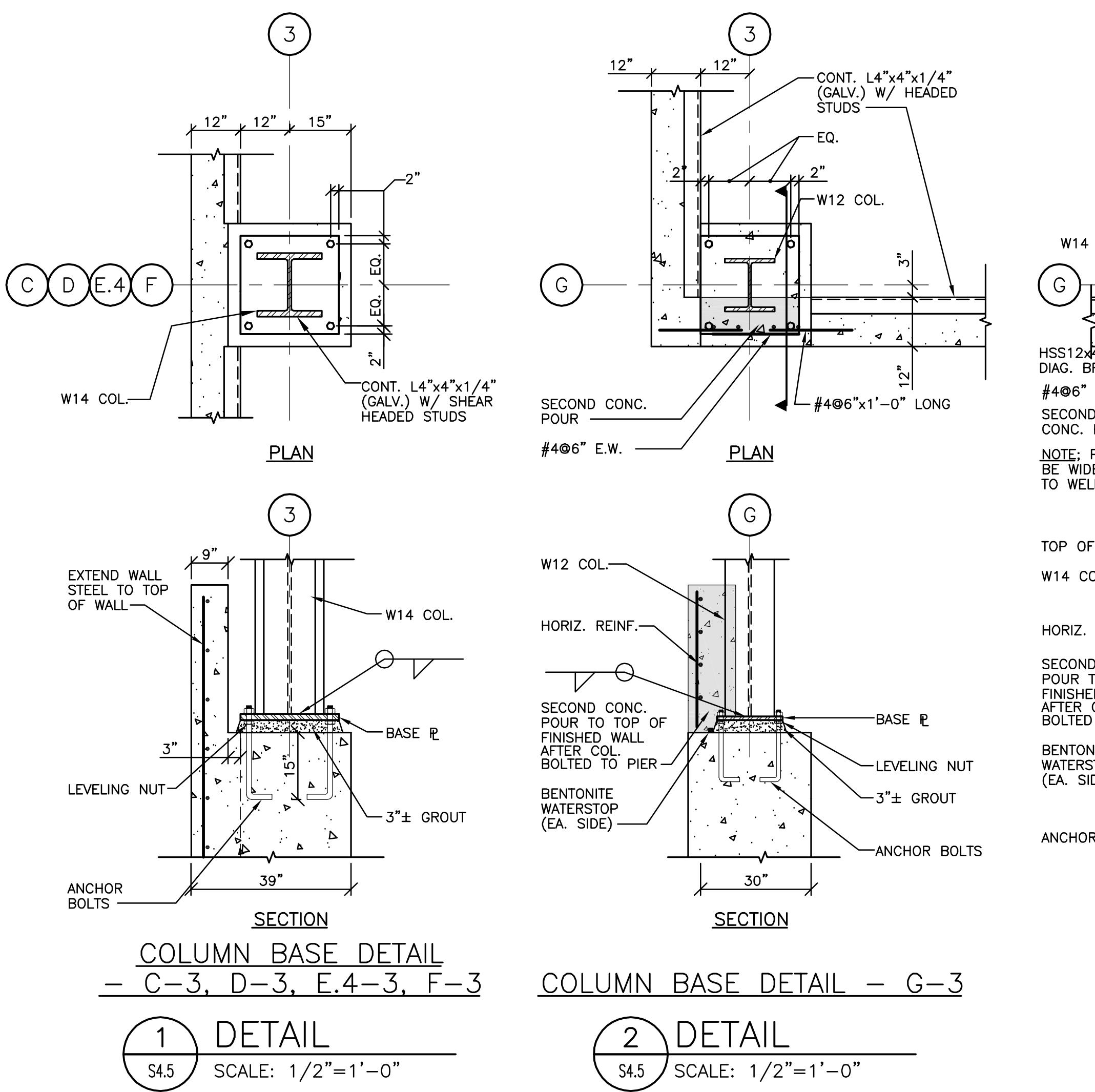
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Column & Base Plate Details

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S4.4



COLUMN BASE DETAIL
 - C-3, D-3, E.4-3, F-3

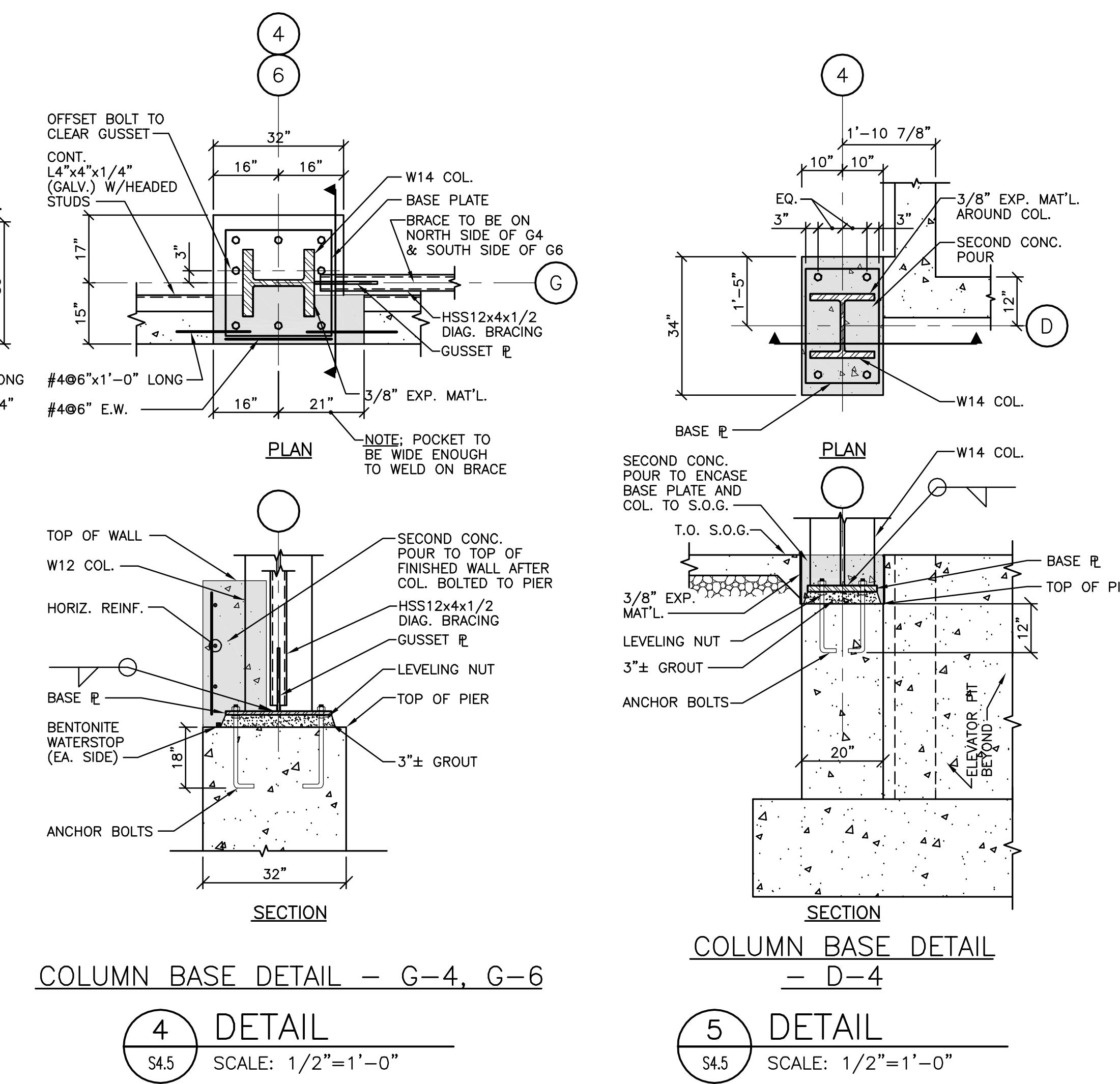
1 DETAIL
 S4.5 SCALE: 1/2"=1'-0"

COLUMN BASE DETAIL - G-3

2 DETAIL
 S4.5 SCALE: 1/2"=1'-0"

COLUMN BASE DETAIL - G-5

3 DETAIL
 S4.5 SCALE: 1/2"=1'-0"



COLUMN BASE DETAIL
 - G-4, G-6

4 DETAIL
 S4.5 SCALE: 1/2"=1'-0"

COLUMN BASE DETAIL
 - D-4

5 DETAIL
 S4.5 SCALE: 1/2"=1'-0"

COLUMN BASE DETAIL
 - F-4

6 DETAIL
 S4.5 SCALE: 1/2"=1'-0"

7 DETAIL
 S4.5 SCALE: 1/2"=1'-0"

MARK	SIZE	REINFORCEMENT	
		VERTICALS	TIES
P1	20"x20"	8-#8	#4@12"
P2	20"x34"	10-#8	#4@12"
P3	24"x24"	8-#8	#4@12"
P4	24"x36"	12-#8	#4@12"
P5	24"x40"	12-#8	#4@12"
P6	30"x30"	12-#8	#4@12"
P7	30"x35"	14-#8	#4@12"
P8	30"x39"	16-#8	#4@12"
P9	32"x39"	16-#8	#4@12"
P10	36"x36"	16-#8	#4@12"
P11	39"x39"	16-#9	#4@12"
P12	36"x44"*	16-#8	#4@12"
P13	38"x44"*	16-#8	#4@12"
P14	40"x48	16-#8	#4@12"
P15	32"x32"	12-#8	#4@12"
P16	16"x16"	8-#8	#4@12"

*SIZE TO SUIT MASONRY, SEE ARCH'S DWGS.

NOTES:
 1. ALL PIERS TO HAVE 5-#4 TIES @ 3" O.C. TOP.
 2. FOR ORIENTATION OF PIERS SEE DWGS. S4.4 & S4.5.
 3. FOR ELEVATIONS OF TOPS OF PIERS, SEE DWG. S1.0.

2 01/29/10 RECORD DRAWINGS
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Column & Base Plate Details

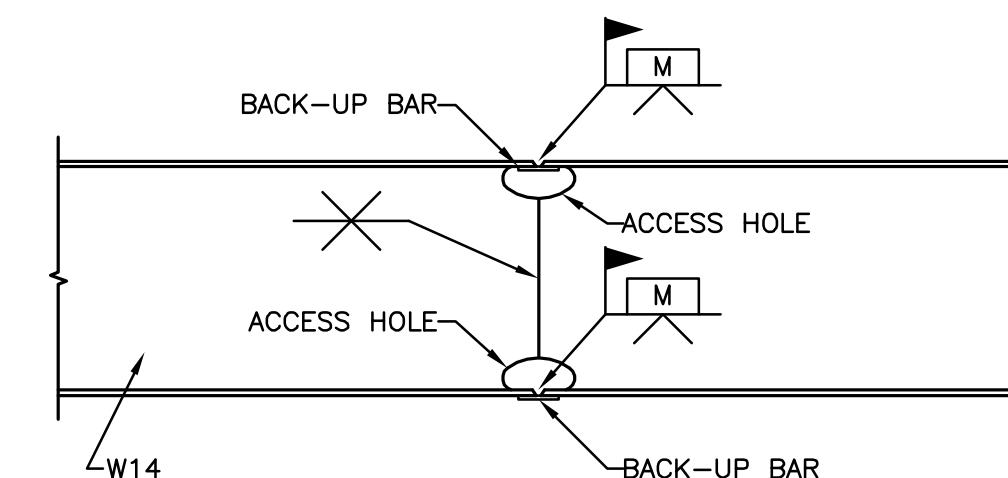
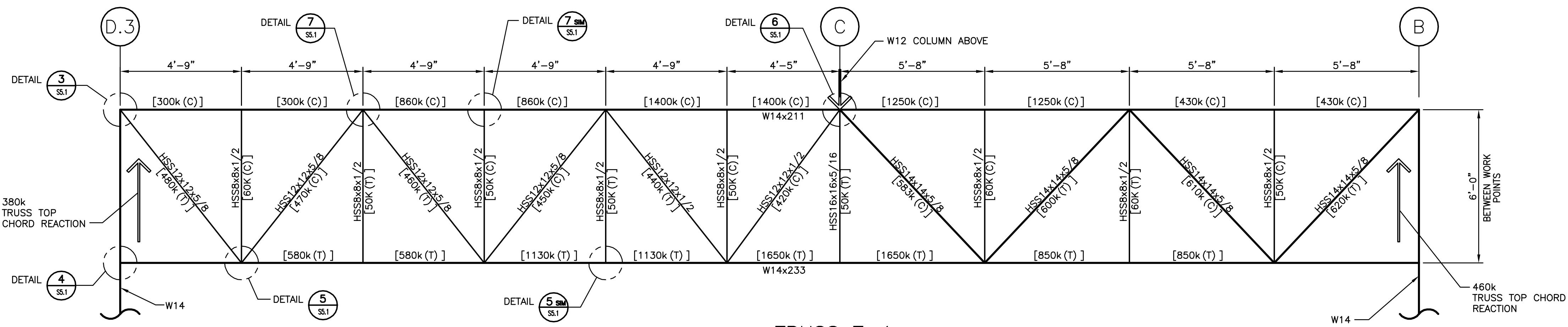
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S4.5



W14 TRUSS CHORD SPLICE

NOTES:

1. ALL WELDS ARE TO BE FULL PENETRATION BUTT WELDS.
2. ALL WELDS FOR SPLICES SHALL BE TESTED BY
ULTRASONIC TESTING.
3. CONTRACTOR SHALL SUBMIT PROPOSED LOCATION OF TOP
AND BOTTOM CHORD SPLICES TO ENGINEER FOR REVIEW
AND APPROVAL.

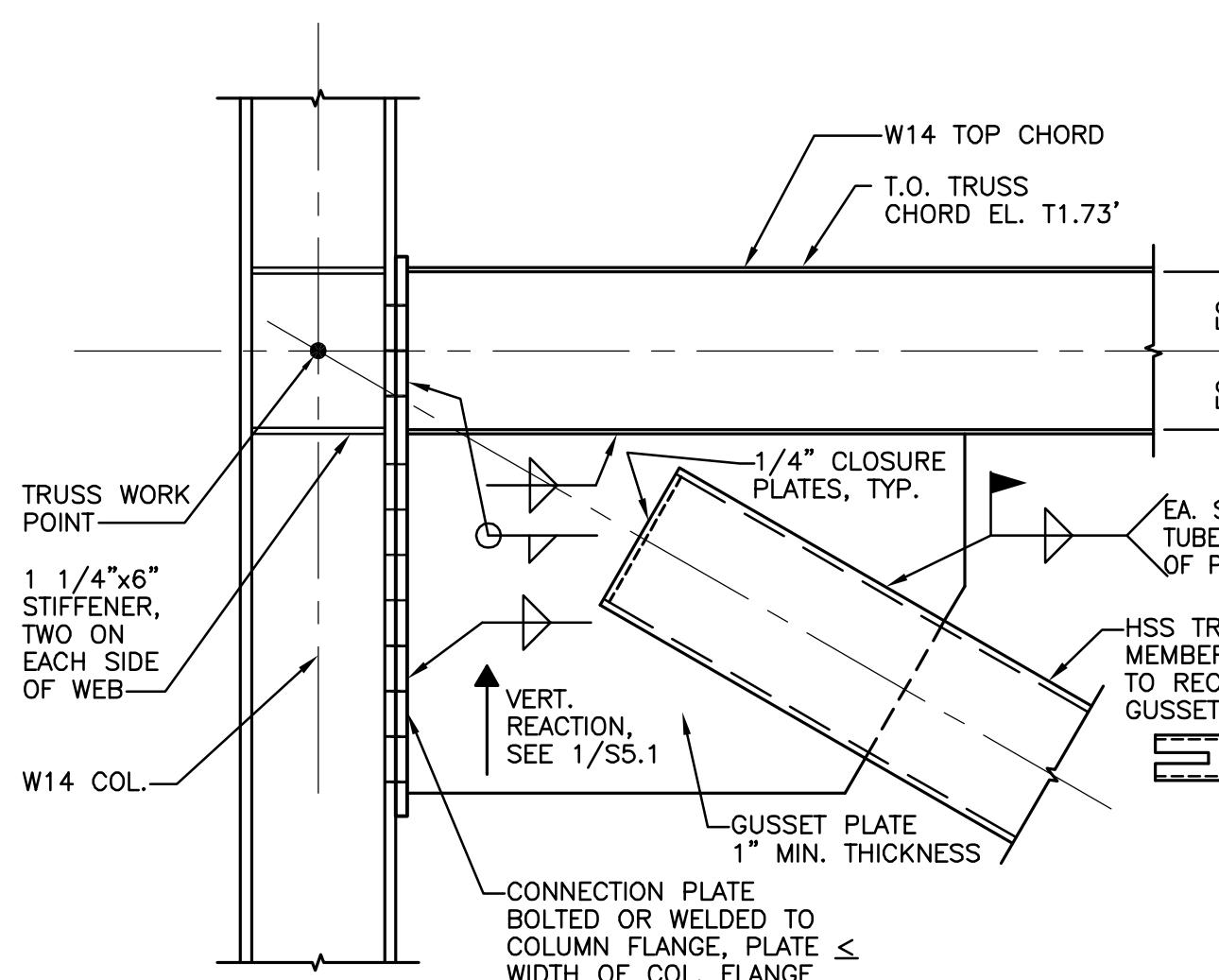
2 DETAIL

S5.1 NOT TO SCALE

TRUSS ELEVATION NOTES:

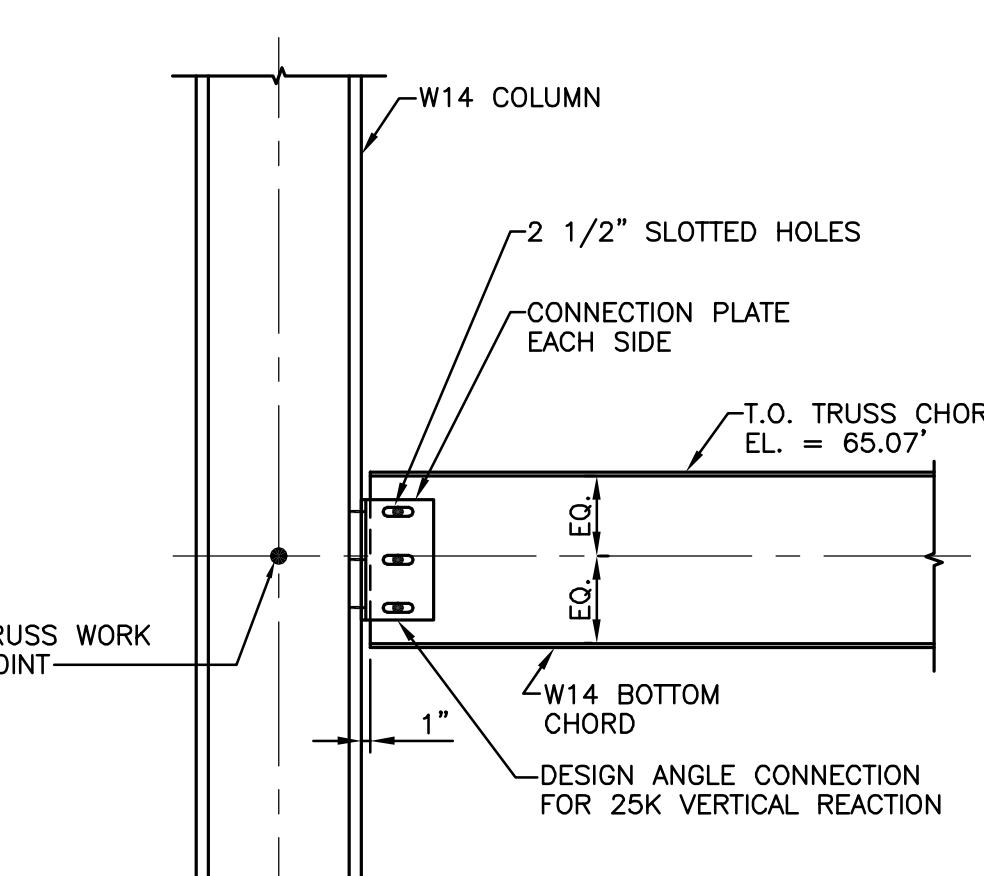
TRUSS ELEVATION NOTES:

1. TUBE-TO-GUSSET & GUSSET-TO-CHORD CONNECTIONS SHALL BE SHOP WELDED, EXCEPT WHERE NOT FEASIBLE.
2. TRUSS CONNECTIONS (INCLUDING GUSSET PLATE SIZES) SHALL BE DESIGNED BY THE STEEL FABRICATOR.
3. GUSSET PLATES SHALL BE 1" THICK MINIMUM, WITH STIFFENERS AS REQUIRED.
4. SEE FRAMING PLANS FOR ADDITIONAL SECTIONS, EXTENT OF CONCRETE SLAB, ETC.
5. CONTRACTOR SHALL SUBMIT PROPOSED SEQUENCE OF ERECTION DRAWINGS FOR THE TRUSS AND SUPPORT COLUMNS PRIOR TO FABRICATION. THE DRAWINGS SHALL SHOW CONSTRUCTION STAGES AND THE EXTENT OF CONSTRUCTION LOADS TO BE SUPPORTED BY THE TRUSS @ EACH STAGE.
6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ADEQUATELY BRACE THE TRUSS AND COLUMNS DURING ALL PHASES OF CONSTRUCTION.
7. STRUCTURAL STEEL FABRICATOR IS TO DESIGN AND DETAIL ALL TRUSS CONNECTIONS AND SUBMIT CALCULATIONS FOR CONNECTIONS SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE OF MARYLAND. FABRICATOR SHALL ALSO VERIFY DETAILS & SECTIONS SHOWN ON THIS DRAWING ARE ADEQUATE FOR LOCATIONS INDICATED AND STRESSES SHOWN.
8. SHOP AND FIELD WELDS ARE TO BE INSPECTED AND TESTED. SEE SPECIFICATIONS.
9. LOADS SHOWN ON MEMBERS THUS^kXXX (KIPS). (c)= COMPRESSION, (t)= TENSION.
10. CONNECTIONS OF TRUSS MEMBERS SHALL DEVELOP THE DESIGN LOADS INDICATED, BUT NOT LESS THAN 50% OF THE EFFECTIVE STRENGTH OF THE MEMBER.



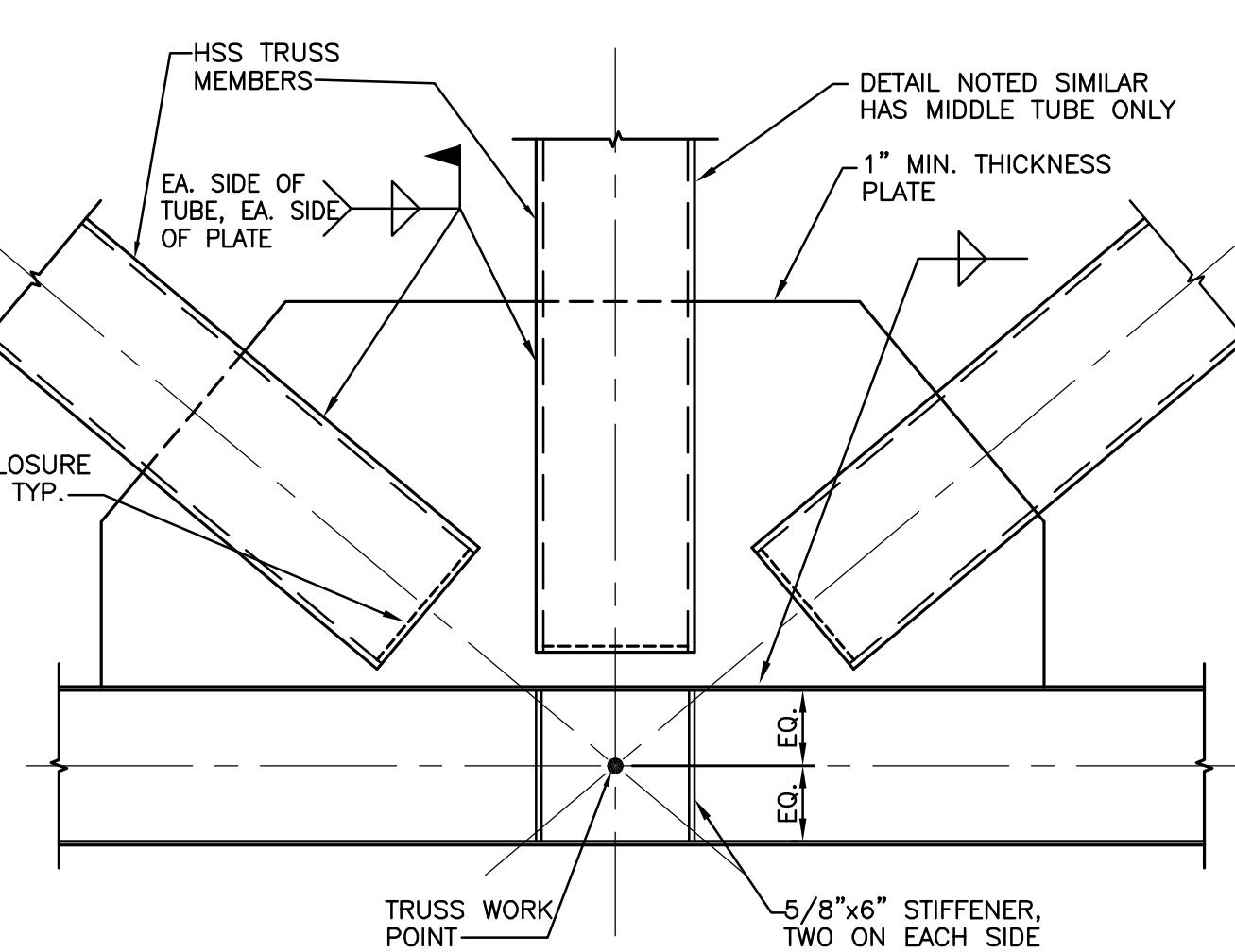
3 DETAIL

S5.1 SCALE: $3/4" \equiv 1'-0"$



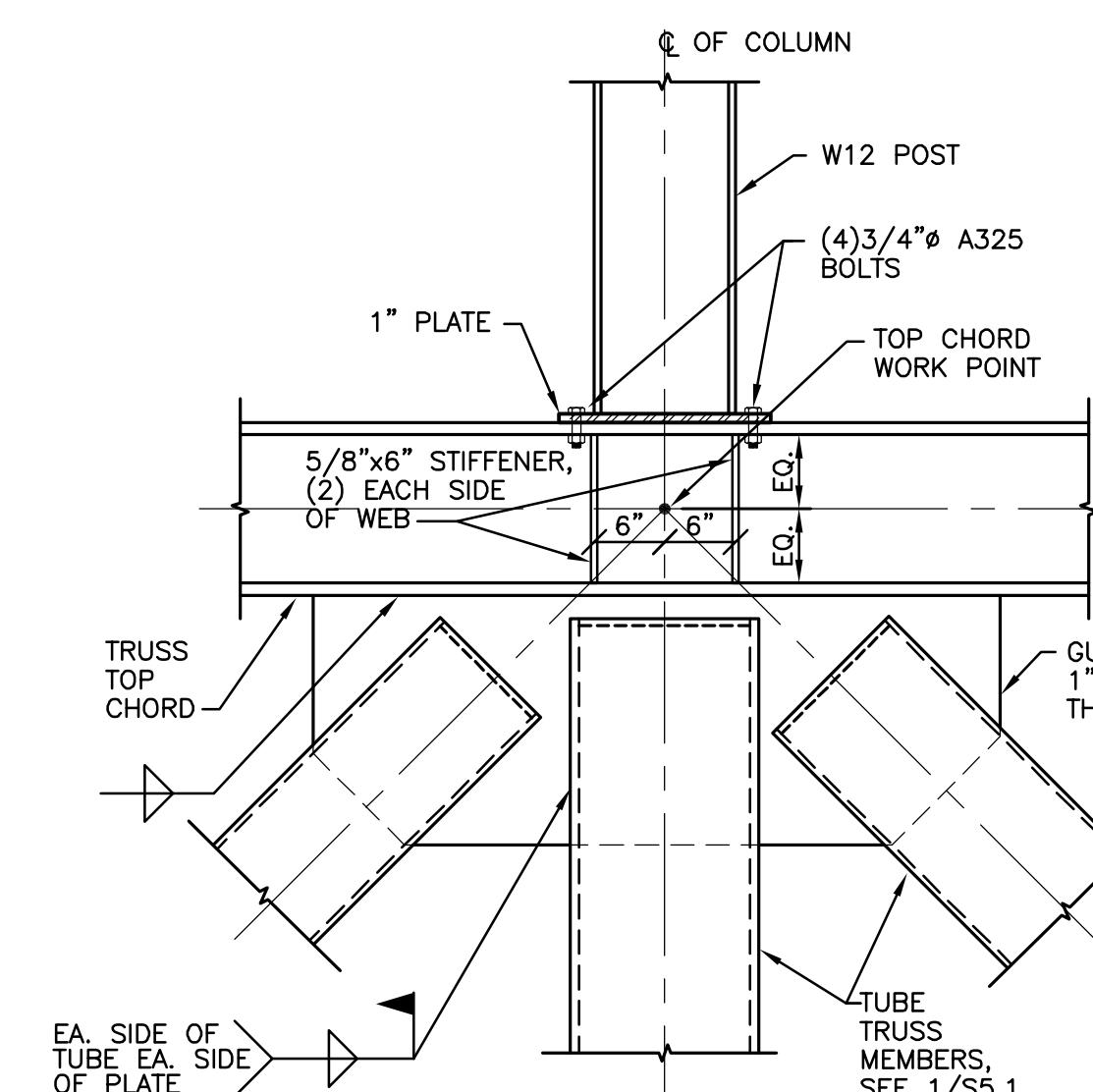
4 DETA

S5.1 SCALE: 3/4" = 1'-0"



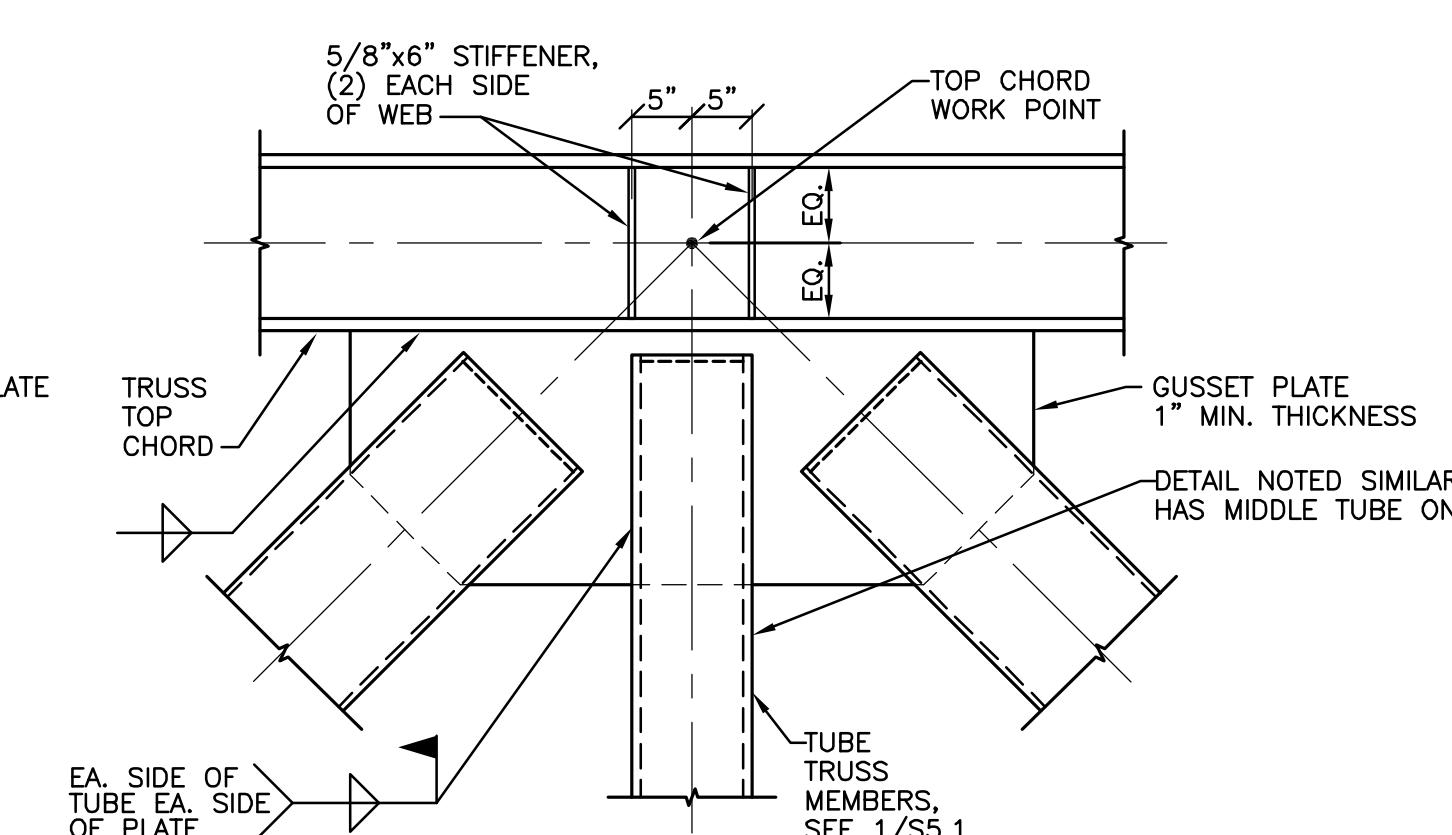
5 5 SIM DETA

S5.1 S5.1 SCAI E: 3/4"≡1'-0"



6 DETA

S5.1 SCALE: $3/4"=1'-0"$



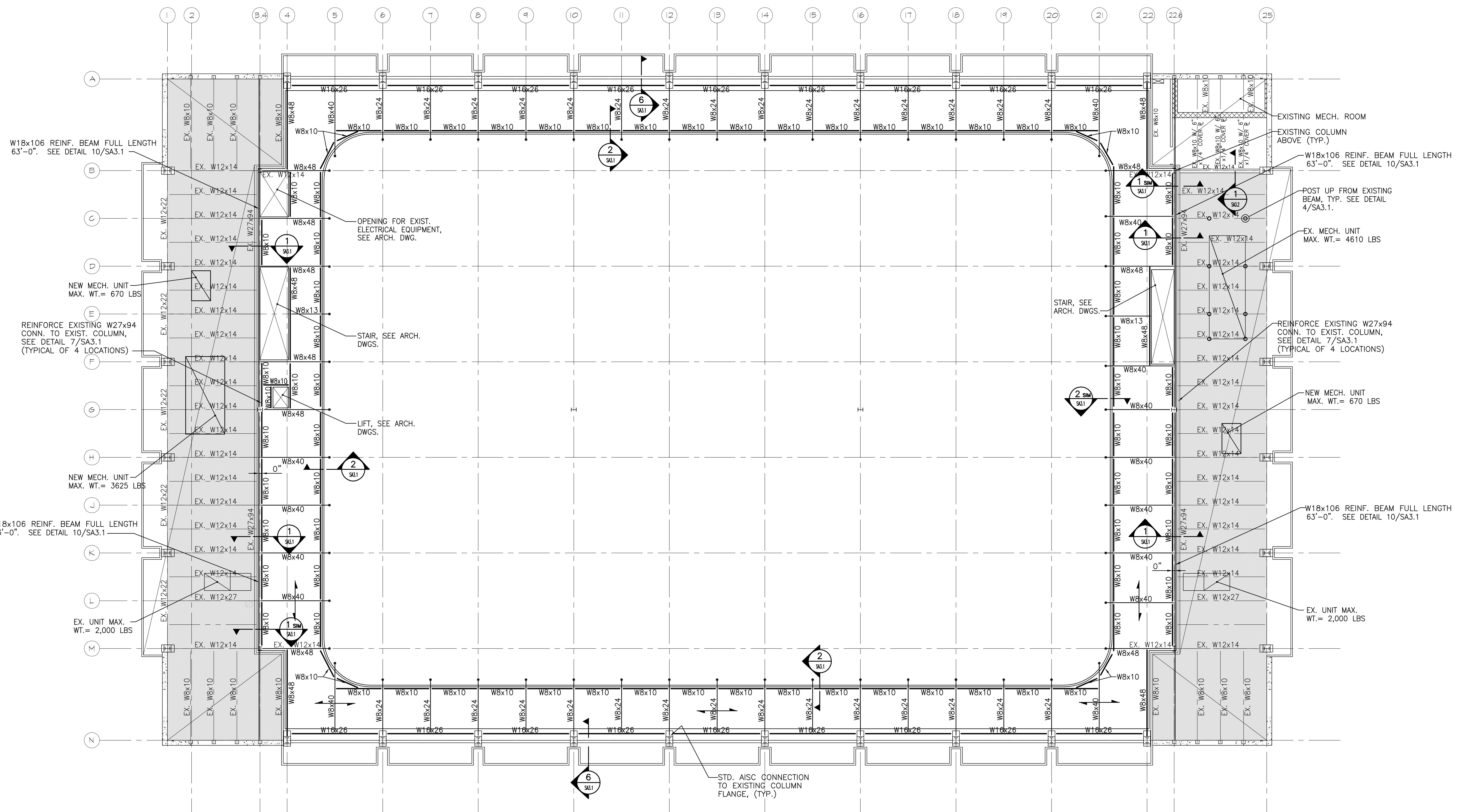
7 7 SIM DETAIL

S5.1 S5.1 SCALE: $3/4'' = 1'-0''$

Steel Truss Elevations And Details

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MEZZANINE
 LEVEL
 PLAN -
 PRATT
 GYMNASIUM

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SA1.1

ROOF FRAMING PLAN

Pratt Gymnasium

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SA1.2


ROOF FRAMING PLAN

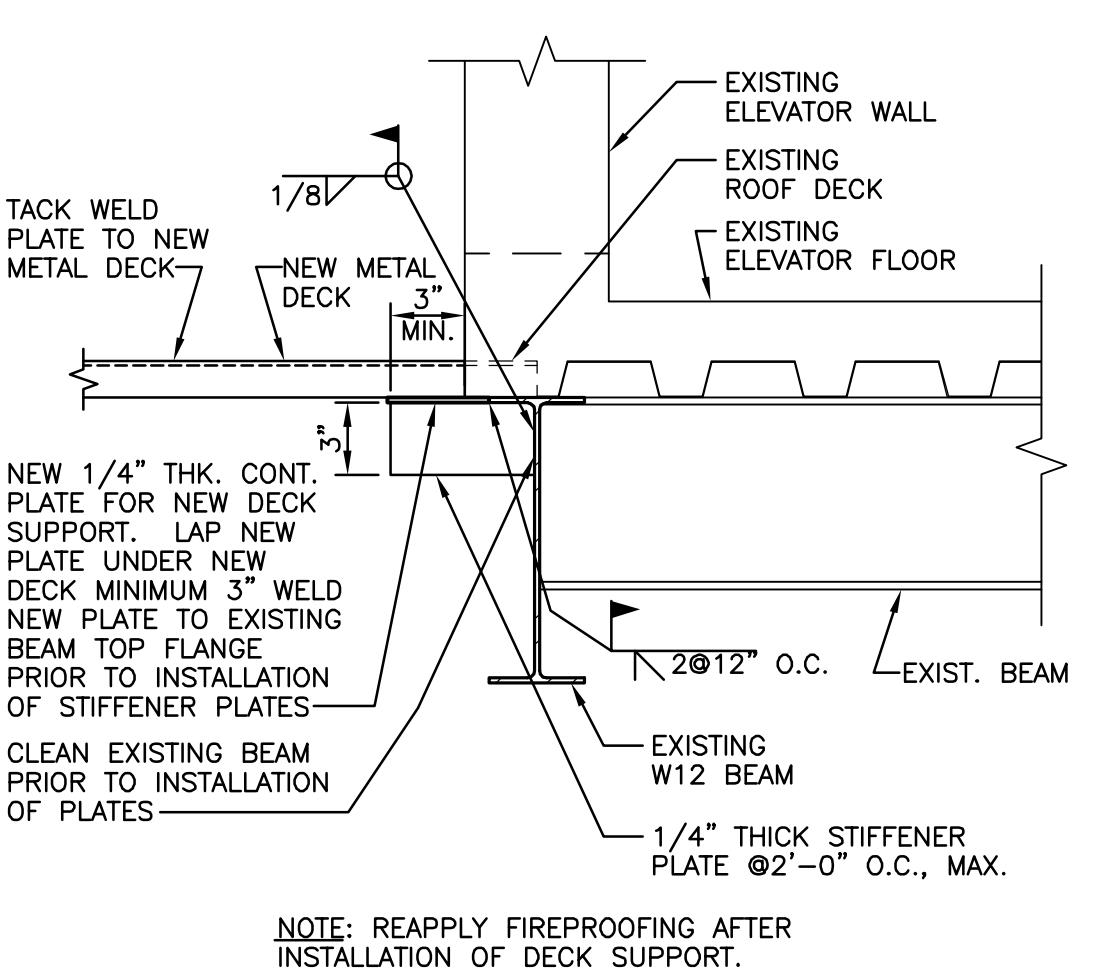
SCALE: 1/8"=1'-0"

PLAN NOTE:
1. []; DENOTES RUNNING TRACK BELOW, SEE AS1.1.
2. JOIST J-X; DENOTES STEEL JOIST REINF., SEE AS3.1.
3. ALL ANGLE REINFORCING SHALL BE A992 STEEL.

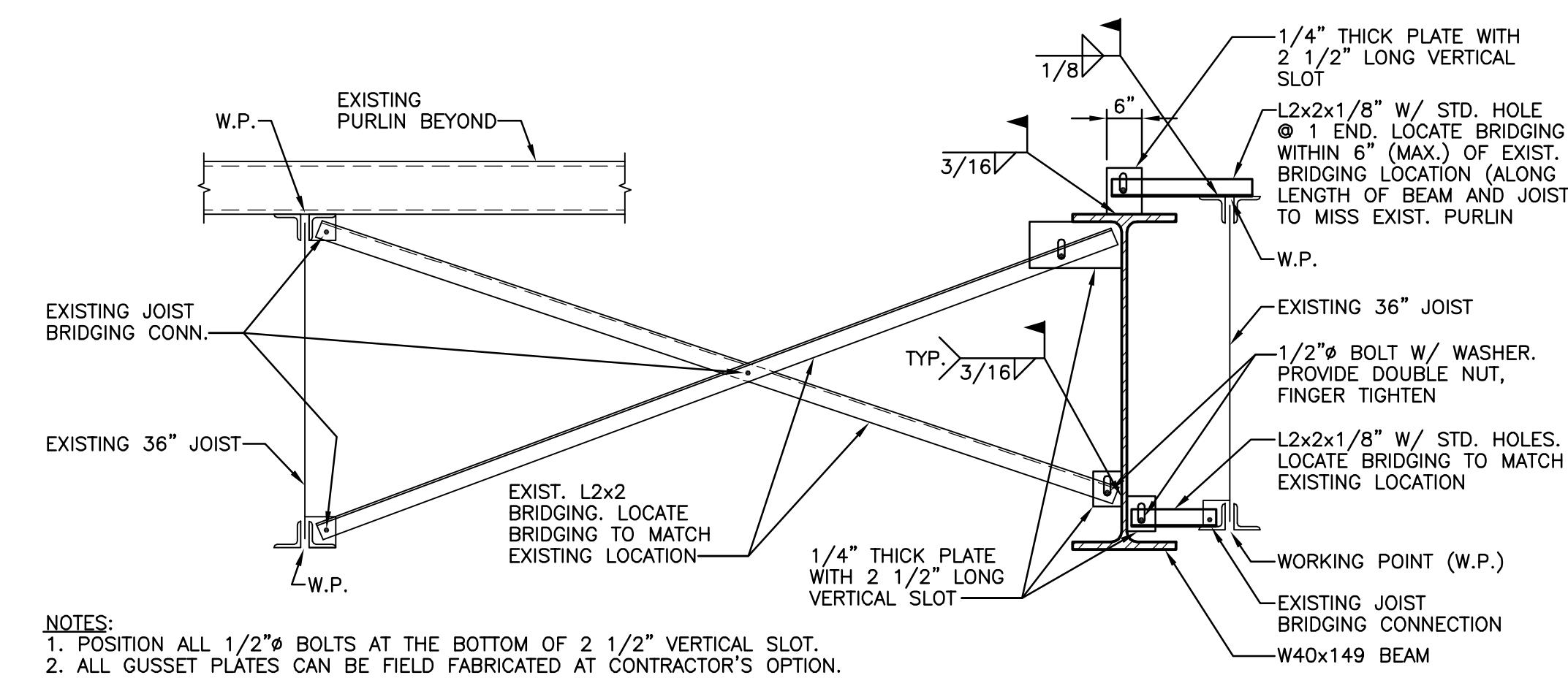
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Jan 22, 2010 - 9:11am User: rwillkins



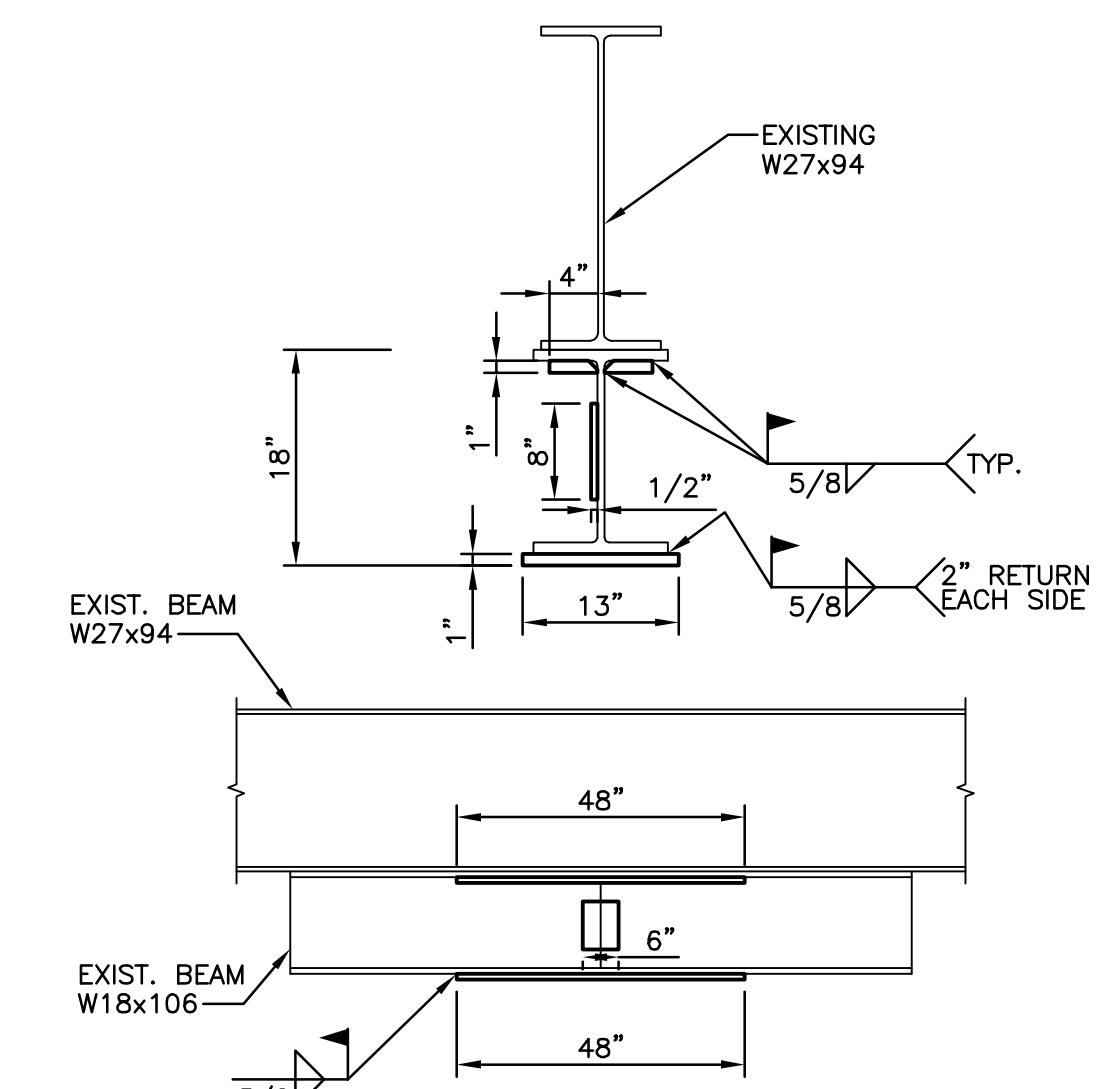


1 SECTION
 943.2 SCALE: 1 1/2" = 1'-0"

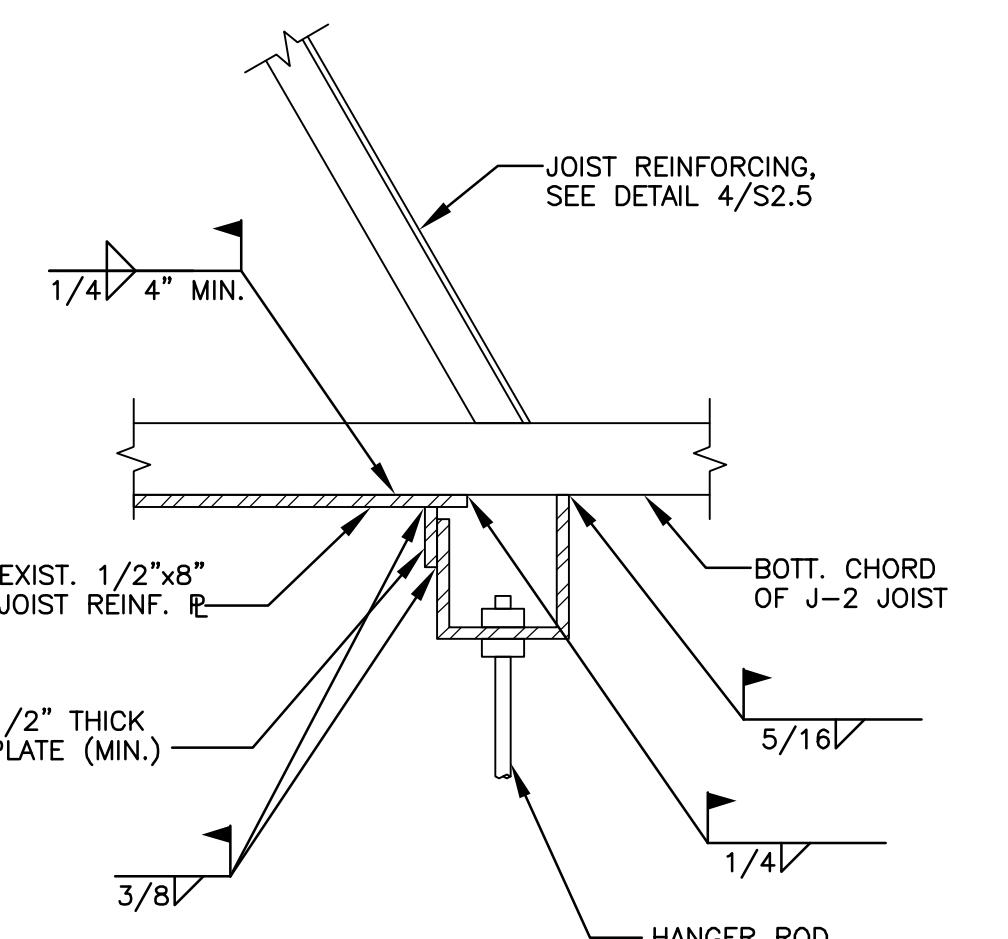


TYPICAL BRIDGING REATTACHMENT

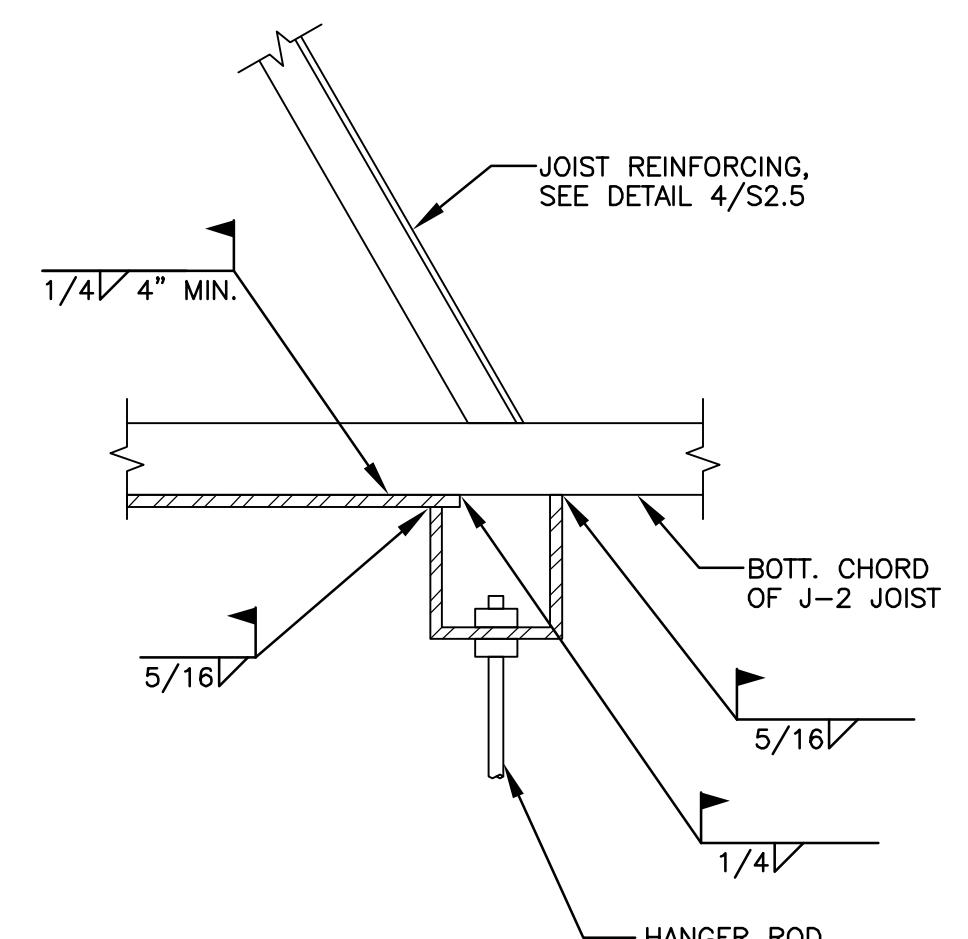
2 SECTION
 943.2 NOT TO SCALE



3 DETAIL
 943.2 NOT TO SCALE



4 DETAIL
 943.2 SCALE: 1 1/2" = 1'-0"



5 DETAIL
 943.2 SCALE: 1 1/2" = 1'-0"

1 01/29/10 RECORD DRAWINGS
 0 05/02/07 FOR CONSTRUCTION
 Rev. Date: Comment:
 Issued: MAY 2, 2007

Campus
 Center

West Lombard Street
 University of Maryland, Baltimore
 Baltimore, MD



Sections
 And
 Details -
 Pratt
 Gymnasium

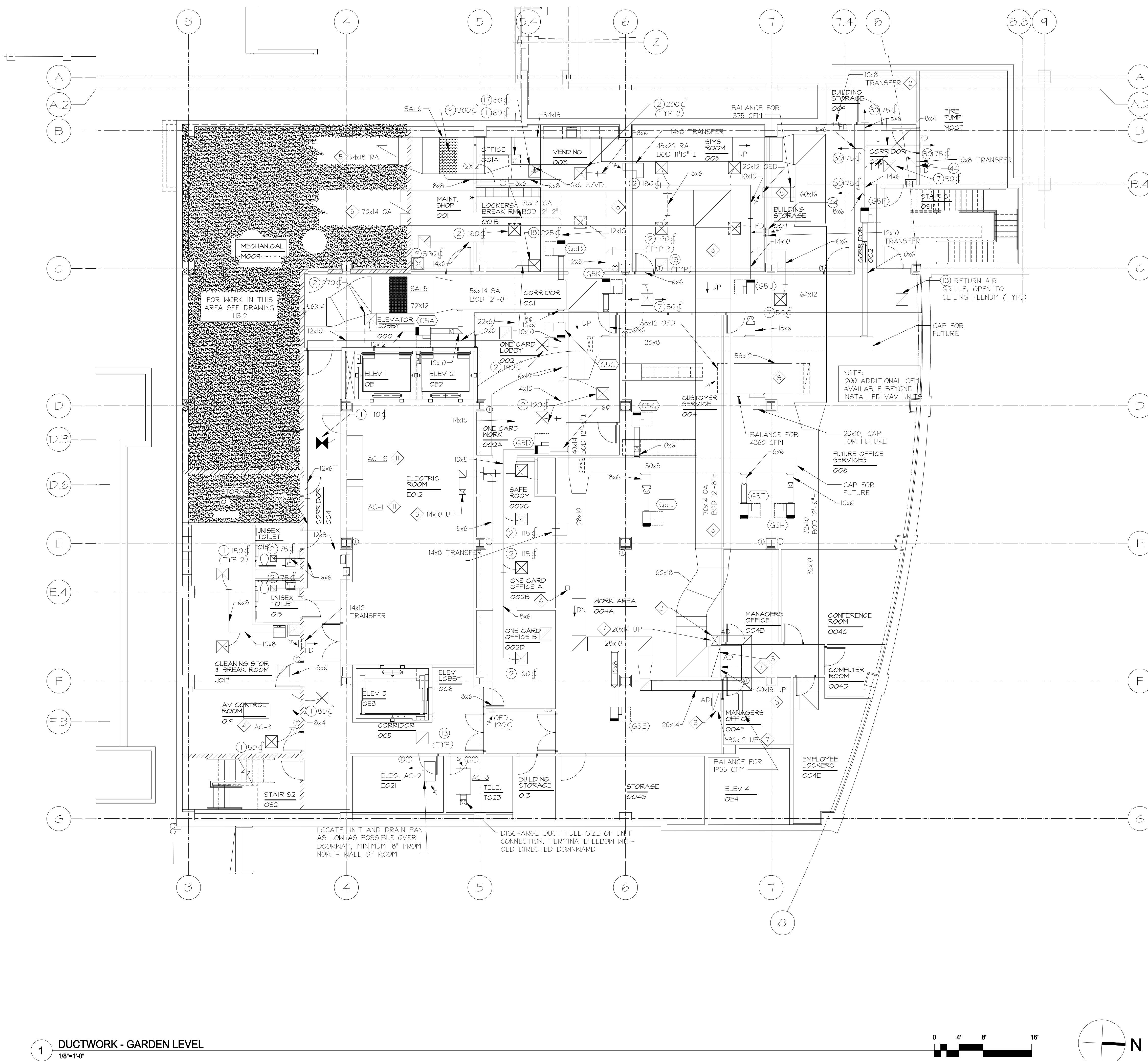
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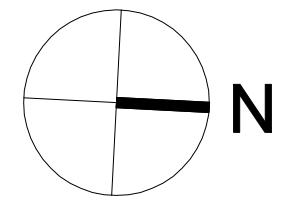
UMB PROJECT NO. 99-311
 WTW PROJECT NO. 703-4091

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SA3.2



1 DUCTWORK - GARDEN LEVEL
18'x14'



◇ SPECIAL NOTES:

- 1 N/A.
- 2 PROVIDE GRILLE (44) ON EACH WALL SURFACE.
- 3 PROVIDE FIRE DAMPER AT SLAB PENETRATION.
- 4 SEE DRAWING H3.5 FOR DUCTWORK ASSOCIATED WITH AC UNIT.
- 5 FOR CONT. SEE DWG. H3.2.
- 6 STATIC PRESSURE SENSOR. SEE AC SCHEMATICS.
- 7 RISER AND FIRE DAMPER IN ORIGINAL SCOPE OF WORK.
- 8 DUCT IN ORIGINAL SCOPE OF WORK.
- 9 N/A.
- 10 N/A
- 11 PROVIDE UNIT WITH OPTIONAL TOP DISCHARGE PLenum, WITH HORIZONTAL THROW LOUVERED AIR DEVICE.

DRAWING NOTES:

1. ALL OPEN END DUCTS SHALL BE PROVIDED WITH BELLMOUTH PER DETAIL.
2. TRANSFER DUCTS SHALL BE ABOVE CEILINGS, AND MIN. 90° AFF. DUCTS WITH ELBOWS SHALL BE MIN 18" LONG IN EACH LEG.
3. TRANSFER DUCTS SHALL BE GALVANIZED SHEET METAL PREP. ROLLING 10 IN. X 60 LINER WITH 1" SOUND LINING INSTALLED PER SECTION H590. INDICATED DIMENSIONS ARE CLEAR AIR SIDE. INCREASE SHEET METAL DIMENSIONS TO SUIT.



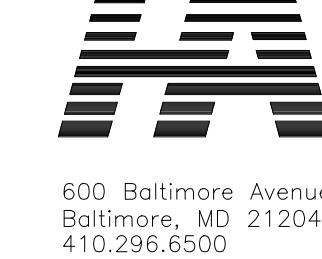
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Seattle, WA 98101-3013
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MCC PROJECT NO. 0512300

20 1/29/10 RECORD DRAWING
19 4/26/09 BULLETIN #120
18 4/26/09 BULLETIN #109
17 4/26/09 BULLETIN #108R
16 3/20/09 BULLETIN #125
15 4/15/09 BULLETIN #94R
14 4/15/09 BULLETIN #94
13 12/16/08 BULLETIN #82
12 12/16/08 BULLETIN #81
11 11/20/08 BULLETIN #79
10 11/19/08 BULLETIN #72 (rev)
9 9/4/08 BULLETIN #63
7 7/28/08 BULLETIN #55
6 6/27/08 BULLETIN #44
5 5/23/08 COORD. DWG. RP. 163
4 5/09/08 BULLETIN #31
3 04/14/08 CB-39
2 03/10/08 CB-32
1 03/10/08 CB-31
0 05/02/07 For Construction
Rev. Date: Comment:
Issued: May 2, 2007

Campus Center

621 West Lombard Street
University of Maryland,
Baltimore
Baltimore, MD

Ductwork
Garden
Level

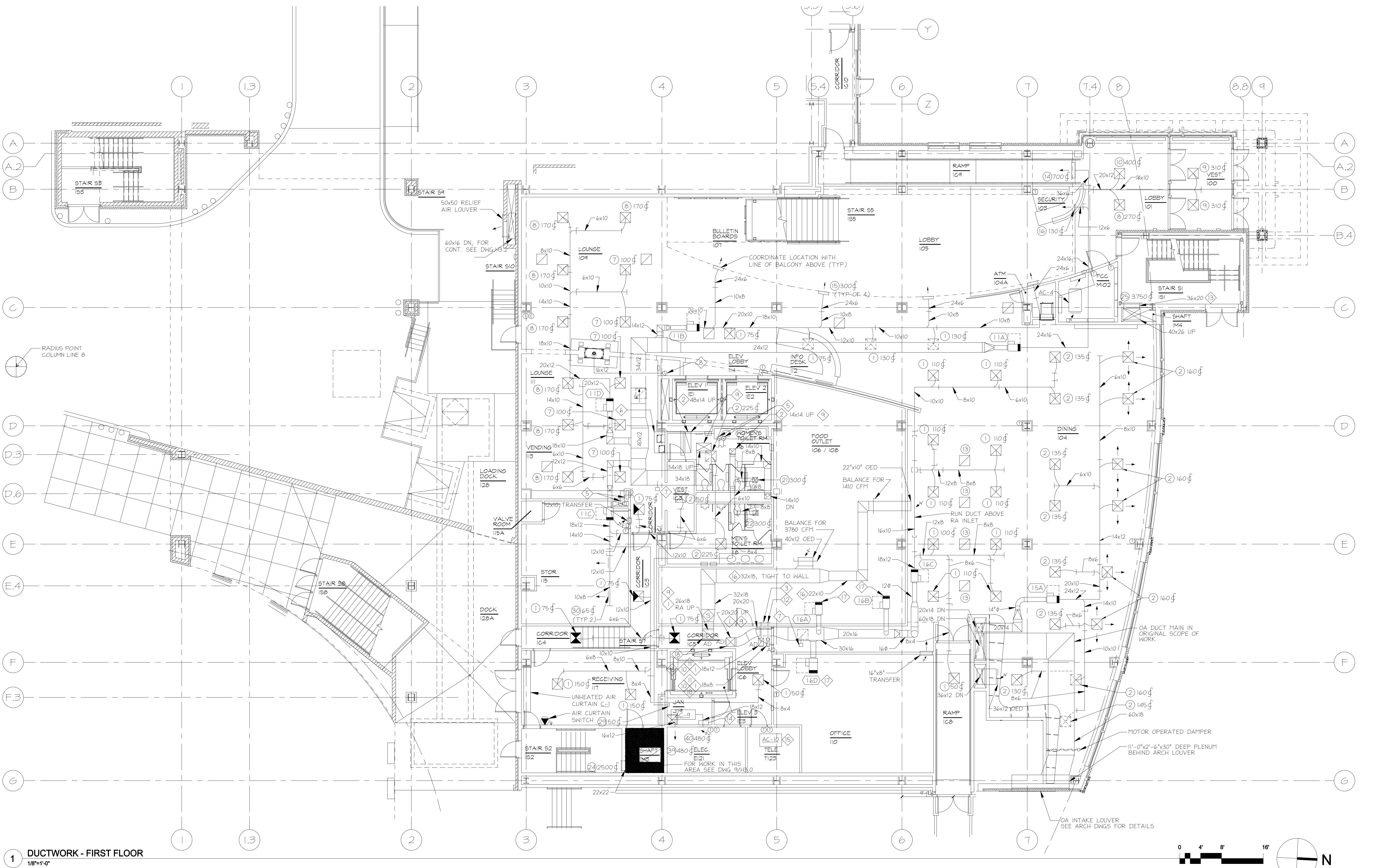
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If this drawing does not measure exactly 30" x 42", it has been reduced or enlarged. Please do not scale this drawing.

UMB PROJECT NO. 99-311
WTW PROJECT NO. 70-4091
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H1.0



DRAWING NOTES:

1. ALL OPEN END DUCTS SHALL BE PROVIDED WITH BELLMOUTH PIER DETAIL.
2. TRANSFER DUCTS SHALL BE ABOVE CEILINGS, AND MIN. 90° AFF. DUCTS WITH ELBOWS SHALL BE MIN 18" LONG IN EACH LEG.
3. TRANSFER DUCTS SHALL BE GALVANIZED SHEET METAL, PRESSURE RATING 1.0 IN. W.G., LINED WITH 1" SOUND LINING INSTALLED PER SECTION 15B90. INDICATED DIMENSIONS ARE CLEAR AIR SIDE, INCREASE SHEET METAL DIMENSIONS TO SUIT.

SPECIAL NOTES:

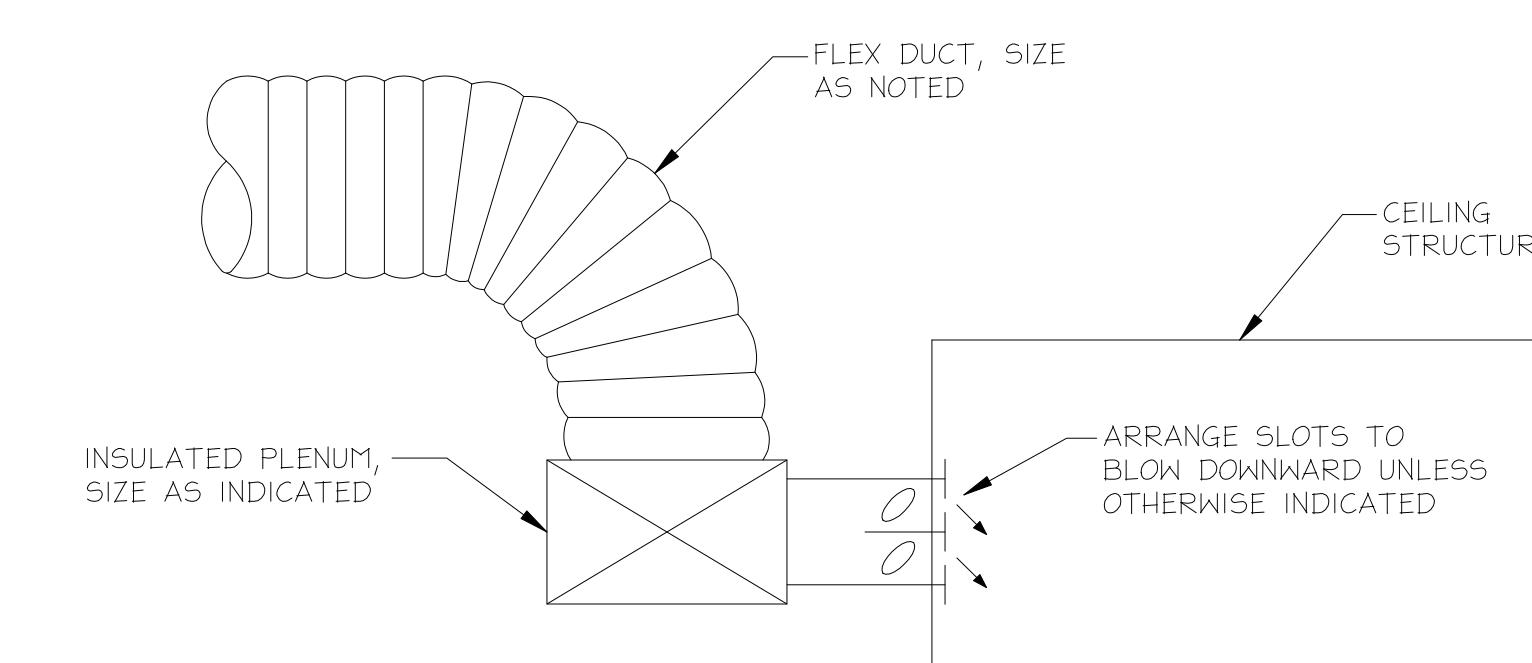
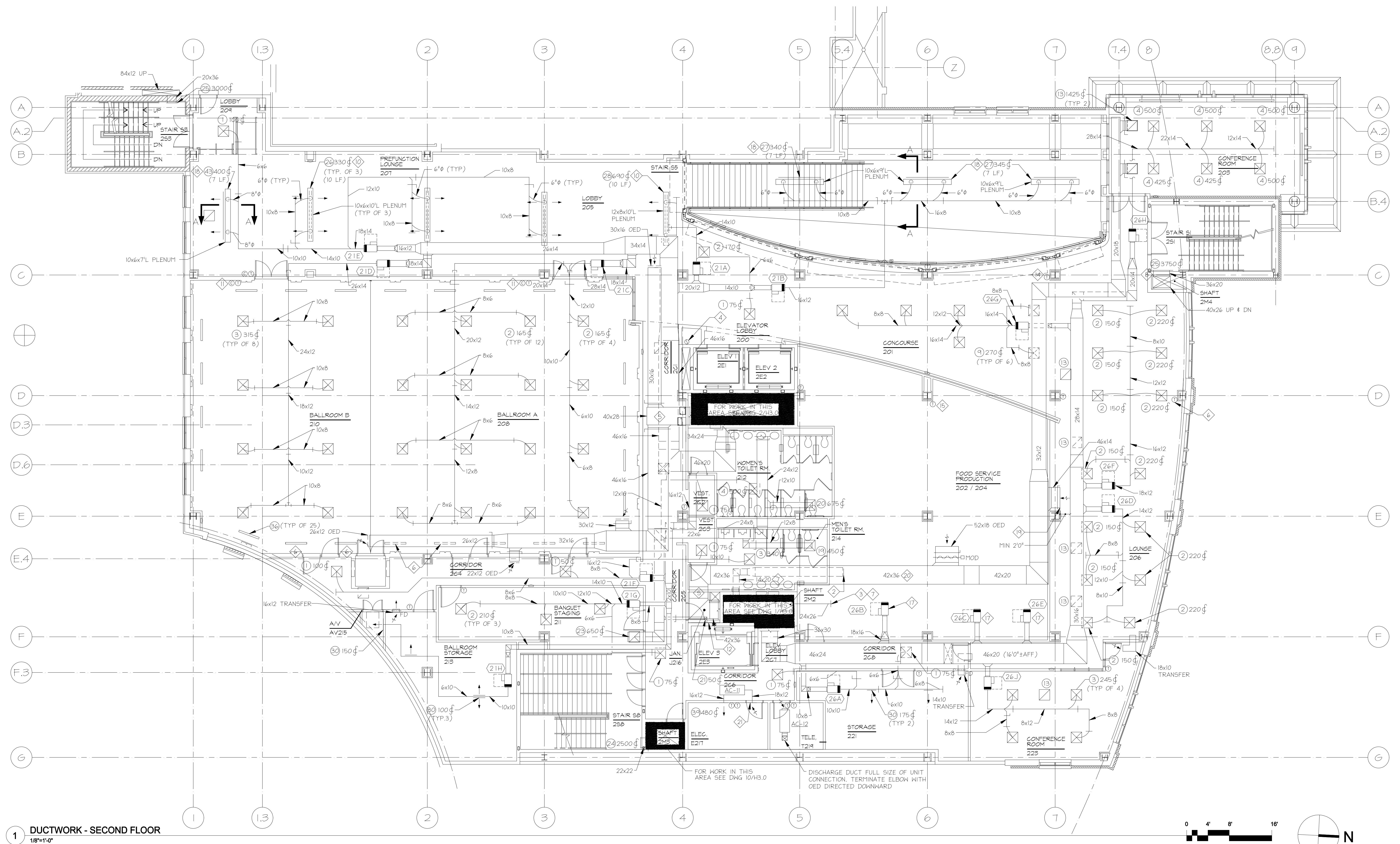
1. FOR CONTINUATION SEE DRAWING 1/H3.0.
2. FOR CONTINUATION SEE DRAWING 2/H3.0.
3. 12x18 CAP FOR FUTURE CONNECTION TO GREASE EXHAUST HOODS.
4. 10x8 EXHAUST DUCT DOWN TO GARDEN LEVEL CEILING SPACE, FOR CONTINUATION SEE H1.0.
5. SEE DWG. H2.1 FOR CHILLED WATER PIPING IN THIS AREA.
6. 48x14 OED.
7. STATIC PRESSURE SENSOR, SEE ATC SCHEMATICS.
8. 10" O.D. TYPE L FLUE VENT UP, COORD LOCATION WITH PLUMBING RISERS IN SHAFT.
9. PROVIDE FIRE DAMPER AT SLAB PENETRATION.
10. GREASE EXHAUST, PROVIDE FIRE RATED INSULATION AS SPECIFIED.
11. PROVIDE RATED CLEANOUT ACCESS DOOR AT BOTTOM OF GREASE EXHAUST RISER.
12. 18x18 CAP FOR FUTURE CONNECTION TO GREASE EXHAUST HOODS.
13. LOCATE GRILLE AS HIGH AS POSSIBLE.
14. ATC PANEL
15. SEE DWG. H3.5 FOR DUCT LAYOUT.
16. TIGHT TO STRUCTURE.
17. FOR FUTURE USE UNDER SEPARATE TENANT FIT-OUT PROJECT. BALANCE ACCORDING TO SCHEDULED AIRFLOW, UNDER THIS PROJECT.
18. RISER AND FIRE DAMPER IN ORIGINAL SCOPE OF WORK.

**Ductwork
First
Floor**

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DOCUMENTS

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H1.1


 2 DETAIL A-A: SLOT DIFFUSERS AT BULKHEADS
NOT TO SCALE

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DRAWING NOTES:

- ALL OPEN END DUCTS SHALL BE PROVIDED WITH BELLMOUTH PER DETAIL.
- TRANSFER DUCTS SHALL BE ABOVE CEILINGS, AND MIN. 90° AFF. DUCTS WITH ELBOWS SHALL BE MIN 18" LONG IN EACH LEG.
- TRANSFER DUCTS SHALL BE GALVANIZED SHEET METAL, PLATE ROLLING 1.0 IN. MIN. LENGTH WITH 1" SOUND LINING INSTALLED PER SECTION 16890. INDICATED DIMENSIONS ARE CLEAR AIR SIDE, INCREASE SHEET METAL DIMENSIONS TO SUIT.

◇ SPECIAL NOTES:

- SLOT DIFFUSERS RETURN OPEN TO PLENUM.
- 20x14 OED, FOR FUTURE CONNECTION TO GREASE EXHAUST HOODS.
- 26x24 OED, FOR FUTURE CONNECTION TO GREASE EXHAUST HOODS.
- 10" OD TYPE L FLUE VENT UP & DN. CORD LOCATION WITH PLUMBING RISERS.
- NOT USED.
- BLANK OFF THIS AIR DEVICE.
- GREASE EXHAUST, PROVIDE FIRE RATED INSULATION AS SPECIFIED.
- LOCATE GRILLE AS HIGH AS POSSIBLE.
- SLOT DIFFUSERS RETURN OPEN TO PLENUM, SUCH THAT DEVICE EXTENDS FULL LENGTH OF ADJACENT 16' HIGH CEILING. BLANK SLOTS SHALL BE OPEN TO CEILING PLENUM FOR RETURN AIR USE.
- LOCATE SLOT DIFFUSER IN BOTTOM OF PLENUM.
- VERIFY SENSOR LOCATIONS WITH OWNER & ARCHITECT.
- ATC PANEL.
- NOT USED.
- CONTROL UNITS 26D AND 26E IN PARALLEL.
- FOR FUTURE USE UNDER SEPARATE TENANT FIT-OUT PROJECT, BALANCE ACCORDING TO SCHEDULED MINIMUM AIRFLOW, UNDER THIS PROJECT.
- PROVIDE ADDITIONAL 1 LF BLANK DIFFUSER AT EACH END TO BRING TOTAL VISIBLE LENGTH TO 9 LF, CENTERED ON BULKHEAD.
- BALANCE FOR 56800.
- TIGHT TO STRUCTURE.
- 16x24 FILTER RETURN GRILLE (TITUS 350RFLI OR EQUAL, WITH 1" MERV 7 FILTER). REMOVE FILTER FROM AC-11.

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